

OLT-23-000534
OLT-23-000939
OLT-23-000940

ONTARIO LAND TRIBUNAL

PROCEEDING COMMENCED UNDER subsection 22(7) of the *Planning Act*, R.S.O. 1990, c. P.13, as amended.

Applicant and Appellant: Caivan (Perth GC) Limited
Subject: Request to amend the Official Plan – Failure to adopt the requested amendment
Description: To permit 940 single detached dwellings and townhomes, a nine-hole golf course, park and open space areas
Reference Number: OPA-01-2023
Property Address: 141 Peter Street, Part of Lots 26 & 27, Concession 1, Part Lots 25, 26 & 27, Concession 2, Geographic Township of Bathurst, and Part Lot 1 in Southeast Half Lot 1, Concession 1, Part Lot 1 in Southwest Half Lot 1, Concession 2, Geographic Township of Drummond, now in the Town of Perth, County of Lanark
Municipality/UT: Town of Perth / County of Lanark
OLT Case No.: OLT-23-000939
OLT Lead Case No.: OLT-23-000534

PROCEEDING COMMENCED UNDER subsection 34(11) of the *Planning Act*, R.S.O. 1990, c. P.13, as amended.

Applicant and Appellant: Caivan (Perth GC) Limited
Subject: Application to amend the Zoning By-law – Refusal or neglect to make a decision
Description: To permit 940 single detached dwellings and townhomes, a nine-hole golf course, park and open space areas
Reference Number: ZBL-03-2023
Property Address: 141 Peter Street, Part of Lots 26 & 27, Concession 1, Part Lots 25, 26 & 27, Concession 2, Geographic Township of Bathurst, and Part Lot 1 in Southeast Half Lot 1, Concession 1, Part Lot 1 in Southwest Half Lot 1, Concession 2, Geographic Township of Drummond, now in the Town of Perth, County of Lanark
Municipality/UT: Town of Perth / County of Lanark
OLT Case No.: OLT-23-000940

PROCEEDING COMMENCED UNDER subsection 51(34) of the *Planning Act*, R.S.O. 1990, c. P.13, as amended.

Applicant and Appellant: Caivan (Perth GC) Limited
Subject: Proposed Plan of Subdivision – Failure of Approval Authority to make a decision
Description: To permit 940 single detached dwellings and townhomes, a nine-hole golf course, park and open space areas
Reference Number: 09-T-22001

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Property Address: 141 Peter Street, Part of Lots 26 & 27, Concession 1, Part
Lots 25, 26 & 27, Concession 2, Geographic Township of
Bathurst, and Part Lot 1 in Southeast Half Lot 1, Concession
1, Part Lot 1 in Southwest Half Lot 1, Concession 2,
Geographic Township of Drummond, now in the Town of
Perth, County of Lanark
Municipality/UT: Town of Perth / County of Lanark
OLT Case No.: OLT-23-000534
OLT Lead Case No.: OLT-23-000534
OLT Case Name: Caivan (Perth GC) v Lanark County

Witness Statement of Jean-Francois Sabourin, M.Eng., P. Eng.

Engineer, Director of Water Resources

JFSA Canada Inc.

Qualifications

1. I am the founder and president of J.F. Sabourin and Associates Inc., now JFSA Canada Inc. (JFSA). JFSA has offered specialized engineering and technical services in water resources and other related disciplines in Ontario for over 30 years.
2. I have 40 years of experience in providing engineering services to public and private clients within Eastern Ontario and the Greater Toronto Area, as well as across the Province of Québec. I have undertaken, managed and directed numerous water resources-related studies, such as detailed conceptual drainage designs for new developments, rehabilitation of municipal projects related to drainage systems, low impact development (LID) research studies, flood mapping studies, watershed studies, rainfall/flow/snowmelt monitoring studies, hydrologic model calibration, master drainage plans, and development and programming of several hydrologic software that are distributed across Canada, such as SWMHYMO.
3. I am currently registered as a Professional Engineer in the Provinces of Ontario and Quebec.
4. My curriculum vitae is attached to this witness statement as **Appendix “A”**. A copy of my Acknowledgement of Expert Duty is attached as **Appendix “B”**.

Retainer

5. JFSA was retained by Caivan (Perth GC) Limited (Caivan) through the office of David Scheaffer Engineering Limited (DSEL) in December 2021 to provide technical guidance related to the stormwater and water resource-related components of the proposed development.
6. As part of this retainer, I provided senior engineering direction and guidance with respect to the establishment of existing hydraulic and hydrologic conditions of the site. This included an update to the current floodplain mapping and input to the preliminary Storm Water Management (SWM) design for the proposed development.

7. JFSA staff have been on-site on numerous occasions since 2021 completing field monitoring and topographic survey work.
8. JFSA has authored and/or contributed to the following:
 - Perth Golf Course Floodplain Alterations Memo – March 2022, JFSA (contained within the Caivan Perth Development - Hydrologic and Hydraulic Conditions Report).
 - Caivan Perth Development - Hydrologic and Hydraulic Conditions Report, February 2023, JFSA.
 - Grants Creek Wetland Integrated Hydrologic Impact Assessment - Perth Golf Course - February 2023, JFSA, GEMTEC, Kilgore & Associates, David Scheaffer Engineering Ltd.
 - Response to RVCA comments: Western Annex Lands Memo, Town of Perth, April 9, 2024, JFSA.
 - Perth Golf Course – Proposed Floodplain Mapping Amendment Memo, May 31, 2024, JFSA.
9. A copy of the “Caivan Perth Development - Hydrologic and Hydraulic Conditions Report” is attached to this witness statement as **Appendix “C”**.
10. A copy of the “Perth Golf Course – Proposed Floodplain Mapping Amendment Memo” is attached to this witness statement as **Appendix “D”**.

Documents Reviewed

11. As part of this retainer and in preparation for my evidence, I reviewed the following documents:
 - a. Technical Guide – River & Stream systems: Flooding Hazard Limit, Ontario Ministry of Natural Resources, 2002.

- b. Stormwater Management Planning and Design Manual, Ministry of the Environment, March 2003.
- c. Ottawa Sewer Design Guidelines, City of Ottawa, October 2012.
- d. Technical Bulletin ISDTB-2014-01, Revisions to Ottawa Design Guidelines – Sewer, City of Ottawa, February 5, 2014.
- e. Technical Bulletin PIEDTB-2016-01, Revisions to Ottawa Design Guidelines – Sewer, City of Ottawa, September 6, 2016.
- f. Technical Bulletin ISTB-2018-01, Revisions to Ottawa Design Guidelines – Sewer, City of Ottawa, March 21, 2018.
- g. Technical Bulletin ISTB-2018-03, Revisions to Ottawa Design Guidelines – Sewer, City of Ottawa, March 21, 2018.
- h. Technical Bulletin ISTB-2019-01, Revisions to Ottawa Design Guidelines – Sewer, City of Ottawa, January 2019.
- i. Technical Bulletin ISTB-2019-02, Revisions to Ottawa Design Guidelines – Sewer, City of Ottawa, July 8, 2019.
- j. Flooding Hazards: Data Survey and Mapping Specifications bulletin, Ministry of Natural Resources and Forestry, December 2023.
- k. Height Reference System Modernization, Natural Resources Canada, October 2020.
- l. Interim Policy for the Administration and Implementation of Ontario Regulation 41/24, RVCA, April 2024.
- m. SWMHYMO User manual, J.F Sabourin and Associates Inc, April 2005.
- n. HEC-RAS Hydraulic Reference Manual, US Army Corps of Engineers, December 2020.
- o. Western Annex Lands, Town of Perth Floodplain Modification, RVCA June 22, 2023.

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- p. Western Annex Lands, Town of Perth – Response to JFSA Comments Floodplain Modification, RVCA, April 30, 2024.
 - q. Functional Servicing Report for Caivan (Perth GC) Limited - Proposed Residential Subdivision, David Schaeffer Engineering Ltd., Feb 2023.
12. As part of the consulting team supporting the proposed development, I am also aware of and have reviewed the water resource related policies of the Provincial Policy Statement, the County of Lanark Official Plan and the Town of Perth Official Plan. However, I rely on the opinions of other experts with respect to the interpretation and application of those policies from a land use planning perspective.

Issues

13. The issues that I will address in this witness statement include the following:

Issue 5a ix. Does the Application conform to the policies, purpose and intent of the Town of Perth Official Plan... Storm Water Management and Drainage?

Issue 10. Can the proposed development be accommodated with appropriately sized and located stormwater infrastructure that avoids potential impacts on the Tay River and Grant's Creek floodplains?

Issue 11. Has the floodplain been properly delineated to allow an assessment of possible natural hazard constraints on the proposed development?

Issue 12. Have the potential impacts of the Tay River and Grant's Creek floodplains been adequately addressed for the proposed subdivision?

Summary Intended Expert Evidence

Issue 5a *Does the Application conform to the policies, purpose and intent of the Town of Perth Official Plan (the “Official Plan”)?*

a. The application of the conformity test will consider, but not be limited to the following policies of the Town of Perth Official Plan:

ix. 5.3 Storm Water Management and Drainage;

C) The principles which Council intends to utilize in its approach to storm-water management are enumerated as follows:

1. That natural hydrological characteristics are maintained, and where possible, enhanced as the means to protecting the base flow of watercourses.

14. In my opinion, the pre-development drainage area to both the Tay River and Grants Creek has been delineated based on the latest available topographic data. The delineation of the existing drainage divide was provided to DSEL to guide the stormwater management design of the subject development and to ensure that the post-development drainage area will match the existing drainage divide to the greatest extent possible. Having reviewed the stormwater management design being proposed by DSEL, I am satisfied that the proposed design will achieve the intent of the above-noted policy.

15. JFSA in association with Kilgour & Associates Ltd, GEMTEC Consulting Engineers & Scientists and DSEL, in March 2023, collectively authored the “Grants Creek Wetland Integrated Hydrologic Impact Assessment” Report, which investigated the proposed development impacts on the Grants Creek Wetland from a servicing, surface water, groundwater and biological perspective. This report outlined potential impacts and mitigating measures and included considerations for the site's stormwater management. It was concluded that:

SWM and LID measures will be designed to closely reflect pre-development inflow locations and SWM outlets will be designed to replicate outflow locations under pre-development conditions.

16. I continue to support the above-noted conclusion.

Issue 5a Does the Application conform to the policies, purpose and intent of the Town of Perth Official Plan (the “Official Plan”)?

a. The application of the conformity test will consider, but not be limited to the following policies of the Town of Perth Official Plan:

ix. 5.3 Storm Water Management and Drainage;

C) The principles which Council intends to utilize in its approach to storm-water management are enumerated as follows:

- That proposed development will not result in increased downstream flooding or erosion or cause adverse effects on receiving waters by appropriate management of storm-water volumes and contaminant loading.

- That a sustainable environmental approach is utilized in protecting water resources

...

D) It is the intent of Council to incorporate storm water management controls into the development review and approval process. Proponents of development projects will be required to plan for and undertake storm water management which complies with the above principles as well as any master drainage plan. This may require a sub-watershed management plan for large tracts of land or a storm water site management plan. Proponents may utilize best management practices where they are consistent with and will achieve the Town's water quality and quantity targets.

17. As part of the JFSA February 2023 report titled “Caivan Perth Development - Hydrologic and Hydraulic Conditions Report”, detailed existing and post-development hydrologic models were developed.

18. The stormwater management ponds (or “SWM” ponds) for the proposed development were sized to ensure no increases in peak flows to either Grants Creek or the Tay River for the various return periods. Tables 6.3A and 6.3B of the JFSA “Caivan Perth Development - Hydrologic and Hydraulic Conditions Report” outline the peak flows and required storage volumes for each of the SWM facilities. The peak flows and storage volumes are based on detailed hydrologic model simulations (SWMHYMO), which are outlined in Appendix E of the same report. This method of hydrologic modelling is commonly used throughout Canada for preliminary SWM pond sizing and has consistently demonstrated its reliability in determining required volumes.

19. The proposed SWM ponds were also sized to provide quality treatment (per the MECP's Stormwater Planning and Design Manual) to ensure that there will be no effect on contaminate loading to the receiving water courses. Section 6.11 of the JFSA "Caivan Perth Development - Hydrologic and Hydraulic Conditions Report" outlines the active storage volumes required for the three (3) SWM ponds to meet MECP quality control requirements. The SWM pond permanent pool volume configurations will be determined at the detailed design stage to ensure that an 80% Total Suspended Solids (TSS) Removal rate or greater will be achieved per MECP design guidelines. I defer to the evidence of DSEL on the required details of the intended design.
20. JFSA's analysis also considered two portions of land within the development where runoff peak flows would not be attenuated via SWM facilities and will instead be treated by proposed oil and grit separator (OGS) units. These areas will incorporate a runoff treatment train approach, which will consist of roof leaders to grassed areas, dry swales, catch basin shields and perforated pipe CB infiltration systems leading to an OGS unit before discharging to local surface water features. This treatment train system will provide quality treatment in excess of 80% of total suspended solid removal, which is the highest level of total suspended solid removal specified in the MECP guidelines. Once again, I defer to the evidence of DSEL on the required details of the intended design.
21. At present, I am not aware of a similar treatment train approach that is already approved or implemented in the town of Perth or Lanark County, but similar treatment train designs have been approved and are currently in construction in the City of Ottawa. In my opinion, the proposed treatment train approach is consistent with MECP's current policies and approaches to stormwater management.
22. Based on the foregoing, it is my opinion that the deployment of the SWM strategy developed by DSEL will result in a development that meets the requirements of the above-noted policies in terms of stormwater quantity and quality requirements. It is also my opinion that a sustainable environmental approach, in accordance with MECP Design Manual guidance, was utilized in the design of the subject project to ensure the protection of relevant water resources.

Issue 10 *Can the proposed development be accommodated with appropriately sized and located stormwater infrastructure that avoids potential impacts on the Tay River and Grant's Creek floodplains?*

23. Tables 6.3A and 6.3B from the JFSA "Caivan Perth Development - Hydrologic and Hydraulic Conditions Report" outline the required volumes for each of the respective SWM facilities proposed and the associated peak flows to each watercourse. These required volumes were provided to DSEL to determine the block sizes required to ensure the necessary onsite storage volumes are achieved.
24. Tables 6.3A and 6.3B from the JFSA "Caivan Perth Development - Hydrologic and Hydraulic Conditions Report" also show that the total peak flows from the proposed development to both the Tay River and Grants Creek will either match or be less than pre-development conditions for 2-year to 100-year rainfall events .
25. As peak flows will either match or be less than pre-development conditions, it is my opinion that the runoff from the development will not negatively impact the Tay River or Grants Creek floodplains.
26. The proposed locations of the SWM facilities were developed in coordination with DSEL to ensure that the proposed development drainage areas and SWM facility locations closely reflect the existing drainage patterns of the site. It is accordingly recommended that three small SWM ponds be implemented across the site instead of one large SWM pond to better respect the drainage divide between the two watercourses.
27. I am aware that the proposed draft plan of subdivision has been modified since the date of JFSA's original reports. I have reviewed these modifications in conjunction with DSEL. While the modifications will affect the amount of drainage captured by individual SWM facilities located on the site, these changes are not, in my opinion, of significant magnitude to affect the conclusions reached by JFSA as outlined in our original reports. Updating memoranda to confirm this opinion can be prepared and submitted to the municipal authority as a condition of draft approval if formalized updates are desired to be on file.

Issue 11. *Has the floodplain been properly delineated to allow an assessment of possible natural hazard constraints on the proposed development?*

28. On May 30, 2024, JFSA issued a new memo titled “Perth Golf Course – Proposed Floodplain Mapping Amendment”, which mapped the floodplain extents based on the latest available topographic data from 2022 using GIS software. JFSA field staff also went on-site on May 9th, 2024, with an RTK GPS unit to take topographic survey points around the site to ground-truth the latest LiDAR and confirmed that the topographic data used for this mapping met the accuracy requirements for floodplain mapping as specified by the MNRF.
29. The floodplain mapping of the proposed development area has been re-delineated based on the latest available (field-verified) topographic data and outlined in the JFSA memo titled “Perth Golf Course – Proposed Floodplain Mapping Amendment Memo” attached to this witness statement as Appendix “D”.
30. Maps outlining the JFSA 2024 and the RVCA regulatory floodplain extents have been provided in Figures 1 and 2 of the JFSA May 31, 2024, Perth Golf Course – Proposed Floodplain Mapping Amendment Memo
31. The update in floodplain mapping results in a 2.8% reduction in floodplain area on the subject lands. The RVCA Regulatory Boundary occupies 79.21 ha of the site while the JFSA 2024 updated mapping occupies 76.99 ha of the site.
32. At the time of drafting this witness statement, RVCA has not provided feedback on JFSA’s Proposed Floodplain Mapping Amendment Memo.
33. It is my opinion, based on the work undertaken by JFSA in support of the Proposed Floodplain Mapping Amendment Memo, that the floodplain as delineated by JFSA accurately reflects the limits of the floodplain and accordingly identifies the limits of the natural hazard for the purposes of development.
34. I defer to the evidence of Ms. Chandler, a qualified land use planner, with respect to the interplay between the RVCA’s process for updating the floodplain and issuing permits for on-site floodplain alterations (i.e. cut/fill) pursuant to the *Conservation Authorities Act* and the *Planning Act* instruments that are now before the Tribunal on this appeal.

Issue 12. *Have the potential impacts of the Tay River and Grant's Creek floodplains been adequately addressed for the proposed subdivision?*

35. In my opinion, the on-site Grants Creek and Tay River floodplain limits have been accurately delineated by JFSA. However, in order to proceed with lot creation as contemplated by the currently proposed draft plan, a balanced cut-fill is being proposed to regularize the boundary between the lot fabric and constraint lands.
36. DSEL is preparing a detailed grading plan of the proposed cut-fill that balances the placement and removal of fill in accordance with the local conservation authorities' regulatory policies. A detailed hydraulic analysis of the proposed balanced cut-fill grading will also be completed by JFSA, to ensure that the proposed cut-fill work for the site will not adversely impact either Grants Creek or the Tay River's floodplain elevations or riparian storage volumes.
37. The above-summarized work will be submitted to RVCA under O. Reg 41/24 and will be evaluated by RVCA against their local regulatory policies. No work within the regulatory floodplain can or will be undertaken unless and until such work is approved by the RVCA under the *Conservation Authorities Act*.
38. As noted above, I defer to the evidence of Ms. Chandler with respect to how the above-summarized cut/fill process can be accounted for and/or secured through the *Planning Act* instruments now before the Tribunal

Conclusions and Recommendations to the Tribunal

39. The proposed development can be accommodated with appropriately sized and located stormwater infrastructure that avoids potential impacts on the Tay River and Grant's Creek floodplains.
40. Stormwater quantity attenuation and quality treatment can be achieved for the site with the use of SWM ponds and a treatment train approach. I defer to the expertise of DSEL with respect to the on-site design details for these SWM facilities.
41. An updated floodplain delineation has been undertaken for this site based on the latest available topographic information to ensure an accurate assessment of the natural hazard limits and accordant constraint lines for the proposed development. Updates to the

floodplain delineation will ultimately be determined by the RVCA outside of the current *Planning Act* appeals.

42. A detailed cut-fill grading design will be completed to ensure all proposed development lands will be out of the floodplain prior to lot creation and development. A detailed hydraulic analysis will also be completed to ensure that the proposed cut/fill will not have an adverse impact on existing flood elevations on both the Tay River and Grants Creek. I defer to the expertise of Ms. Chandler with respect to how best to implement/secure this process in the context of the current *Planning Act* appeals.
43. The recent changes to the draft plan do not affect the fundamental conclusions regarding stormwater management outlined in this witness statement.
44. I will be in attendance before the Tribunal to answer any relevant questions concerning the above-mentioned Reports drafted by JFSA, the contents of this WS and the contents of any WS or other evidence filed by parties opposite

APPENDIX A



EDUCATION

- Bachelor in Applied Sciences (B.A.Sc.Eng.), University of Ottawa, 1979-83
- Masters in Engineering (M.Eng., Water Resources) University of Ottawa, 1987-1992 (part time)

LANGUAGES

- English
- French

EMPLOYMENT

- 2024-present, JFSA Canada Inc.
- 1993-2024, J.F. Sabourin and Associates Inc.
- 1987-1993, Paul Wisner & Associates - Water Resources
- 1986-1987, University of Ottawa - IMPSWM Research Group
- 1984-1986, W.Rourke Ltd - General Construction Division
- 1983, University of Ottawa - IMPSWM Research Group
- 1982, (Co-op term) Ontario Hydro - Transmission Line Div.
- 1981, (Co-op term) Parks Canada - Prof. Services Division

MEMBERSHIPS

- Professional Engineer, Professional Engineers of Ontario
- Ingénieur, Ordre des ingénieurs du Québec

POSITION

Director of Water Resources Projects
Director of Water Resources Projects
Associate & Senior project manager
Research assistant coordinator
Project manager and Field Supervisor
Research assistant
Co-op student
Co-op student

PROFILE

Mr. Sabourin graduated in 1983 from the University of Ottawa where he obtained both a Bachelor of Applied Science degree in Civil Engineering and a Master's degree in Engineering (Water Resources). With his numerous years and depth of experience, Mr. Sabourin is considered to be an expert in various aspects of water resources, such as urban hydrology, sewer and river hydraulics, flood plain and flood line mapping, development of computer models, and planning / design of stormwater management systems. As President and Director of Water Resources Projects for JFSA, Mr. Sabourin has participated / coordinated / undertaken over 1500 environmental related projects, with a primary focus on water resources. Mr. Sabourin's practical experience, combined with his research capabilities, exceptional communication and relationship building abilities are the basis for his frequent involvement in the development of policy and guidelines in the water resource arena, as well as regular appointment as a reviewer of reports and documents prepared in support of national, provincial and local initiatives/development. Mr. Sabourin oversees the work of more than 30 employees at JFSA's offices located in Ontario and Quebec, ranging in disciplines including water resources engineers, hydrologists, hydrogeologists, biologists, land planners and technical support staff.

WORK EXPERIENCE:

DEVELOPMENT/ REVIEW OF GUIDELINES AND REPORTS

- **Project manager and main author of a computerized "Selection Tool" for the assessment of urban drainage systems.** Developed with and for The Toronto and Region Conservation Authority, (1999), Toronto, Ontario.
- **Contributor to Impacts and Adaptation of Drainage Systems, Design Methods and Policies -** investigated and quantified the potential effects of climate change on drainage infrastructure and performance of existing stormwater management ponds. Study by Kije Sipi Ltd. in partnership with City of Edmonton, Regional Municipality of Ottawa-Carleton and the Mississippi Valley Conservation

Authority. For Natural Resources Canada, Climate Change Action Fund, 2000 - 2001.

- **Contributor to a Climate Change Action Fund research project**; investigated and quantified the potential effects of climate change on drainage infrastructure and performance of existing stormwater management ponds.
- **Project manager and coordinator of an updated research study on the use of grass swales and perforated pipe drainage systems**. Undertaken in partnership with RMOC, MOEE/SWAMP, City of Ottawa, City of Gloucester, City of Nepean, City of Kanata, and four pipe manufacturers, Ottawa, Ontario.
- **Development of a Hydrologic Model Calibration Guideline**, for the City of Ottawa, Ontario.
- **Guidance for the Evaluation of Water Monitoring Networks for Climate Change Adaptation**, CCME. The project resulted in a reference document (Selected Tools to Evaluate Water Monitoring Networks for Climate Change Adaptation) for non-specialist water managers and climate change adaptation planners. The document was developed to help Canadian federal, provincial and territorial governments determine the suitability of their water monitoring networks to provide the data needed to plan for and to adapt to a changing climate. The document describes proven and practical ways for jurisdictions to set priorities for water monitoring networks for climate change adaptation, and then evaluate the ability of these networks to provide the data needed to support climate change adaptation needs. (August 2010 to January 2011)
- **Updated and developed a Design Guideline and oversaw the development of the GSPP Design Tool for the design and construction of Grass-Swale-Perforated-Pipe-Drainage systems** for the City of Ottawa, Ontario.
- **Update to City of Ottawa SWM Guidelines** to address key issues associated with stormwater management and conveyance in new development projects within the City of Ottawa including: developing flat lands where major system capacity is limited; the impact of increasing imperviousness (due to intensification) and changing design standards on existing and proposed infrastructure and overland flow routes; and the need to plan for climate change. To help visualize how the proposed standard changes, City of Ottawa, Ontario. (May 2017)
- **Initiated an Analysis of Rainfall Drying Time on Common Urban Materials** with RiverLabs in Cornwall, Ontario. Purpose of these investigations was to improve estimates of evaporation by quantitatively measuring the drying time of common anthropogenic surfaces. The project took into consideration temperature, wind speed and relative humidity, all of which impact the rates of evaporation of water on anthropogenic surfaces. (March 2017)
- **Climate Change Adaptation – Water Monitoring Data Requirements and Indicators**, Canadian Council of Ministers of the Environment (CCME). Study which identified the hydrologic parameters that should be measured by water monitoring networks in Canada for climate vulnerability assessment and climate change adaptation planning.
- **National Floodplain Mapping Assessment contributor and reviewer** for report establishing framework for Canadian Government development of floodplain management supporting national documents and guidelines. (2015/16)
- **Review of PIEVC Protocols that deal with stormwater related public infrastructure**, Infrastructure Canada. The purpose of this study was to assess if the current PIEVC protocols can adequately assist a city with the evaluation of the vulnerability of their storm water infrastructure to climate change.
- **Review of Low Impact Development Stormwater Management Guidance Manual** for Ministry of Environment and Climate Change Low (2017/18)
- Designer, Project Manager and Coordinator of development and delivery of **Flood Preparation and Response Tool** for City of Ottawa (2018/19)
- Reviewer of **Risk Return on Investment Tool Technical Manual** for Credit Valley Conservation Authority and Risk Sciences International (2019 ongoing).
- Review of **Floodplain Mapping Policies for Future Developments** for the City of Ottawa (2019

ngoing).

FLOOD PLAIN MANAGEMENT / WATERSHED ANALYSIS / URBAN STORMWATER MANAGEMENT

- **Conducted or was senior technical advisor on numerous flood plain, flood line and fill line delineation studies** on numerous watercourses: Cataraqui Creek; Highgate Creek; Grand Canal, La Drize and La Jogne Rivers, Switzerland; Shirley's Brook; Sixteen Mile Creek; Taylor Creek; Upper Thames River; Garry River; Ottawa River near Britannia; Ottawa River near Moussette Beach, Gatineau (formerly Hull), Québec; Gatineau River at Lac Leamy, Gatineau (formerly Hull), Québec; Ottawa River near Clarence Island; Ottawa River near Hamilton Island, Hawkesbury, Ontario; Rideau River at City Hall, Ottawa, Ontario; Tributaries C and D, Brampton, Ontario; Daigneault Creek, Brossard, Québec; Moore Creek, Gatineau, Québec; Humber River within the City of Toronto, Ontario; Jock River, Ontario; Ottawa River near Rockland, Ontario; Village of Richmond Flood Plain Mapping, City of Ottawa, Ontario; Gatineau River Floodplain Mapping between Ottawa River and Farmer Rapids Power Dam, Gatineau, Québec, (March 2012 to June 2013); Marcoux Municipal Drain, North Glengarry; Castor River Tributary preliminary floodplain mapping.
- **Conducted or senior technical advisor to numerous technical reviews and peer reviews of flood plain mapping reports and related work**, including hydrologic and hydraulic analyses and modelling, including: Technical Review of 2015/2016 and 2018-2020 (ongoing) Conservation Authority Flood Plain Mapping Reports for City of Ottawa; Ottawa River Flood Risk Mapping, City of Ottawa; John Boyce and Osgoode Garden Cedar Acres Municipal Drain, South Nation Conservation; Glen Meadows Estates, Town of Arnprior (stormwater management plan, flooding issues); Rideau River (Hogs Back Falls to Ottawa River; Kars to Hogs Back Falls, and Buritts Rapids to Kars); Constance Creek;
- **Senior investigator and one of the authors for a National Floodplain Mapping Assessment**, conducted for Public Safety Canada, Ottawa with MMM Group (lead), to assess the current status of flood plain mapping in Canada (2013/2014).
- Participated in several **watershed analyses and Master Drainage Plans (MDP)**: Ingleside; Area 1A, Markham; Barcelona (for 1992 Olympics), Spain; Riviere Milette, Trois-Riviere; Le Grand Canal, Switzerland; La Drize, Switzerland; Bath Creek, Bath; Relief sewer analysis, Hawkesbury; Riverbend Road Neighborhoods, Edmonton; Exhibition Creek, Markham; MGS Lands, Markham; Garry River, Alexandria; Moore Creek, Aylmer-Hull; Moose Creek, Casselman; Daigneault Creek, Brossard; Moreau Creek, Gatineau; Jock River, Ottawa; Moore Creek, Gatineau.
- **Designed Stormwater Management Plans** for various types of development: Aldermac Orphan Mine Site, Val d'Or; Cardinal Trail 2, Ottawa; Scugog Island Casino, Scugog; Zellers Property, Hawkesbury; Cardinal Trail 3, Ottawa; Le Plateau de la Capitale, Hull; Lazyboy, Innes Road, Ottawa; Secteur C, Ville de Brossard; Cardinal Trail 6, Ottawa; Vales of Castlemoore South Subdivision, Brampton; Tumbridge Subdivision Phase 1, Brampton; Forest Ridge Infill Development, Ottawa; Hull Armories, Gatineau; Morris Village, Rockland; Moodie Drive Townhouse Infill Development, Ottawa; Cambrian Heights Phase V, Ottawa; Corvenelli Development, Russell; Shomberg Subdivision, King Township.
- **Designed and built a physical river and town model** to determine flood water levels in conjunction with a mathematical river model. Both models were verified against measured flood data and alternatives to reduce risks of flooding were determined, St. Mary's, Ontario.
- **Dispersion and assimilative studies** undertaken for a proposed causeway on the Ottawa River: Clarence Island, Clarence-Thurso; for a proposed sewage treatment outfall on the St-Lawrence River: Ingleside, Ontario.
- **Developed Infiltration / Inflow models** for the analysis of sanitary sewer systems: RMOC, Ottawa; City of St. Therese, Québec; City of Edmonton, Alberta.
- **Developed Stormwater Management (SWM) design criteria** for on-site detention in Markham, Ontario.

- **Water budget analyses** prepared for Summerside Wetland in London, Ontario; Mattamy Staines Subdivision Morningside Heights Community, Toronto, Ontario.
- **Overseeing surface water component of water budget analysis** for East Urban Community Mixed Use Development and watercourse assessment and feature specific water budget analysis for the proposed Fernbank Quarry, Ottawa, Ontario
- **Conducted flood damage estimations** from high river water levels and wind effects (wave + setup). Potential flood damage reduction measures were also identified. Lac Deschenes, Ottawa, Ontario.
- **Senior technical advisor for Hydrologic and Hydraulic Assessment**, Chamberland Street, City of Clarence-Rockland: assessment of flooding potential.
- **Collaboration with fluvial geomorphologist** on the Greens Creek Watershed: Integrated Fluvial Geomorphology and Hydrological Study in which data on the watershed's existing conditions were gathered from background information, field investigations, and hydrological analysis and used with the geomorphological assessment to advance understanding of the hydrological functioning of the watershed and subwatersheds, for National Capital Commission, Ottawa, Ontario
- Participated to the **development of the Stormwater Management Strategy**, Phase 1 - Understanding the System, for the City of Ottawa, Ontario.
- **Undertook subwatershed analysis** to evaluate the SWM needs for the development of a site by the Department of National Defence, in the Ottawa, Ontario.
- **Stormwater Management Planning and Design** for Hunt Club Road Extension from Hawthorne Road to new MTO Interchange on Hwy 417, City of Ottawa, Ontario.
- **Hydrology/hydraulic components** of Pinecrest Creek Restoration Plan: Integrating Fluvial Geomorphology, Hydrology and Ecology, for National Capital Commission, Ottawa, Ontario
- **Pinecrest Creek/Westboro Stormwater Management Retrofit Study, Pinecrest/Centerpointe Stormwater Management Criteria Study, and SWM Guidelines for Pinecrest Creek/Westboro Area (in progress), and the Feasibility Study** for the Implementation of SWM Retrofit Measures on City Owned Properties using LID measures, for City of Ottawa, Ontario.
- **Preliminary SWM plan, pond design, floodplain analysis and channel realignment** for Richmond Village Development (in progress), for DSEL, City of Ottawa., Ontario.
- **Westboro Lot Level Pilot Project**, provision of technical assistance to Carleton University engineering students preparing preliminary modelling to design and assess lot level SWM retrofit project, for City of Ottawa, Ontario
- **SWM Facility Feasibility Study** for a Surface Stormwater Management Facility on NCC Lands at Baseline Road and Woodroffe Ave, for City of Ottawa, Ontario.
- Monahan Drain Constructed Wetland, **preparation of Design Brief for the Reconstruction of the Monahan Drain Constructed Wetland** and associated studies and retrofit, for DSEL, Ottawa, Ontario.
- Monahan Drain Constructed **Wetland, Cell 1 model update**, for the City of Ottawa, Ontario.
- Feedmill Creek **SWM Criteria Study**, for City of Ottawa, Ontario
- **Pinecrest Creek Cumulative Impacts Study** (Morrison Hershfield Ltd. for City of Ottawa, Ontario)
- **Technical advisor in the design of a retrofit SWM pond** at the head of Pinecrest Creek, with Morrison Hershfield Ltd. for the City of Ottawa.
- **Hydrodynamic Analysis** of the Lower Reach of the Jock River, for Caivan Properties, in Ottawa, Ontario (2017 to 2019).
- **Lead engineer in a 2D Hydraulic analysis** of McKinnons Creek floodplain analysis, in Ottawa, Ontario (September 2017).
- Technical advisor for the **functional designs for permeable pavement demonstration projects**, LIDs for the City of Ottawa. (2018-2019)
- **Lead engineer and designer of a Flood Preparedness 3D Visual Tool** to manage emergency

responses during flood periods on the Ottawa River and Rideau River, City of Ottawa, 2019. The tool was effectively used by the City and the Canadian military during the 2019 flood.

- **Lead water resources engineer** for the proposed Tewin development project in the east end of Ottawa. The work covered, review of watershed modelling and floodplain analysis, water budget/ balance analysis, rainfall and flow monitoring, preliminary evaluation of quantity/ quality and erosion control requirements, and evaluation of LIDs, (project is ongoing).

CONCEPTUAL DESIGN

- **Prepared preliminary and detailed designs of stormwater quantity /quality pond:** JML Subdivision, Alexandria, Ontario; Secteur C (5 ponds), Brossard, Québec; Ruisseau Leamy, Hull, Québec; Rollin Subdivision, Vars; Cambrian Heights Phase V, Ottawa; Le Breton Flats Development, Ottawa; Morris Village, Rockland; Mayfield West Community (11 ponds), Town of Caledon; Quartier des Bois Subdivision, Casselman; Tranquility West Subdivision, Town of Richmond Hill; Mattamy on Rouge Subdivision, City of Toronto; Timber Trails Phase II, City of Pickering; Lafèche Environmental Landfill Site (200 ha), Casselman; Deglos Landfill Site (60 ha), St. Lucia (UK); Le Plateau de la Capitale (4 ponds), Gatineau;
- **Participated in several trunk sewer designs:** Lynnwood Village, Ottawa; Hawskbury, Ontario; Markham, Ontario; Brossard, Québec.
- **Developed a model for the design of Grass Swale Perforated Pipe drainage systems:** MOE research study.
- **Designed strip marshes within an existing drainage system** to reduce nutrient levels in contaminated stormwater: Maitland, Ontario.
- **Prepared preliminary and detailed design of new storm sewers** for Crestview Road: Ottawa, Ontario.
- **Analysed the initial and prepared final drainage design** for the Earl Armstrong Park and Ride facility in Ottawa, Ontario; in order to meet previously set target flows rates, the use of underground storage and surface storage had to be incorporated in the design requiring extensive hydraulic analyses; in Ottawa, Ontario.
- **Oversaw the hydrologic and hydraulic analyses** required to design the necessary underground storage and surface storage for the stormwater management for the Woodroffe Ave. (Chapman Hills) Park and Ride facility in Ottawa, Ontario.
- **Conceptual Drainage Design**, Cardinal Creek Village, for DSEL, in Ottawa, Ontario.
- Participated to the **design of a permeable pavement demonstration project** for parking rehabilitated projects with the City of Ottawa, 2019-21.

SEWER INFRASTRUCTURE ANALYSES AND UPGRADE / I-I ANALYSES / MODEL DEVELOPMENT & CALIBRATION / CSO ANALYSES

- **Infiltration/Inflow analyses** in sanitary sewers using measured data and calibrated mathematical models: Alta-Vista, Ottawa; Bathurst & Wilson, North York; RMOC, Ottawa, Ontario
- Undertook an **Infiltration/Inflow and CSO Modelling Study** for the upgrade of a treatment plant, Ste-Thérèse and Blainville, North of Montréal, Québec.
- **CSO Modelling and Infiltration/Inflow Study** for the upgrade of a treatment plant, Ste-Thérèse and Blainville, North of Montréal, Québec.
- **Analyses of flow monitoring data and calibration: of a complex hydrologic/hydraulic I/I model**, RMOC; of a partially and combined sewer system in the North Rosedale and Moore Park districts, City of Toronto, Ontario.
- **Sewer Rehabilitation and Sewer Hydraulic study**, Town of Hawkesbury; Crestview Road, Ottawa;

Fallowfield Road, Ottawa.

- **Development of a Variable Diurnal DWF model**, City of Edmonton, Alberta. This model has been used successfully for the City of Toronto, and in the Ottawa area.
- **Infiltration/Inflow Analysis, Sewer Hydraulic, Model Calibration, and Sewer Rehabilitation Study** to identify feasible and cost-effective solutions to reduce the occurrence of basement flooding, City of Gatineau, Québec.
- **Analysis of flow data and combined sewer overflows to calibrate a hydrologic model** which was then used to evaluate the effects of various flow reduction measures: City of Toronto, Ontario.
- **Servicing study** for the Sector C in Brossard, Québec.
- **Sewer Rehabilitation, CSO Reduction and Sewer Hydraulic study** for the Old Ottawa East area, Ottawa, Ontario.
- **Estimation of sewer flows (sanitary and storm) for new storm sewers** and partially separated sewers for Argyle Street and Park Avenue Sewer Rehabilitation, Ottawa, Ontario.
- **SWMHYMO model calibration and validation, as well as PCSWMM model validation for the drainage area to the Beaver Pond in the Kanata Lakes subdivision (2018-2020).**
- **SWMHYMO model calibration for the Shirley's Brook subwatershed in Kanata, (2021)**

WATER QUALITY & EROSION CONTROL

- **Development of a methodology for the assessment of chloride impacts** on receiving waters from several alternative snow dump sites in the City of Ottawa (formerly Ottawa-Carleton Regional Municipality), Ontario. The method is based on a regional analysis of monthly low flows and background concentrations. Based on snowmelt runoff rates from snow dumps and potential for dilution in the receiving water bodies, impact indices were determined.
- **Undertook flow/velocity/quality measurements and dispersion analyses** for the design of a treatment plant outfall in the St Lawrence River, Ingleside, Ontario.
- **Conducted erosion protection studies and analyses of the effects of urbanization on erosion:** Caledon, Ontario; Taylor Creek, Cumberland; Shirley's Brook, Kanata, Ontario.
- **Undertook flow/velocity measurements and shear stress analyses** for the design of a beach protection scheme: Hull, Québec.
- **Undertook field measurements and conducted a detailed water budget analysis** of Lac Leamy to investigate the possibility of opening it to the Gatineau and Ottawa rivers: Hull, Québec.
- **Undertook a water budget analysis of an existing wetland** to identify impacts of a proposed future development where a portion of the future drainage area would be diverted: London, Ontario.
- **Developed a stormwater quality management plan for an industrial site** where high levels of nitrogen-based compounds are contaminating nearby surface water bodies: Maitland, Ontario.
- **Participated to various Source Protection Surface Water Vulnerability Assessment studies** for the towns/cities of Hawkesbury, Rockland, Plantagenet, Wendover, Perth, Smiths Falls, Carleton Place, Ontario.
- **Hydrologic and hydraulic assessment along Mud Creek to support a Fluvial Geomorphic assessment** for the Mud Creek Slope Stability Study, with Golder Associates for City of Ottawa, Ontario.

CONSTRUCTION, FIELD WORK AND MONITORING

- **In charge of the construction supervision of a sewage lagoon** for the Department of National Defence. Carp, Ontario.

- **In charge of the construction supervision of an underground helicopter garage** at a Department of National Defence base (Defenbunker), Carp, Ontario.
- **Project engineer and general site manager of a \$ 2 million construction project** for the City of Ottawa. Work included quantity estimates, orders, dealings with architects, engineers, unions and all subtrades. All aspects of work, schedules and meetings were coordinated and progress billings, invoicing, time sheets, etc. were also accounted for City of Ottawa, Ontario.
- **Coordinated and participated in field investigations to obtain water quality and flow measurement quantities:** St.Lawrence River, Ingleside; Ottawa River, Moussette Beach-Hull; Jock River, Goulbourn Township, Ontario
- **Coordinated and participated in various sewer flow and water quality measurements:** Amberwood sub-division, Nepean; Bravard subdivision, Manotick; Pineglen subdivision, Nepean; Heart's Desire subdivision, Nepean; Landfill site, Casselman, Ontario..
- **Coordinated and participated in infiltration capacities and groundwater monitoring:** Pineglen subdivision, Nepean; Heart's Desire subdivision, Nepean.
- **Coordinated and participated in a GPS survey of approximately 2 km of a creek to be modelled with HEC-RAS,** Ruisseau Moore, Hull, Québec.
- **Coordinated and oversaw the gathering of ADCP flow and continuous water level monitoring on:** Poole Creek (2008-2009) at the Amberwood Golf Course in Stittsville, Ontario; Dickinson Creek (2009) at the Nation Golf Club, in Curran, Ontario; and Greens Creek (2008), in Ottawa, Ontario.
- **Coordinated and oversaw the gathering of ADCP flow measurements on:** Pinecrest Creek (2009), in Ottawa, Ontario.; Carp River (2008-2009) at the Kinburn Gauge, in Ottawa, Ontario; Cardinal Creek (2012-2013) in Ottawa, Ontario; Jock River (2017), in Ottawa, Ontario.
- **Coordinated and oversaw the gathering of continuous rainfall, storm sewer flow and pond water level data** in and around the Kanata Lakes subdivision and Kanata Lakes Golf Course. The data was used to calibrate and validate hydrologic models (2018-19).
- **Coordinated and oversaw the undertaking of surface infiltration tests at various locations;** Amberwood Village (Ottawa, Ontario), Westport (Ontario), Barrhaven (Ottawa, Ontario), Stittsville wetland (Ottawa, Ontario), 2018-21.

EXPERT WITNESS, REVIEW and OMB/ OLT HEARINGS

Mr. Sabourin has conducted several reviews, analyses and acted as an expert witness in several project of legal cases with water resources and hydrological components.

- July 1987 storm and flooding in the City of Montréal, Québec (analysis of rainfall and modelling analysis)
- April 1994 flooding on the Lapèche River, Québec (analysis of rainfall and modelling analysis)
- August 1996 storm and flooding in the Ottawa region, Ontario (analysis of rainfall and radar data, and modelling)
- August 1996 storm and flooding in the City of Gatineau, Québec (analysis of rainfall and radar data, and modelling)
- June 1997 flooding in the Village of l'Orignal, Ontario (modelling analysis)
- July 1997 storm and flooding in the City of St-Hubert, Québec (analysis of rainfall and radar data)
- July 1997 storm and flooding in the City of Chambly, Québec (analysis of rainfall and radar data)
- June 1998 storm and flooding in the City of Gatineau, Québec (sewer modelling analysis)
- January 1999 flooding on the Salmon River, New York (interpretation of data)
- June 2000 storm and flooding in the City of Gatineau, Québec (analysis of rainfall and radar data, and sewer modelling analysis)

- June 2001 storm in the City of St-Hubert, Québec (analysis of rainfall and radar data)
- 2001 to 2003 rainfall events in St. Lucia, UK (analysis of rainfall data)
- January 2003 flooding event at the Hull Casino, Québec (analysis and interpretation of event)
- December 2003; served as expert witness in drainage related matters in an OMB hearing for FrancoScenie.
- July 2004 storm and flooding in the City of Saint-Hyacinthe, Québec (analysis of rainfall and radar data)
- July 2004 storm in the City of Gatineau - Buckingham, Québec (interpretation of rainfall intensities and radar data).
- April 2005 of well contamination in the Town of l'Original, Ontario (analysis and interpretation of event)
- June 2005 storm and flooding event in the City of Shawinigan, Québec (analysis and interpretation of rainfall event using radar data).
- September 2005 storm and flooding event in the City of Québec, Québec (analysis and interpretation of rainfall event).
- August 2006 storm and flooding event in the Cities of Saint-Eustache and des Deux Montagnes, Québec (analysis and interpretation of rainfall event using radar data).
- August 2006 storm and flooding event in the Town of Saintes-Annes-des-Plaines, Québec (analysis and interpretation of rainfall event using radar data).
- June and July 2007 storm and flooding event in the City of Levis, Québec (analysis and interpretation of rainfall event using radar data).
- August 2, 2008, storm and flooding event in City of Montréal, Québec (analysis and interpretation of rainfall event using radar data, liaison with insurance companies and legal representatives). November 2009 onward,
- Progressive flooding of lands in the City of Terrebonne, Québec, following the construction of Highway 640 (continuous hydrologic and hydraulic simulations to determine if changes in drainage patterns caused by the construction of Highway 640 can explain the increase flooding frequency and duration of subject lands, and the presence of anthropic wetlands). Court appearance in the fall of 2014.
- September 2010 storm event in the Cities of Saint-Eustache and Rosemère, Québec (analysis and interpretation of rainfall event using radar data).
- June 23-24, 2012, storm and flooding event in City of Gatineau, Québec (analysis and interpretation of rainfall event using radar data - in progress).
- October 2014, expert witness report for the August 22, 2013, storm event in the City of Drummondville, Québec (analysis and interpretation of rainfall event using radar data).
- April 2015, served as expert witness in drainage related matters in an OMB hearing for the development of the Chenier Lands, in Ottawa.
- October 2015, served as expert witness in drainage related matters in an OMB hearing for the development of the Bronte Green Lands, in Oakville, Ontario. Summer of 2016 acted as key hydrologic and hydraulic engineer in dealings to settle the case.
- February 2017, retained by both City of Ottawa and homeowner to assess and resolve a perceived drainage problem with the Emerald Estate subdivision, in Ottawa.
- December 2019, second trial (previous one was in 2014) on the impacts of improper design and maintenance of Highway 640 drainage which created wetlands on adjacent private properties in Terrebonne (Qc). Judgment prescribed what drainage infrastructure had to be replaced and constructed within 18 months.
- January, February 2022, Ontario Land Tribunal (OLT), ClubLink Corporation ULC, to permit the redevelopment of the lands (Golf Course) for residential and open space uses.
- September 2022, third trial (previous one was in 2019), as the works that were prescribed in the

previous judgment were not completed, this judgment imposed temporary works that are to be in place before the Spring of 2023 to allow for the drainage of the subject lands around Highway 640.

- January 2023-June 2023, preparation for Ontario Land Tribunal (OLT), retained by Southwest Georgetown Landowners Group Inc., Proposed Official Plan Amendment No. OPA 32. (in progress)
- April 2023-June 2023, retained by the City of Gatineau (Qc), for prepare an expert opinion on presence (natural or anthropogenic) of wetlands within the Forêt Boucher. (in progress)

USE OF APPLIED HYDRAULIC AND HYDROLOGIC MODELS

- **OTTHYMO**: An improved version of HYMO (HYdrologic MOdel) which generates runoff from not only large non-urban basins, but also from urban basins.
- **INTERHYMO/OTTHYMO-89**: The newly expanded version of OTTHYMO.
- **SWMHYMO**: A single event and continuous model based on the principles of HYMO and OTTHYMO. Latest version of model also incorporates the Infiltration / Inflow capabilities of SEWHYMO.
- **QUALHYMO**: Is a unit hydrograph based hydrologic model used to predict the long-term quantity and quality of runoff from rural and urban watersheds. The model can also produce exceedance curves for overflows and pollutants in ponds.
- **SEWHYMO**: A single event and continuous model for the analysis of Infiltration / Inflow and combined sewer overflows
- **HEC-2 / HEC-RAS**: A mathematical river model used to determine water surface profiles for sub-critical and super-critical flow conditions.
- **IMPRAM**: A program for the Improved Rational Method used for sizing storm sewers.
- **CFA_3.1**: Is the Consolidated Frequency Analysis Package from Environment Canada used to generate flood frequency curves fitted to a series of annual floods or levels.
- **STORM**: Is the Storage Treatment Overflow Runoff Model used to predict the long-term quantity and quality of runoff from urban basins.
- **STORMS 2000 and STORM 2010**: A program used for the frequency of observed rainfall events
- **OTTSWMM and DDSWMM**: Is the University of Ottawa's improved version of SWMM (Storm Water Management Model from EPA) which generates runoff from urban basins by analysing both minor (pipes) and major (street) flows.
- **EXTRAN**: Is the EXtended TRANsport model distributed by EPA and used to dynamically analyse flow routing and surcharges in complex sewer systems.
- **HYSTEM/EXTRAN**: is the German version of EXTRAN.
- **SWMM / PC-SWMM / XP-SWMM / XP-STORM**: EPA's Stormwater Management Model.
- **MIDUSS**: Microcomputer Interactive Design of Urban Stormwater Systems, developed by A. Smith et al.

SOFTWARE DEVELOPMENT

Mr. Sabourin has over 30 years of programming experience including the development of numerous in-house programs to improve the company's and client's efficiency. Those programs and/or applications include:

- **IMPRAM**: The Improved Rational Method.
- **TRAINHYD**: A training hydrologic software.
- **FASTHYMO, MINI-OTTHYMO, LUMPHYD, ULTRA**: User friendly versions of the hydrograph commands from OTTHYMO
- **SEWHYMO**: A single event and continuous model for the analysis of Infiltration / Inflow and combined

sewer overflows

- **OTTHYMO-89:** An improved version of the original OTTHYMO hydrologic model
- **SWMHYMO:** A single event and continuous model based on the principles of HYMO, OTTHYMO and QUALHYMO. Latest version of model also incorporates the Infiltration / Inflow capabilities of SEWHYMO.
- **STORMS, STORMS 2000 and STORMS 2010:** Programs to generate design storms, plot hyetographs, generate IDF curves and analyse the return period of an observed rainfall event.
- **PLOTHYD:** A program used to plot hydrographs generated by OTTHYMO-89 or SWMHYMO
- **DVMS for SEWER VIDEOS:** A digital video management system to accelerate the review and improve the use of sewer videos
- **DIG-VT:** Digital-Virtual Tour; a system that integrates the use of digital photos and GPS to allow users to conduct a virtual tour of, for instance a creek, from their computer.
- **Drainage System Selection Tool:** Designed the concept and directed the development of an EXCEL spreadsheet program that allows user to identify drainage system components that are compatible with specific site characteristics, development characteristics and SWM objectives. Based on user selected components, the tool provides approximate capital and operational costs for the drainage system.
- **GSPP:** Directed the development of a software integrated within an EXCEL spreadsheet that allows users to design grass swale and perforated pipe drainage systems.
- **Daily Water Budget Model:** Developed a new computational method to undertake daily computations of water budget based on daily temperature, precipitation, snow accumulation, and daily computed heat index, frost depth, snowmelt, interception, potential evapotranspiration, adjusted evapotranspiration based on daylight hours, infiltration, transpiration, and runoff.

RESEARCH CONTRIBUTIONS

- Was research associate of the IMPSWM program at the University of Ottawa.
- Developed several user friendly micro-computer programs: IMPRAM (Improved Rational Method), TRAINHYD (used to learn hydrologic principles), FASTHYMO, MINI-OTTHYMO, LUMPHYD, ULTRA.
- Contributed to the calibration of OTTHYMO on several watersheds using observed rainfall and runoff measurements.
- Tested, debugged and improved the QUALHYMO model.
- Tested and debugged the EXTRAN model.
- Developed and was main author of the INTERHYMO/OTTHYMO-89 hydrologic model.
- Analysed of real storms and determination of new design storms for the Town of Markham, Ontario.
- Compared of real storms with design storms for the City of Laval, Québec.
- Project manager of an MOE funded research study on the potential use of grass swales and perforated pipe storm drainage systems to control and reduce urban stormwater pollutant loadings, (1991-92).
- Developed and main author of the SEWHYMO infiltration/inflow model for sanitary sewers, (1993-94).
- Conducted a detailed analysis to determine the use of design storms in infiltration/inflow modeling. (M.Eng. Thesis) Developed and main author of the SWMHYMO model, an improved version of the popular OTTHYMO-89 hydrologic model, (1995-99). Project manager and main researcher for a study entitled
- "Evaluation of Roadside Ditches and Other Related Stormwater Management Practices" (1997) conducted for Toronto and Region Conservation Authority, Toronto, Ontario.
- Project manager and principal investigator for a demonstration project, Keep the Rain Out of the Drain, using alternative drainage methods for the City of Toronto, Moore-Park & Rosedale areas (1997-98),

Toronto, Ontario.

- Project manager of an investigative study to measure the drying time of several types of surfaces / materials such as asphalt, concrete, roof singles, pavers, etc. (2017).
- Developed a methodology and managed the continuous monitoring of rainfall precipitation in a forest setting to determine / observe the process of wetting losses caused by trees and their leaves (2018 and 2019).
- Developed a methodology and managed the field work for the continuous monitoring of rainfall precipitation, wetting losses, retention, and evaporation of various soils, materials, and plants (2020).

LECTURES, PUBLICATIONS AND PAPERS

Mr. Sabourin has lectured at several Urban Hydrology short courses in Markham, Ontario; Mississauga, Ontario; London, Ontario; Edmonton, Alberta; Calgary, Alberta; Laval, Québec; Vancouver, British Columbia; Toronto, Ontario. He is also the author/co-author of several published papers which were presented at conferences across Canada and abroad:

- "Comparison of the IMPRAM Model (Improved Rational Method) with other Hydrologic Models."
- "Rule of Simplified Micro-Computer Models as Parts of Multi-level Hydrological Packages," Stormwater and Water Quality Management Modelling and SWM Users Group Meeting, Toronto, Sept. 17-18, 1986.
- "Teaching Models for Simulation and Real Time Control Operation of Urban Drainage Systems."
- "Use of Physical and Mathematical Modelling for Bridge Hydraulics," XXII Congress IAHR, Lausanne, Switzerland, 1987.
- "Development of a Multi-Level Package of Stormwater Management Models."
- "Design of Windpower Pumped Water Storage Reservoirs by Microcomputers," Microcomputers and Civil Engineering, Orlando, Florida, November, 1987.
- "Joint Use of the HEC-2 Model and a Physical Model for Floodline Delineation Upstream of a Bridge," Can. J. Civ. Eng., Vol 16 No 1, 1989.
- "The Use of Grass Swales and Perforated Pipes as a Stormwater Quality Control Alternative," Good Roads Conference, Penticton, B.C. (1991) and Toronto, Ont. (1993).
- "Mechanics of Scour in the Vicinity of Bridge Piers", Working paper, University of Ottawa, December, 1987.
- "Performance Review of Grass Swale Perforated Storm Sewer Systems", Annual Conference - Water Environment Association of Ontario, Windsor, Ontario, April 1994.
- "SEWHYMO-4, A Model for the Analysis of Infiltration / Inflow in Sanitary Sewer Systems", unpublished.
- "Mathematical Simulation of Wet Weather Processes in a Sewerage System", WEFTEC '94, Water Environment Federation 67th Annual Conference & Exposition, Chicago, Illinois, October 15-16, 1994.
- "Developing and Using a Dry Weather Flow Model for Sewerage Systems Analysis", 1995 Annual Conference of the Canadian Society for Civil Engineering, June 1-3, 1995, Ottawa, Ontario.
- "The use of Design Storms for Infiltration / Inflow Modelling", 1995 Annual Conference of the Canadian Society for Civil Engineering, June 1-3, 1995, Ottawa, Ontario.
- "Evaluation of Roadside Ditches and Other Related Stormwater Management Practices", Controlling Stormwater: 2001 and Beyond, October 16-17, 1996, Burlington, Ontario.
- "Selection of an Optimum Road Drainage System", SWAMP, Stormwater/CSO Technology Transfer Conference, February 23-24, 1998, Toronto, Ontario.
- Taught the 4th university course CVG 45114 "Hydraulics of Water Supply and Sewer Systems" at the University of Ottawa, 2005.

- “Grass Swale and Perforated Pipe Drainage Systems a 20 year Performance Evaluation”; Credit Valley Conservation Workshop on “Lessons Learned from Impacting LID in Northern U.S. and Canada”, Oct. 7th 2008.
- “Grass Swale and Perforated Pipe Drainage Systems a 20 year Performance Evaluation”; Water2010 International Conference, Québec City, July 2010.
- “Grass Swale and Perforated Pipe Drainage Systems a 20 year Performance Evaluation”; WEFTEC 2011. 84th Annual Water Environment Federation Technical Exhibition and Conference, Los Angeles, USA, October 2011.
- “Measurement of rainfall interception of tree canopies – how we have it wrong, all these years”, Unpublished, June 2020.

APPENDIX B



Ontario
 Ontario Land Tribunal
 Tribunal ontarien de l'aménagement du territoire

Acknowledgment Of Expert's Duty

OLT Case Numbers	Municipality
OLT-23-000534	County of Lanark Town of Perth
OLT-23-000939	
OLT-23-000940	

1. My name is **(Jean-Francois Sabourin)**
 I live at the **(Ottawa)**
 in the **(n/a)**
 in the Province of Ontario.

2. I have been engaged by or on behalf of **Caivan (Perth GC) Limited** to provide evidence in relation to the above-noted Ontario Land Tribunal ('Tribunal') proceedings.

3. I acknowledge that it is my duty to provide evidence in relation to this proceeding as follows:
 - a. to provide opinion evidence that is fair, objective and non-partisan;
 - b. to provide opinion evidence that is related only to matters that are within my area of expertise;
 - c. to provide such additional assistance as the Tribunal may reasonably require, to determine a matter in issue; and
 - d. not to seek or receive assistance or communication, except technical support, while under cross examination, through any means including any electronic means, from any third party, including but not limited to legal counsel or client.

4. I acknowledge that the duty referred to above prevails over any obligation which I may owe to any party by whom or on whose behalf I am engaged.

Date...June 12, 2014.....



Signature

APPENDIX C/

CAIVAN PERTH DEVELOPMENT - HYDROLOGIC AND HYDRAULIC CONDITIONS REPORT

FEBRUARY 2023



Prepared For:

Caivan Communities
3713 Borrisokane Road
Nepean, ON
<Postal Code>
<Phone Number>

Prepared By:

J.F. Sabourin and Associates Inc.
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CAIVAN PERTH DEVELOPMENT - HYDROLOGIC AND HYDRAULIC CONDITIONS REPORT

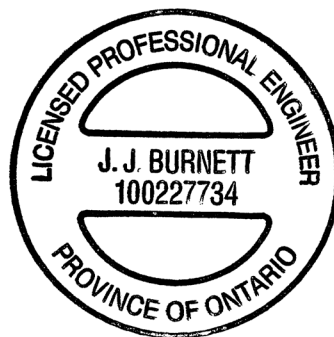
In Perth, Ontario

FEBRUARY 2023

Prepared for:

Caivan Communities

Prepared by:



Jonathon Burnett, B.Eng, P.Eng
(J.F. Sabourin and Associates Inc.)

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- Appendix B – Surface Water Monitoring
- Appendix C – Floodplain Mapping
- Appendix D - Existing Water Budget
- Appendix E – Preliminary SWM pond Sizing

1 Introduction

This Report has been prepared to provide a comprehensive understanding of the current hydrologic and hydraulic conditions of the future residential development site located at 141 Peter Street, Perth, Ontario, referred here on as the “Perth GC development”. The report is intended to serve as a baseline for future studies and design work related to water resources management, floodplain management, and other hydrologic and hydraulic analyses.

Figure 1 below outlines the development site, major watercourses and the Grants Creek wetland. As shown in this figure the development is located between the Tay River and Grants Creek, with the Grants Creek wetland located to the south of the site, additional details regarding the development and drainage areas have been outlined in Section 2 below. The report also covers a range of hydrologic and hydraulic characteristics, which includes an overview of surface water monitoring completed by JFSA in 2022, an updated existing floodplain delineation, a conceptual existing conditions water budget based on continuous hydrologic modelling and preliminary SWM pond sizing for the future development.

Each of the key items addressed in the report has been laid out in individual sections to ensure that the report provides a comprehensive and easy-to-understand overview of each of the components of the hydrologic and hydraulic conditions in the study area.

Figure 1. Site Overview



2 Existing Drainage Area

The following section provides an overview of the various drainage areas surrounding and within the development study area and outlines the land use compositions of each of the major watersheds.

2.1 Development Site

As outlined in **Figure 1** above the site is bound by the Tay River to the north and east of the site and by Grants Creek to the south. Under existing conditions, the Perth development site has a total drainage area of approximately **44.86 ha**, with **22.85 ha** currently draining north to the Tay River and the remaining **22.01 ha** draining south to the nearby wetland and Grants Creek. Currently, the development site is approximately split 50-50 between Grants Creek and the Tay River. The site under existing conditions consists primarily of a golf course with well-maintained lawns, surrounded by irregular forest patches. Refer to **Figure A1** in **Appendix A**, for a visual overview of the study area's pre-development drainage divide.

Under proposed conditions, the Perth development has a total drainage area of approximately **44.86 ha**, with **28.54 ha** draining north to the Tay River with the remaining **16.32 ha** draining south to the nearby wetland and Grants Creek. This is a drainage area adjustment between the two watersheds of **5.69 ha**. Note that efforts have been made to maintain the existing drainage areas within the development site as much as possible with consideration for grading and servicing limitations. Refer to **Figure A2** in **Appendix A**, for a visual overview of the study area's post-development drainage divide.

2.2 Grants Creek

The total existing drainage area of Grants Creek (upstream and surrounding lands - minus the development site) is approximately **9351.78 ha**. A land use summary in **Table 2.1** below shows that land cover within the drainage area is primarily natural features (**60.8%**), agricultural lands (**23.4%**), and open water (**9.3%**). The remainder of the land cover within the watershed is bedrock (**4.8%**) and community/infrastructure (**1.7%**).

Table 2.1 – Grant's Creek - Land Use Summary

Land Cover	Area (ha)	Total Area (ha)	Percentage of Total
Natural (Forest/Meadows/Swamps)	5689.8	9351.78	60.8%
Agricultural	2189.1	9351.78	23.4%
Water	868.3	9351.78	9.3%
Bedrock	447.2	9351.78	4.8%
Community/Infrastructure	157.3	9351.78	1.7%

Under post-developed conditions, the Perth GC Development would decrease the total area of the Grants Creek watershed by around **5.69 ha**, a change of **0.06%**. **Figure A3** provides an overview of the various Grants Creek drainage areas and land use.

Table 2.2 - Grants Creek Wetland Drainage Area Change

Development Conditions	Upstream and Surrounding Area (ha)	Perth GC Development (ha)	Total Area (ha)	Difference	
				(ha)	(%)
Pre-Dev	9351.78	22.01	9373.8	-	-
Post-Dev	9351.78	16.32	9368.1	-5.69	0.06%

2.3 Tay River

The total drainage area of the Tay River watersheds (upstream and surrounding lands – minus the development site) is around **58,382.7 ha**. A land use summary in **Table 2.3** below shows that land cover within the drainage area is primarily natural features (**64.7%**), agricultural lands (**14.3%**), and open water (**13.9%**). The remainder is bedrock (**6.2%**), and community/infrastructure (**0.9%**).

Table 2.3 – Tay River - Land Use Summary

Land Cover	Area (ha)	Total Area (ha)	Percentage of Total
Natural (Forest/Meadows/Swamps)	37799.6	58382.7	64.70%
Agriculture	8325.8	58382.7	14.30%
Water	8131.2	58382.7	13.90%
Bedrock/Sand/Gravel	3594.9	58382.7	6.20%
Community/Infrastructure	531.2	58382.7	0.90%

As shown in **Table 2.4** below, the post-developed Perth GC site would result in an increase of **5.69 ha** to the Tay River Watershed, representing a change in the total drainage area of **0.009%**. **Figure A4** outlines the total drainage area and land use types within the watershed.

Table 2.4 - Tay River Wetland Drainage Area Change

Conditions	Upstream and Surrounding Area (ha)	Perth GC Development (ha)	Total Area (ha)	Difference	
				(ha)	(%)
Pre-Dev	58382.7	22.85	58405.5	-	-
Post-Dev	58382.7	28.54	58411.2	5.69	0.009%

2.4 Summary

This section provided an overview of the various drainage areas around and within the development study area. As discussed above, under existing conditions the Perth development has a total drainage area of approximately **44.86 ha**, with **22.86 ha** currently draining north, while the remaining **22.01 ha** draining south to the nearby wetland and Grants Creek. The development is approximately split 50-50 draining to Grants Creek and the Tay River. The primary land cover of the Grants Creek and Tay River watersheds are natural features, agriculture, and open water.

Under proposed conditions, the Perth GC development has a total drainage area of approximately **44.86 ha**, with **28.54 ha** draining north to the Tay River and **16.32 ha** draining south to the nearby wetland and Grants Creek. The total existing drainage area upstream of and surrounding Grants Creek is **9351.8 ha**. The proposed development would result in an overall decrease of **5.69 ha**, or **0.06%** to the total Grants Creek watershed. The Tay River drainage area would receive an increase of **5.69 ha**, an increase of **0.009%** to the Tay River watershed. Given the size of these watersheds, and the location of the drainage area change (at the confluence of the two watersheds) it is unlikely that this change under post-development conditions will have a quantifiable impact on the hydraulic and hydrologic conditions of the surrounding watercourses.

3 Surface Water Monitoring (2022)

As a part of the Perth GC development, J.F. Sabourin & Associates (JFSA) has been commissioned by Caivan Communities to complete surface water monitoring throughout the subject area. These works are intended to develop a strong understanding of how the watercourses react to various environmental conditions, and how flows and water levels are related at key locations within the surrounding lands. This work included surface water monitoring and precipitation monitoring in the study area from June 2022 to November 2022. The following section briefly outlines the data obtained and conclusions drawn from this 2022 monitoring window.

3.1 Overview

The 2022 monitoring program consisted of 2 level loggers, 1 barometric logger, and 1 rain gauge implemented on and around the site. A level logger was installed on Grants Creek at Glen Tay Road, on the upstream side of the road crossing, to monitor the flow contributions from the upstream drainage area to the Grants Creek wetland. A secondary level logger was installed within the Grants Creek Wetland near the confluence with the Tay River to monitor water levels within the wetland itself. Both the Barometric logger and rain gauge were located within the existing Perth Golf Course site. Refer to **Figure B1** for the monitoring locations from 2022.

3.2 Rainfall

A tipping bucket rain gauge was installed on-site on **June 10, 2022**, until **November 2, 2022**, providing **152 days** of rainfall data. The gauge was placed in a flat, open area on a platform to avoid any interference from nearby vegetation and trees and to withstand large rainstorms and wind without shifting. The gauge was calibrated before installation, and the lip of the funnel was installed level with the surrounding ground. The rain gauge was inspected monthly to ensure that it was level and functioning properly, and the data recorded during that month was downloaded.

Throughout the monitoring period of **June 10, 2022**, until **November 2, 2022**, there was a total of **325.3 mm** of rainfall. Based on the rainfall data acquired during this window 'significant' rainfall events were then identified. For this study, a 'Significant Rainfall Event' was defined as a single event if the total rainfall volume was greater than 5 mm and was followed by at least 12 hours without any additional rainfall. A total of **18** significant rainfall events took place in 2022. The largest event recorded over this duration occurred on July 18, from 09:40 AM to 21:20 PM (duration of 11:40) and had a total rainfall volume of **38.3 mm**. **Table 3.1** provides a full summary of these significant events.

Table 3.1: Significant Rainfall Events, 2022
(Events with more than 5 mm and separated by at least 12 hours of no rain)

Event	Start Date/Time	Finish Date/Time	Duration (Hr : Min)	Total Rainfall (mm)
1	2022-06-21 10:00	2022-06-21 14:40	4:40	6.1
2	2022-06-29 12:10	2022-06-29 17:50	5:40	6.2
3	2022-07-12 13:50	2022-07-12 14:35	0:45	13.1
4	2022-07-18 09:40	2022-07-18 21:20	11:40	38.3
5	2022-07-22 17:10	2022-07-22 17:40	0:30	13.9
6	2022-07-24 21:15	2022-07-25 00:35	3:20	8.4
7	2022-08-07 20:55	2022-08-08 02:10	5:15	17.9
8	2022-08-21 12:20	2022-08-21 17:15	4:55	8.9
9	2022-08-22 14:55	2022-08-22 16:35	1:40	7.4
10	2022-08-22 23:10	2022-08-23 00:30	1:20	21.6
11	2022-08-29 17:25	2022-08-29 18:35	1:10	20.9
12	2022-08-30 13:20	2022-08-30 18:45	5:25	7.1
13	2022-09-03 20:45	2022-09-03 23:00	2:15	10.1
14	2022-09-13 12:00	2022-09-14 06:05	18:05	18.3
15	2022-09-18 11:00	2022-09-19 01:20	14:20	16.8
16	2022-09-19 06:35	2022-09-20 07:50	1:15	24
17	2022-10-13 08:00	2022-10-13 21:15	13:15	8.6
18	2022-10-17 05:20	2022-10-17 19:00	13:40	10.8

The Rainfall-Duration Max Intensity summary for the 2022 collected rainfall has been compared to the IDF curves for Perth (**Table 6.1**) and assessed for various rainfall intervals, with the summary outlined below in **Table 3.2** for this study period. Based on this analysis it was seen that the rainfall intensities observed in 2022 equate to either less than a 2-Year or 5-Year event, depending on the duration observed.

Table 3.2: Rainfall Duration/Max Intensity Summary, 2022

	Duration	Maximum Measured Rainfall Intensity (mm/hr)	Return Periods Based on Ottawa Airport IDF (Years)
2022	5 Minute	93.60	<2-Year
	10 Minute	76.20	<5-Year
	15 Minute	62.00	<5-Year
	30 Minute	31.80	<2-Year
	60 Minute	21.10	<5-Year
	2 Hour	10.80	<2-Year
	6 Hour	4.65	<2-Year
	12 Hour	3.19	<2-Year
	24 Hour	1.60	<2-Year

3.3 Glen Tay Road Crossing

A level logger was placed at the upstream side of Glen Tay Road on Grants Creek. The minimum, maximum and average for both water temperature and depth, as well as the number of days with zero depth readings, are provided in **Table 3.3**. Graphs showing continuous water depth vs. rainfall events are available in **Appendix B**.

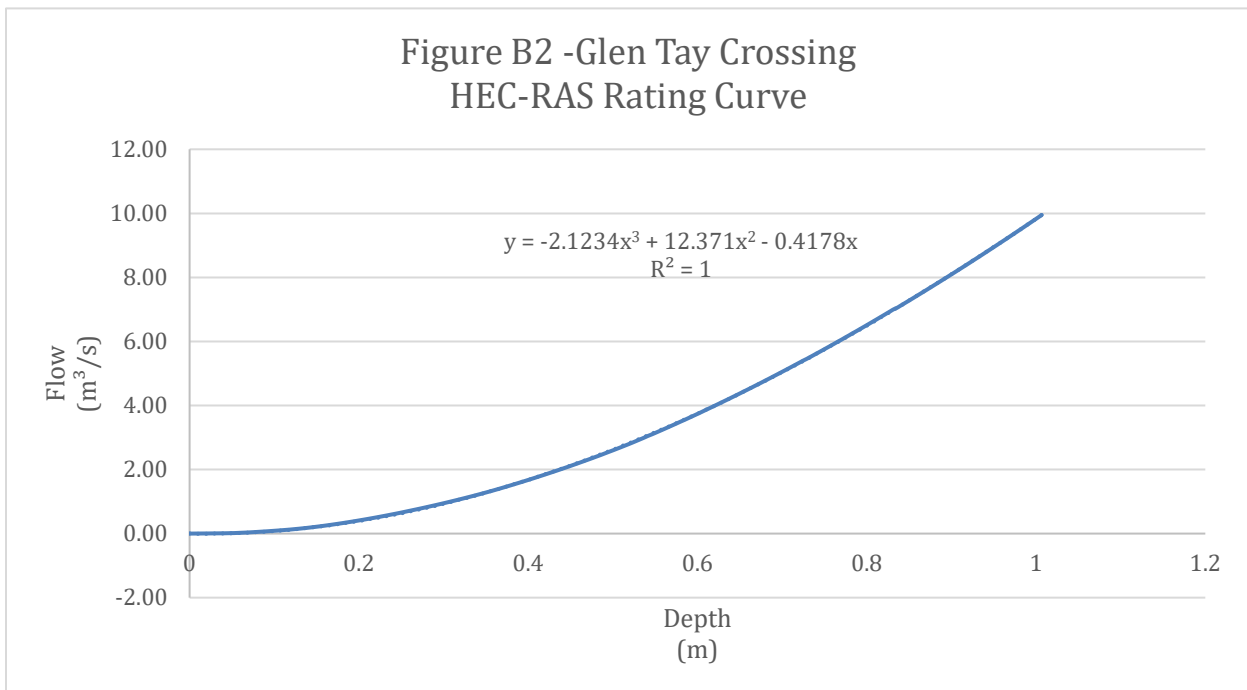
Table 3.3: Glen Tay Road Level Logger Monitoring Summary, 2022

Site	Monitoring Duration (days)		Water Temperature (°C)	Water Depth (m)	Duration of measured zero depth
Glen Tay Road	152	Min	5.3	0.07	0 days / 0%
		Max	30.9	0.38	
		Avg	18.6	0.18	

Based on the above results the average water depth at this location was **18 cm**, and only fluctuated by **31 cm** over the full monitoring period. While the average water temperature was **18.6 °C** varied by **25.6 °C** over the monitoring period. Note that there was a constant based flow at this location and the channel never went dry during the monitoring period.

3.4 Glen Tay Road Rating Curve / Flow Derivation

A detailed survey of the bridge crossing under Glen Tay Road was completed by JFSA field staff, and the crossing details were incorporated into a simple HEC-RAS model to derive a rating curve (Depth vs Flow) relationship at this location (refer to **Figure B2**), to allow for the continuous water level depths measured at this location to be converted to flows. This analysis assumes that the crossing operates under inlet control the entire time and that downstream/tailwater conditions have no impact on the flows through this crossing.



Based on the rating curve derived by HEC-RAS the total flow through this crossing was **49,759,017 m³**. There was a total rainfall volume of **325.3 mm** recorded by the gauge for this duration and the total drainage area to this location is approximately **7,986.2 ha**. Based on the above the upstream area has a runoff coefficient of **1.92**, which is not possible. As such the assumption that this crossing operates under inlet control may not always be valid and future monitoring will require a level logger both upstream and downstream of this crossing to accurately capture the impacts that the tailwater has on flows through this crossing and to the wetland.

3.5 Grants Creek Wetland

A level logger was placed at the downstream extent of the Grants Creek wetland near the confluence with the Tay River. The minimum, maximum and average for both water temperature and depth, as well as the number of days with zero depth readings, are provided in **Table 3.4**. Graphs showing continuous water depth vs. rainfall events are available in **Appendix B**.

Table 3.4: Grants Creek Wetland Level Logger Monitoring Summary, 2022

Site	Monitoring Duration (days)		Water Temperature (°C)	Water Depth (m)	Duration of measured zero depth
Grants Creek Wetland	152	Min	7.0	0.320	0 days / 0%
		Max	26.8	0.558	
		Avg	18.8	0.411	

Based on the above results the average water depth at this location was **41 cm**, and only fluctuated by **24 cm** over the full monitoring period. While the average water temperature was **18.8 °C** and varied by **19.8 °C** over the monitoring period. Note that there was a constant based flow at this location and the channel never went dry during the monitoring period.

3.6 Tay River (WSC Gauge)

Water Survey Canada (WSC) has an active gauge on the Tay River approximately 500 m downstream of the Perth GC development site which reports both water levels and flows at this location. This location includes contributions from both the Tay River and Grants Creek. The flow and level data recorded at the gauge have been overlaid with the rainfall data collected by JFSA from the subject site to give an overview of the response of the Tay River and to approximate a runoff coefficient. Full figures for this location have been provided in **Appendix B**.

Based on the JFSA rain gauge a total rainfall volume of **325.3 mm** fell over the window from June 10th to November 2nd, 2022. Water Survey Canada indicates that the gauge at this location has a total drainage area of approximately **66,100 ha** with a total flow volume of **54,152,004 m³** over this duration. Based on the above the Tay River at this location has a runoff coefficient of approximately **0.25**, which is in line with the land use for this area.

3.7 Summary

Rainfall and surface water monitoring was completed on the site from June 2022 to November 2022. The rainfall monitoring recorded **18** significant events (with volumes greater than 5mm) but all rainfall events recorded this year had return periods less than the 5-year event. Water level monitoring at the Glen Tay Road crossing indicated that tailwater impacts at this location may affect the flows through the crossing and to the wetland, as such additional loggers are advised for future monitoring to capture these impacts. The level logger in the Glen Tay wetland showed that the wetland water levels fluctuated by only **24 cm** over the full monitoring period. Combining the rainfall data with the flows recorded at the water survey Canada gauge, it was found that the Tay River at this location (which includes both the Tay River and Grants Creek) has a runoff coefficient of approximately **0.25**. As no significant rainfall events were observed in this year of monitoring it is advised that the surface water monitoring program is to be continued in the following years.

4 Floodplain Mapping Update

Accurate floodplain mapping is required for the future Perth GC development. Upon review of the floodplain mapping based on the 2013 model of the Tay River provided by the Rideau Valley Conservation Authority (RVCA), J.F. Sabourin & Associates (JFSA) determined that additional topographic data would improve/refine the floodplain boundaries affecting the subject property. This included acquiring higher resolution site-specific LiDAR of the area and completing field checks to determine culvert locations and elevations. This resulted in a site-specific refinement of the floodplain extent on the subject lands. Note that no modifications have been made to the existing hydraulic model produced by RVCA in 2013, simply the topography that the flood elevations have been mapped on has been updated using the latest available LiDAR and onsite topographic survey of existing culverts through the existing golf course.

4.1 Updated Analysis

Site-specific imagery was collected by First Base Solutions Inc. retained by David Schaeffer Engineering Ltd (DSEL), on November 7th, 2021 at a ground sample distance of 6 cm and controlled with Airborne Kinematic GPS and Surveyed Ground Control points. The resulting mapping was compiled with a DTM capable of producing 0.25m contours and map accuracies of +/- 12cm at a 95% confidence level on well-defined, easily visible objects within the imagery. Note that this LiDAR has been used in other analyses outlined in this report such as section 2.1-Development Site. This site-specific data is at a much higher resolution than the 1m contour data used in the RVCA model. **Figure C1** outlines the regulatory floodplain boundary based on the work completed by RVCA as a part of the “Tay River Flood Risk Mapping Report, Glen Tay Road to Lower Rideau Lake, 2013”, and is the current official floodplain extent for this location. JFSA used the newer LiDAR to run the original HECRAS model with updated terrain to delineate a refined floodplain extent shown in **Figure C2**. It is important to note that no model parameters have been changed from the RVCA model of record for the Tay River. The floodplain extents produced by this model have simply been remapped using the latest available topographic data.

4.2 Field Verification

To further the accuracy of the results obtained through the updated topographic data, staff from J.D Barnes Limited (JDB) surveyed culvert locations on the golf course. Several key locations were identified as potentially having a direct impact on the floodplain extent outcome, as they would allow the Tay River floodplain to connect to Grant’s Creek through the subject property. JDB field staff surveyed the culvert invert and obvert elevations to determine whether floodwaters would be permitted to pass through. Results from the survey can be seen in **Figure C3**. All culverts surveyed were determined to be below their adjacent floodplain elevation, which means that they would allow floodwaters to spill onto the proposed development site between the Tay River and Grant’s Creek.

4.3 Summary

Based on the compiled data and completed field checks, JFSA is confident in the accuracy of the floodplain extent generated using the updated topographic data. The refined boundaries produced by JFSA are georeferenced and have been provided to JDB and Caivan (Perth GC) Ltd. in support of the development of the proposed draft plan of subdivision and used in the balance cut fill analysis completed by DSEL. Note that no modifications have been made to the existing hydraulic model produced by RVCA in 2013, simply the topography that the flood elevations have been mapped on has been updated.

5 Existing Water Budget Modelling

A continuous SWMHYMO hydrologic model has been developed to assess the site's pre-development water budget. The model makes use of the pre-development water budget analysis completed by GEMTEC for this site. Model parameters have been adaptively adjusted as a part of this analysis to ensure the continuous simulation results are in line with the static MOE-style water budget completed by GEMTEC.

5.1 Continuous Simulation Modelling

The continuous SWMHYMO model was run using 36 years of hourly rainfall data from the Ottawa International Airport from 1967 to 2003 (excluding missing 2001 rainfall data), and the average annual evaporation, infiltration and runoff volumes from the subject site were computed and compared. Note that this rain gauge is generally only operational for the months of April-November. Outside of this window precipitation is more likely to be in the form of snowfall and the soils are also more likely to be frozen, making it difficult to simulate such conditions with a hydrologic model using conventional parameters, as such, this period has not been considered in the analysis. Note that GEMTEC's water budget analysis considered the full year, while the JFSA analysis only considered April to November. To resolve this disconnect the total percentage of rainfall that evaporates infiltrates and runs off each year has been matched as an alternative to matching annual volumes, which is not possible due to the difference in analysis windows.

5.2 Model Parameters

As a part of this analysis the Initial Abstraction (IA) value and Curve Number (CN) values have been iteratively adjusted to calibrate the model to produce similar results to the GEMTEC water budget. It is justifiable to adjust the IA value as it is known that the typical initial abstraction/wetting loss that occurs throughout the year fluctuates with the seasons and amount of vegetation present. Additionally, although CN can be derived empirically by simply looking at the land use and soil type alone, for natural lands again the volume of runoff that infiltrates varies throughout the year depending on the soil conditions (High runoff in the winter-freshet when the soils are either frozen or saturated and very little runoff in the summer when the soils are dry) and the degree of vegetation present throughout the year.

Based on this analysis the typical average annual Initial Abstraction value for the site was found to be 8.75 mm. Although this is relatively high compared to typical IA values assumed for grassed lands (5 mm), it is within the range outlined in the Design & Construction of Urban Stormwater Management Systems, ASCE, (1992) for vegetated areas which range from 2.5 mm to 12.7mm depending on the extent and type of vegetation present. Based on this analysis the typical average CN value for the site is 90 and 92 for the Grants Creek and Tay River drainage areas respectively. Again, these values are higher than what is typically assumed for design storms due to two factors; the water budget analysis considers winter /spring freshet months when the soils are frozen and little infiltration will occur, where CN values of 95-99 are typically assumed for these conditions. Additionally, a large portion of the site is currently bedrock at the surface or with very shallow overburden, as such these areas are treated as impervious (CN 99).

It is important to note that the model parameters adopted above are an annual average representation of the site and will fluctuate throughout the year with various seasonal changes. As such the typical Textbook IA and CN values have been adopted for the preliminary SWM ponding sizing completed in Section 6 below, as the design storms assumed for that analysis area based on summer rainfall events.

5.3 Model Results

The complete SWMHYMO modelling input and summary files have been provided in **Appendix D. Table D.1 and D.2** provide a full summary of the SWMHYMO modelling, based on the 39 years of data, and outline the maximum, minimum and average volumes and percentages of precipitation that evaporate infiltrate and runs off for the Grants Creek and Tay River portions of the development site respectively. **Table 5.1** below is an excerpt from this summary.

Table 5.3: Pre-Development Water Budget based on Continuous Simulations

Location	Precipitation (mm)	Total Evaporation (mm)	Total Infiltration (mm)	Total Runoff (mm)
Grants Creek	589.1	361.2 62%	85.2 15%	142.8 24%
Tay River	589.1	361.2 62%	93.9 16%	134.0 22%

Based on the continuous simulations using 39 years of historical rainfall data it was determined that for the total development site, approximately **22-24%** of the annual rainfall will result in runoff, **62%** will evaporate and **15-16%** will infiltrate.

5.4 Summary

Continuous hydrologic modelling has been completed which has made use of the water budget modelling completed by GEMTEC. Based on this analysis it was determined that under pre-development conditions for the total development site, approximately **22-24%** of the annual rainfall will result in runoff, **62%** will evaporate and **15-16%** will infiltrate.

6 Preliminary SWM Facility Sizing

The following section details the preliminary stormwater management (SWM) facility sizing for the development site based on a post-development concept plan. The various SWM facilities throughout the site will provide water quality treatment, peak flow attenuation, and flood control for the project site and have been sized to ensure that total peak flows to both Grants Creek and the Tay River match Pre-Development conditions. This analysis has been completed using SWMHYMO hydrologic modelling software, as it is well suited to simulating large undeveloped lands as well as lumped conceptual future development lands. Note that this analysis has simply been completed to provide an order of magnitude of the required storage volume of the various possible SWM facilities throughout the site, and does not give any directive on exactly where or how this volume should be provided, which will be addressed during detailed design. The primary intention of this study is to ensure that sufficient land is set aside under post-development conditions to ensure that there is adequate room for SWM facilities to meet the objectives specified above.

6.1 Design Storms

Design storms for the development were derived using historical rainfall data outlined in the MTO IDF Curve lookup tool for the exact site locations. Full details of this tool and IDF data for this location can be found using the link below.

http://www.eng.uwaterloo.ca/~dprincz/mto_site/results_out.shtml?coords=44.892975,-76.275126

Table 6.1 below outlines the rainfall intensity provided in the MTO tool. These intensities were then fit to the formula below to derive A, B and C values which were then used to derive synthetic designs storms. For this analysis, the 3-hour Chicago and 24-Hour SCS design storms were used for both pre and post-development.

$$\text{Rainfall Intensity} \left(\frac{\text{mm}}{\text{hr}} \right) = \frac{A}{(t_c + B)^C}$$

Table 6.1: Rainfall Intensity – Perth – MTO IDF Tool

Return Period (Yr)	5 mins	10 mins	15 mins	30 mins	60 mins	2 hrs	6 hrs	12 hrs	24 hrs	A	B	C
2	115.9	71.4	53.8	33.1	20.4	12.6	5.8	3.6	2.2	362.018	0.102	0.702
5	153.4	94.5	71.2	43.8	27.0	16.6	7.7	4.8	2.9	478.921	0.106	0.701
10	178.9	110.2	83.0	51.1	31.5	19.4	9.0	5.5	3.4	559.506	0.102	0.702
25	210.2	129.5	97.5	60.1	37	22.8	10.6	6.5	4.0	655.462	0.104	0.701
50	233.4	143.8	108.3	66.7	41.1	25.3	11.7	7.2	4.5	723.892	0.087	0.700
100	256.7	158.1	119.1	73.4	45.2	27.8	12.9	8.0	4.9	796.709	0.078	0.700

6.2 Drainage Areas

Figure E1 provides an overview of the development area under pre-development conditions. For this analysis under pre-development conditions, the site has simply been broken into two subcatchments, one that represented the development area contributions to the Tay River and one that represented the development area contribution to Grants Creek. As outlined above in

section 2.1 the site under existing conditions is essentially a 50-50 split to Grants Creek and the Tay River.

6.3 Land Use

Under pre-development conditions, the site primarily consists of a mix of manicured grass (golf course) surrounded by forest. Land use data has been taken from Southern Ontario Land Resource Information System (SOLRIS) v3.0 Land Use Data, publicly available through Land Information Ontario (LIO). **Figure E2** in **Appendix E** provides a visual overview of the respective land use data for each of the subcatchments within the study area.

6.4 Soil/Infiltration Data

Soil data within the study area has been taken from Soil Survey Complex Data publicly available from Land Information Ontario (LIO). **Figure E3** in **Appendix E** provides a visual overview of the respective soil type data for each of the subcatchments within the study area. From this data the site primarily consists of Monteagle and Monteagle Sandy Loam soils, which are considered a Type B SCS soil group. The site also consists of Muck and North Gower soils, which are considered a Type D SCS soil group.

6.5 Curve Number (CN)

Curve Numbers (CN) were calculated, based on underlying Land Use Type and Soil Classification at each location within the subcatchments based on values outlined in Tables A2 and A3 in the SWMHYMO Manual. Each Curve Number was then weighted based on the total area within a given subcatchment to determine the weighted CN for that subcatchment. Full CN derivation Tables have been provided in **Table E1** in **Appendix E**.

6.6 Time to Peak

Flow paths have been discretized based on the topographic data using GIS tools and the longest major flow path within each subcatchment identified; refer to **Figure E4** in **Appendix E** for the flow paths discretized for each subcatchment. The upstream and downstream topographic elevations and flow lengths were identified for each subcatchment and used in the calculations. For these natural subcatchments, the Federal Aviation Administration (FFA) method was determined to be the most appropriate method to calculate the Time to Peak. Full details of these calculations have been provided in **Table E2** in **Appendix E**, along with other time-to-peak values using alternative tp calculation methods.

6.7 Initial Abstraction

For undeveloped lands, an initial abstraction value of 5 mm has been assumed which is typical for undeveloped lands commonly used throughout Ontario (parameters are in line with typical rates per City of Ottawa Storm Sewer Design Guidelines). Full SWMHYMO modelling input and summary files for pre-development conditions have been provided in **Appendix E**

6.8 Pre-Development Results

As outlined above the model has been run using both the 3-hour Chicago and 24-hour SCS design storms, for the 2-, 5-, 10-, 25-, 50- and 100-Year events as well as the 25mm event. **Table 6.2** below outlines the peak flows from the development site to both Grants Creek and the Tay River under pre-development conditions. Note that although the drainage area to Grants Creek is

slightly smaller than that to the Tay River, the Grants Creek drainage area is producing slightly larger peak flows for most events. This is due to the fact that the Grants Creek drainage areas are slightly steeper, which results in higher peak flows for an equivalent area.

Table 6.2 – Pre-Development Peak Flow Summary

Event	Grants Creek (22.01 ha) Peak Outflow (m ³ /s)	Tay River (22.86 ha) Peak Outflow (m ³ /s)
25MMC3H	0.125	0.126
2YRCHI3HR	0.149	0.150
5YRCHI3HR	0.288	0.285
10YRCHI3HR	0.399	0.393
25YRCHI3HR	0.559	0.546
50YRCHI3HR	0.688	0.669
100YRCHI3HR	0.830	0.804
2YRSCS24HR	0.463	0.444
5YRSCS24HR	0.790	0.750
10YRSCS24HR	1.044	0.986
25YRSCS24HR	1.382	1.298
50YRSCS24HR	1.653	1.545
100YRSCS24HR	1.932	1.800

6.9 Post-Development Drainage Area and Imperviousness

To assist in the post-development SWM facility sizing a conceptual site servicing plan has been developed by DSEL. Based on this plan the site will have 3 SWM ponds and 2 uncontrolled areas that will provide water quality and quantity control through LIDs and OGS units. **Figure E5** in **Appendix E** provides an overview of the conceptual development plan. Based on this plan approximately **16.32 ha** will drain to Grants Creek and the remaining **28.54 ha** to the Tay River. Note that efforts have been made to maintain the existing drainage areas within the development as much as possible with consideration for grading and servicing limitations.

Based on this conceptual plan there will be two SWM ponds (Ponds 1 & 3) that will discharge to the Tay River and a small segment (**1.35 ha**) that will have an OGS and LIDs. There will be one SWM Pond (Pond 2) that will discharge to Grants Creek and a small segment (**1.75 ha**) of development that will have an OGS and LIDs. To simplify this analysis while also being conservative it was assumed that the whole development would have a runoff coefficient of 0.7 (66% imperviousness) and it was assumed that 90% of the impervious area will be directly connected to the storm sewer infrastructure, which is a conservative assumption.

To ensure sufficient storage volume is provided for the lands treated by the OGS units, these drainage areas have been lumped with the areas treated by the SWM pond. This allows flexibility at the detailed design stage to allow for adjustment of drainage areas (controlled and uncontrolled) while ensuring sufficient storage volume is provided to the site.

6.10 Soil Infiltration

To represent the proposed developed land infiltration rates, Horton's infiltration has been used. For these lands, the following Horton's Infiltration parameters have been applied: $F_o=76.2$ mm/hr, $F_c=13.2$ mm/hr, $DCAY=4.14$ /hr, $F=0$ mm. These Horton infiltration rates are typical for urban grassed areas and are commonly used throughout Ontario (infiltration parameters are in line with typical rates per City of Ottawa Storm Sewer Design Guidelines)

6.11 Quality Control Volumes

Quality control active storage volumes for each of the facilities have been calculated as per MOE guidelines, based on the required 40 m³/ha. There is a total of **28.54 ha** of land that will drain to the Tay River and as such will require **1,142 m³** of quality control storage. There is a total of **16.32 ha** of land that will drain to Grants Creek and as such will require **653 m³** of quality control storage. Note that the SWM pond preliminary sizing has considered this quality control volume to drain over 48 hours, and this active quality control volume has been included in the preliminary facility sizing.

6.12 Quantity Control Volumes

As mentioned above the SWM Facilities will be designed to meet pre-development peak flows at the two respective watercourses/receivers. **Tables 6.3A** and **6.3B** below outline the pre-and post-development flows and the associated required SWM Facility storage volumes to ensure that the proposed development meets these pre-development rates. Note that the total peak flow to Grants Creek and the Tay River are either equal to or less than that specified under pre-development conditions for all events. Full SWMHYMO modelling input and summary files for post-development conditions have been provided in **Appendix E**.

From **Table 6.3A** below it is seen that the development area draining south to Grants Creek will need a total of approximately **6,343 m³** of active storage to attenuate post-development flows to pre-development conditions up to and including the 100-year event. From **Table 6.3B** below it is seen that the development area draining north to the Tay River will need a total of approximately **13,662 m³** of active storage to attenuate post-development flows to pre-development conditions up to and including the 100-year event. Note that the required unitary storage volumes (m³/ha) are higher for the area draining to the Tay River due to the increase in total drainage area when compared to pre-development conditions.

Table 6.3A: Grants Creek - Preliminary SWM Pond Sizing

Event	Pre Development 22.01 Peak Outflow (m ³ /s)	Post Development							
		SWM Pond 2 + OGS2 16.32 ha		OGS2 1.75 ha		SWM Pond 2 14.57 ha			
		Peak Outflow (m ³ /s)	Required Volume (m ³)	Peak Outflow (m ³ /s)	Required Volume (m ³)	Peak Outflow (m ³ /s)	Required Volume (m ³)		
Quality Control*	-	0.002	653	0.0002	70	0.002	583		
25MMC3H	0.125	0.125	1,723	0.013	185	0.112	1,538		
2YRCHI3HR	0.149	0.149	1,974	0.016	212	0.133	1,762		
5YRCHI3HR	0.288	0.288	2,684	0.031	288	0.257	2,396		
10YRCHI3HR	0.399	0.399	3,151	0.043	339	0.356	2,812		
25YRCHI3HR	0.559	0.559	3,773	0.060	406	0.499	3,367		
50YRCHI3HR	0.688	0.688	4,223	0.074	454	0.614	3,769		
100YRCHI3HR	0.830	0.830	4,678	0.089	503	0.741	4,175		
2YRSCS24HR	0.463	0.408	3,184	0.044	342	0.364	2,842		
5YRSCS24HR	0.790	0.672	4,167	0.072	448	0.600	3,719		
10YRSCS24HR	1.044	0.918	4,795	0.099	515	0.819	4,280		
25YRSCS24HR	1.382	1.382	5,409	0.149	581	1.233	4,828		
50YRSCS24HR	1.653	1.652	5,874	0.178	631	1.474	5,243		
100YRSCS24HR	1.932	1.930	6,343	0.207	682	1.723	5,661		

Quality control volume (40 m³/ha) released over 48 hours

Table 6.3B: Tay River - Preliminary SWM Facility Sizing

Event	Pre Development 22.86 Peak Outflow (m ³ /s)	Post Development								
		SWM Pond 1 11.26 ha		SWM 3 + OGS 17.28 ha		OGS1 1.353 ha		SWM Pond 3 15.93 ha		Tay Total 28.54
		Peak Outflow (m ³ /s)	Required Volume (m ³)	Peak Outflow (m ³ /s)	Required Volume (m ³)	Peak Outflow (m ³ /s)	Required Volume (m ³)	Peak Outflow (m ³ /s)	Required Volume (m ³)	Peak Outflow (m ³ /s)
Quality Control*	-	0.001	450	0.002	691	0.000	54	0.002	637	-
25MMC3H	0.126	0.050	1,318	0.076	2,025	0.006	159	0.070	1,866	0.126
2YRCHI3HR	0.150	0.059	1,540	0.091	2,363	0.007	185	0.084	2,178	0.150
5YRCHI3HR	0.285	0.112	2,089	0.173	3,204	0.014	251	0.159	2,953	0.285
10YRCHI3HR	0.393	0.155	2,452	0.238	3,759	0.019	294	0.219	3,465	0.393
25YRCHI3HR	0.546	0.215	2,943	0.331	4,510	0.026	353	0.305	4,157	0.546
50YRCHI3HR	0.669	0.264	3,325	0.405	5,097	0.032	399	0.373	4,698	0.669
100YRCHI3HR	0.804	0.317	3,695	0.487	5,664	0.038	443	0.449	5,221	0.804
2YRSCS24HR	0.444	0.175	2,502	0.269	3,831	0.021	300	0.248	3,531	0.444
5YRSCS24HR	0.750	0.296	3,342	0.454	5,116	0.036	400	0.419	4,716	0.749
10YRSCS24HR	0.986	0.389	3,871	0.597	5,931	0.047	464	0.550	5,467	0.986
25YRSCS24HR	1.298	0.512	4,481	0.786	6,869	0.062	538	0.724	6,331	1.298
50YRSCS24HR	1.545	0.609	4,940	0.936	7,560	0.073	592	0.862	6,968	1.545
100YRSCS24HR	1.800	0.710	5,397	1.090	8,265	0.085	647	1.005	7,618	1.800

Quality control volume (40 m³/ha) released over 48 hours

6.13 Summary

A preliminary SWM Facility sizing has been completed for the proposed development site. The SWM Facility release rates have been determined based on the pre-development conditions modelling outlined above. Based on this analysis it was found that the development area draining north to the Tay River will need a total of approximately **13,662 m³** of active storage to attenuate post-development flows to pre-development conditions up to and including the 100-year event. The development area draining south to Grants Creek will need a total of approximately **6,343 m³** of active storage to attenuate post-development flows to pre-development conditions up to and including the 100-year event. Note that the primary intention of this analysis is to guarantee that sufficient land is set aside under post-development conditions to ensure that there is adequate room for SWM facilities to meet the objectives specified above.

7 CONCLUSION

This report has provided an overview of the various existing hydraulic and hydrologic conditions of the site. A drainage area analysis has been completed for the Tay River and Grants Creek. Based on post-development plans there will be a drainage area boundary revision of approximately **5.69 ha** from existing conditions. This results in an increase of **+0.009%** to the Tay River watershed and a decrease of **-0.06%** to the total Grants Creek watershed. Given the size of these watersheds, and the location of the drainage area change (at the confluence of the two watersheds) it is unlikely that this change under post-development conditions will have a quantifiable impact on the hydraulic and hydrologic of the surrounding watercourses.

Rainfall and surface water monitoring was completed on the site from June 2022 to November 2022. The rainfall monitoring recorded **18** significant events (with volumes greater than 5mm) and all rainfall events recorded had return periods less than the 5-year event. Water level monitoring at the Glen Tay Road crossing indicated that tailwater impacts at this location may affect the flows through the crossing and to the wetland, as such additional loggers are advised for future monitoring to capture these impacts. The level logger in the Glen Tay wetland showed that the wetland water levels fluctuated by only **24 cm** over the full monitoring period. Combining the rainfall data with the flows recorded at the water survey Canada gauge, it was found that the Tay River at this location (which includes both the Tay River and Grants Creek) has a runoff coefficient of approximately **0.25**.

The RVCA floodplain mapping boundaries surrounding the subject area have been updated based on the latest available data to ensure accurate delineation supporting the balance cut fill analysis completed by DSEL. No modifications have been made to the existing hydraulic model produced by RVCA in 2013, simply the topography that the flood elevations have been mapped on has been updated.

Continuous hydrologic modelling has been completed which has made use of the water budget modelling completed by GEMTEC. IA and CN values were iteratively adjusted to align with GEMTEC's pre-development annual water budget. Based on this analysis it was determined that under pre-development conditions for the total development site, approximately **22-24%** of the annual rainfall will result in runoff, **62%** will evaporate and **15-16%** will infiltrate.

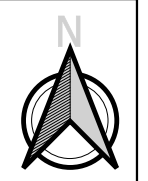
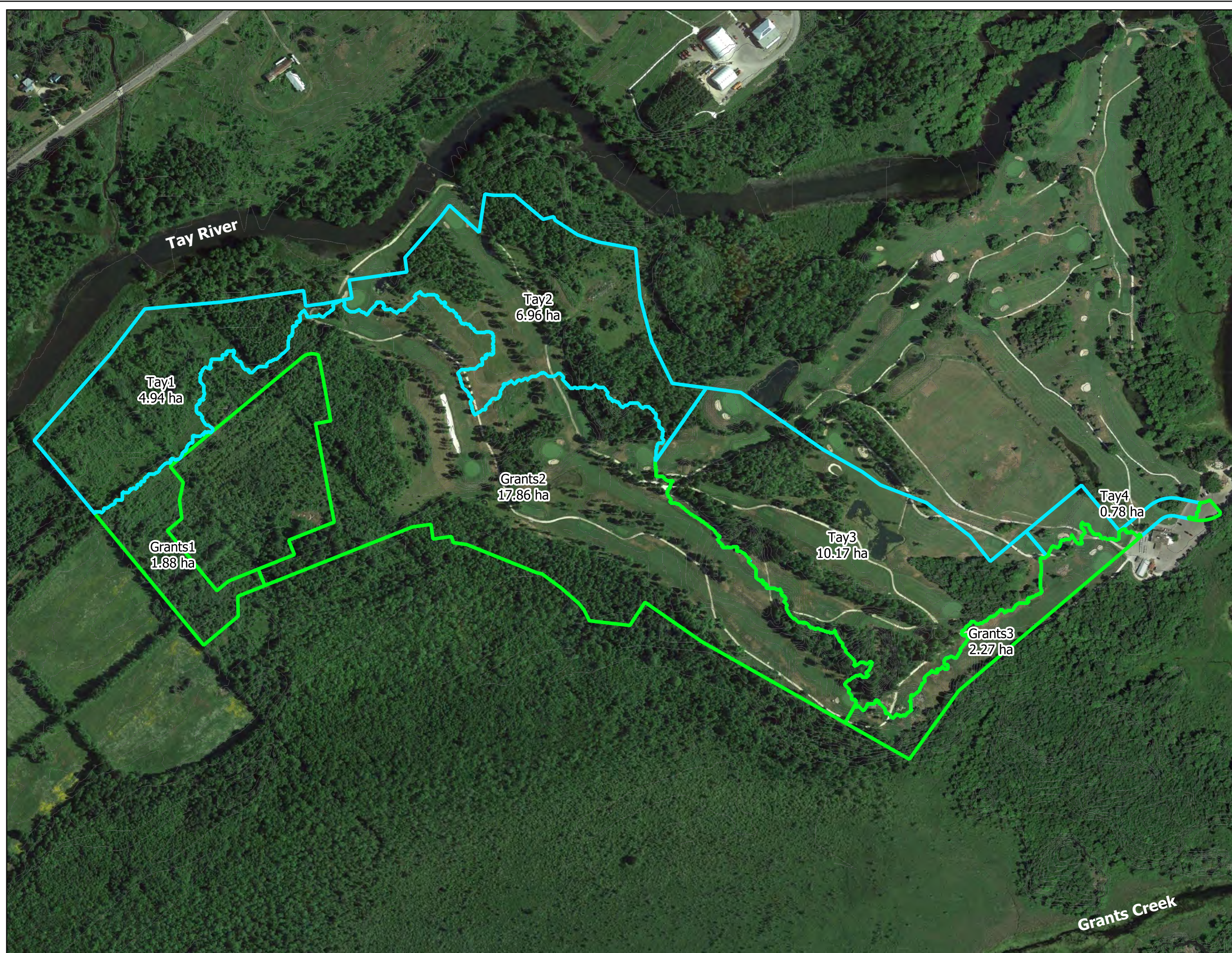
A preliminary SWM Facility sizing has been completed for the proposed development site. The SWM Facility release rates have been determined based on the pre-development conditions. Based on this analysis it was found that the development area draining north to the Tay River will need a total of approximately **13,662 m³** of active storage to attenuate post-development flows to pre-development conditions up to and including the 100-year event. The development area draining south to Grants Creek will need a total of approximately **6,343 m³** of active storage to attenuate post-development flows to pre-development conditions up to and including the 100-year event. Note that this analysis is simply a conceptual analysis, with the primary intention to ensure that sufficient land is set aside under post-development conditions to ensure that there is adequate room for SWM facilities to meet the objectives specified above.

8 JFSA STATEMENT OF LIMITATIONS

J.F. Sabourin and Associates Inc. (JFSA) has prepared this report and performed the services described in this report, in a manner consistent with the level of care and skill normally exercised by members of the engineering and science professions currently practicing under similar conditions in the jurisdiction in which the services are provided, subject to the time limits and financial and physical constraints applicable to the services. No other warranty, expressed or implied, is made. This report has been prepared for the exclusive use of the client representative, for the specific site, objective, and purpose described to JFSA by the client. The factual data, interpretations and recommendations pertain to a specific project as described in this report and do not apply to any other project or site location. Any change in site conditions, purpose and/or development plans may alter the validity of the report. The report, which specifically includes all tables, figures and appendices, is based on data and information assembled by JFSA and is based on the conditions at the site and study area at the time of the work and on the information provided by others. JFSA has relied in good faith on all information provided and does not accept responsibility for any deficiencies, misstatements, or inaccuracies contained in the report as a result of omissions, misinterpretation, or fraudulent acts of the persons contacted or errors or omissions in the reviewed documentation and data. Any use that a third party makes of this report, or any reliance on, or decisions to be made based on it, are the responsibilities of such third parties. JFSA accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this report.

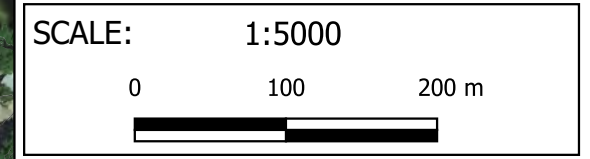
Appendix A

Existing Drainage



Legend

- Drainage Area
[Name]
[Area]
- Drains to Tay River
 - Drains to Grants Creek
- Contours (m)
— 0.5



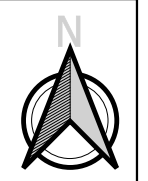
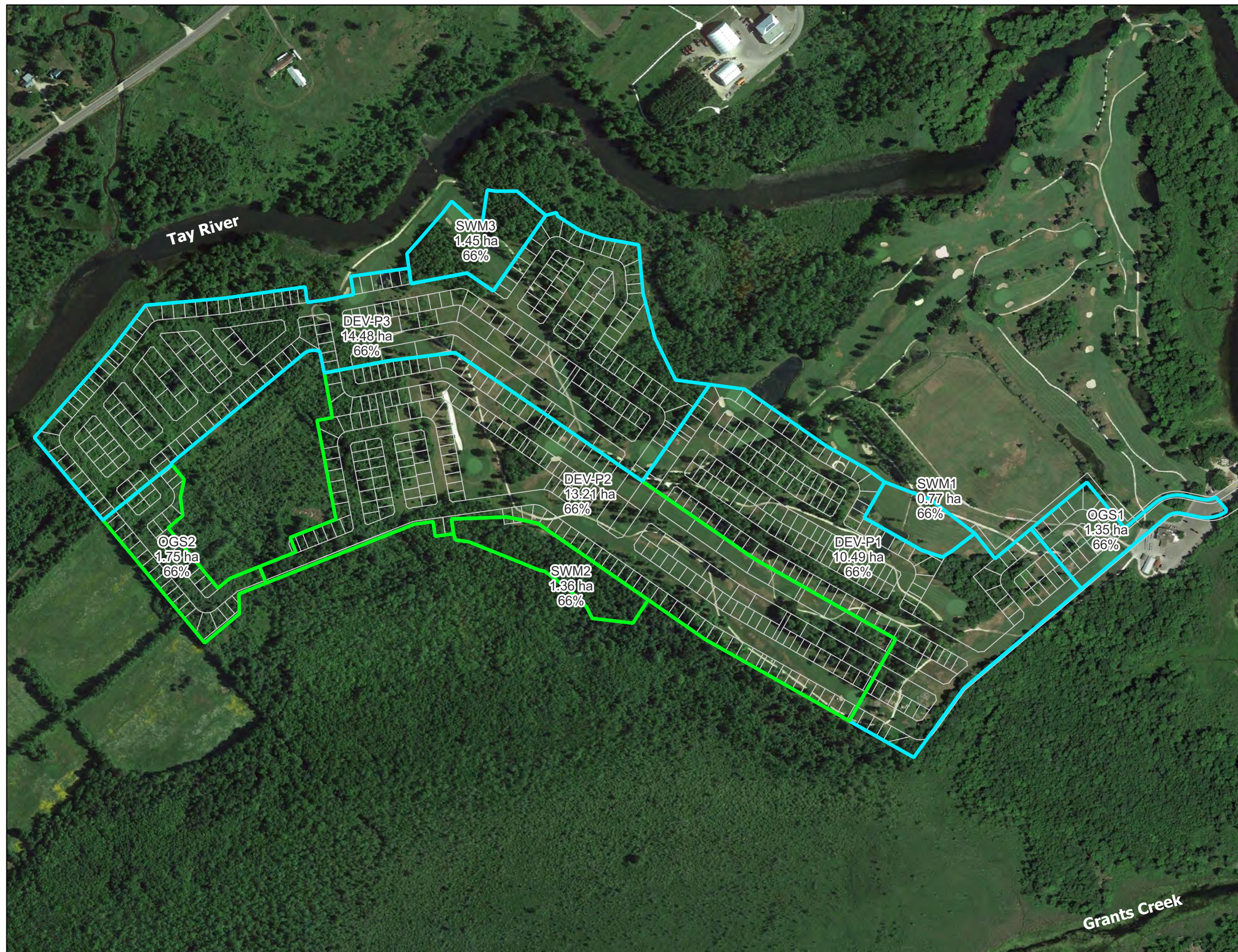
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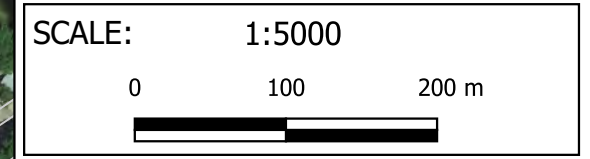
Figure A1: Existing Drainage Conditions

PROJECT	2118-21
DRAWN	BT
DATE	February 2023



Legend

- Drainage Area
 [Name]
 [Area]
 [% Imp]
- Drains to Tay River
 - Drains to Grants Creek
 - Proposed Development Plan



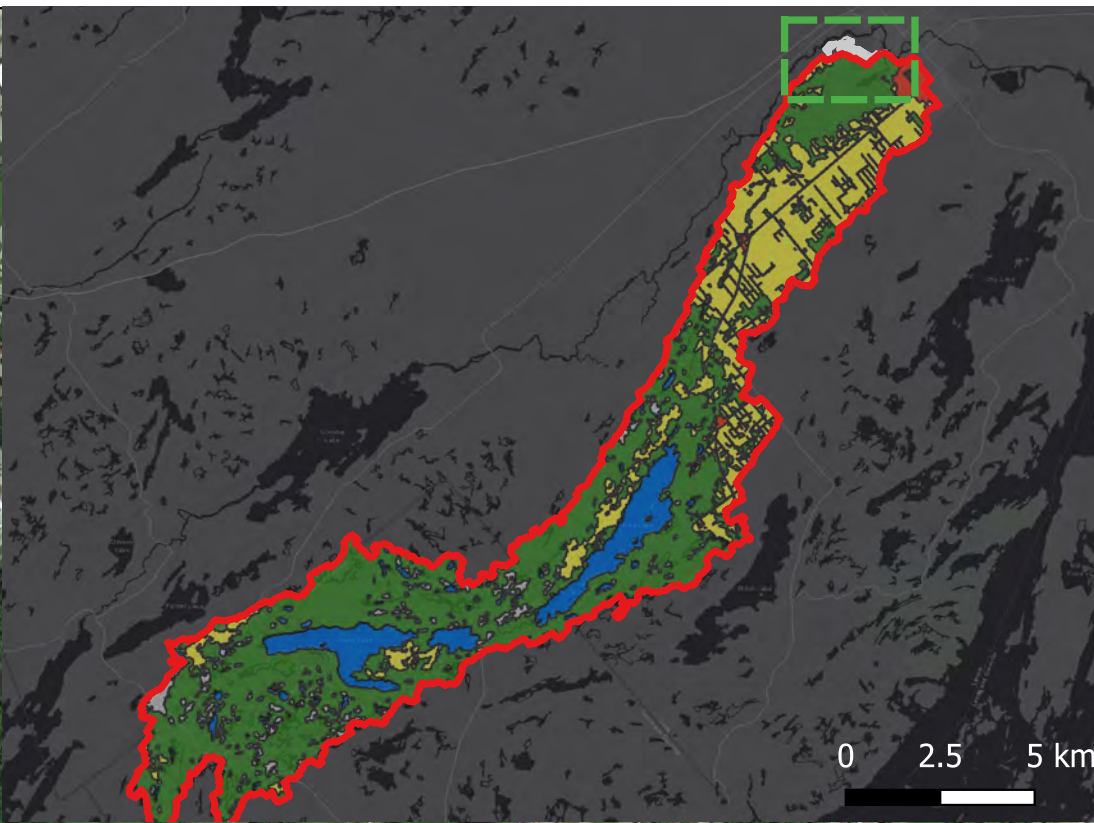
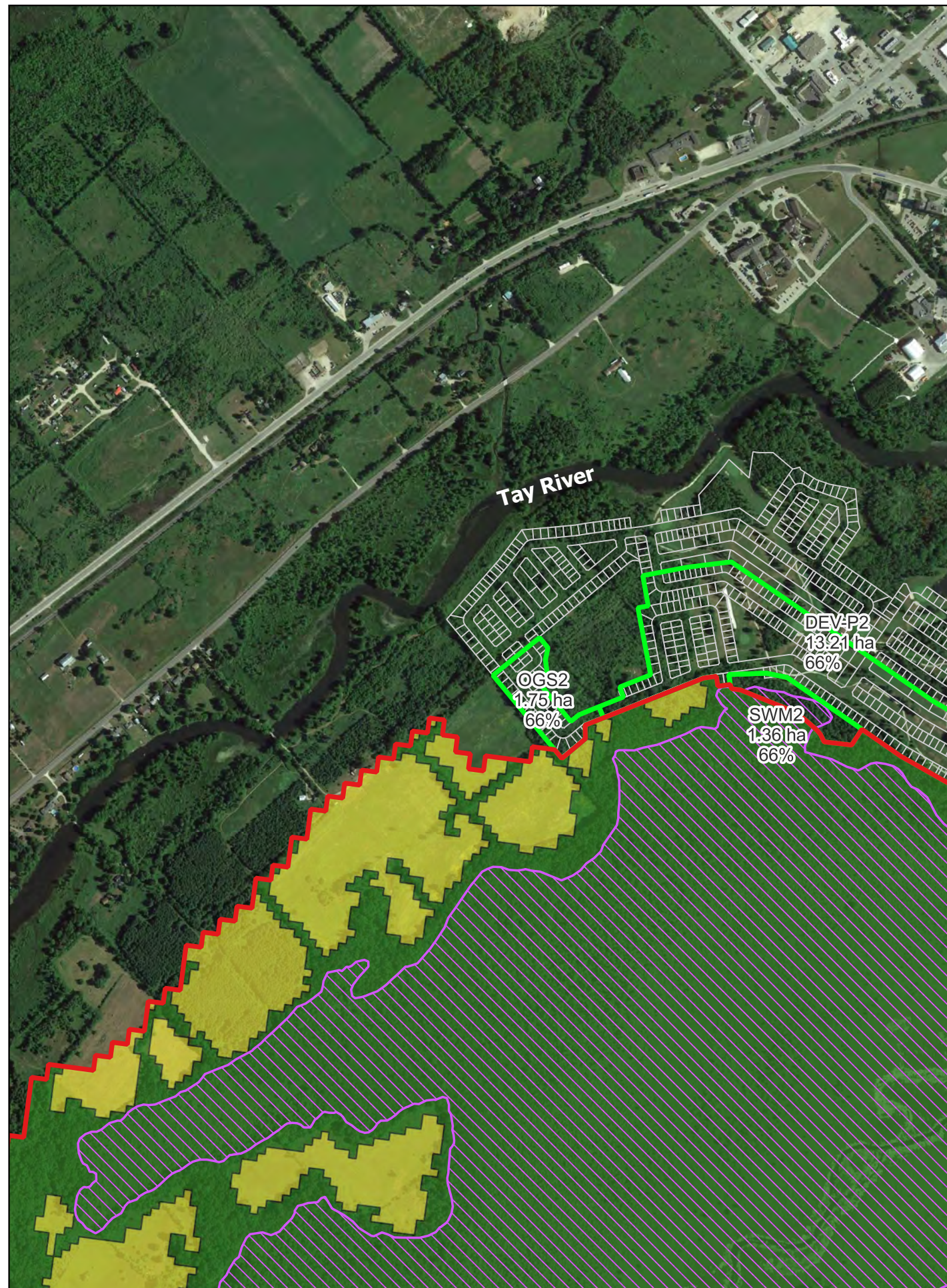
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Figure A2: Post-Development Drainage Area

PROJECT	2118-21
DRAWN	BT
DATE	February 2023



N

Legend

- Grants Creek Watershed (9,351.78 ha)
- RVCA Significant Wetland
- Study Area
- Post-Development Drainage Area
- Grants Creek
- Proposed Development Plan
- Land Cover
- Agriculture
- Bedrock
- Community/Infrastructure
- Natural
- Water

SCALE: 1:10000

0 200 400 m

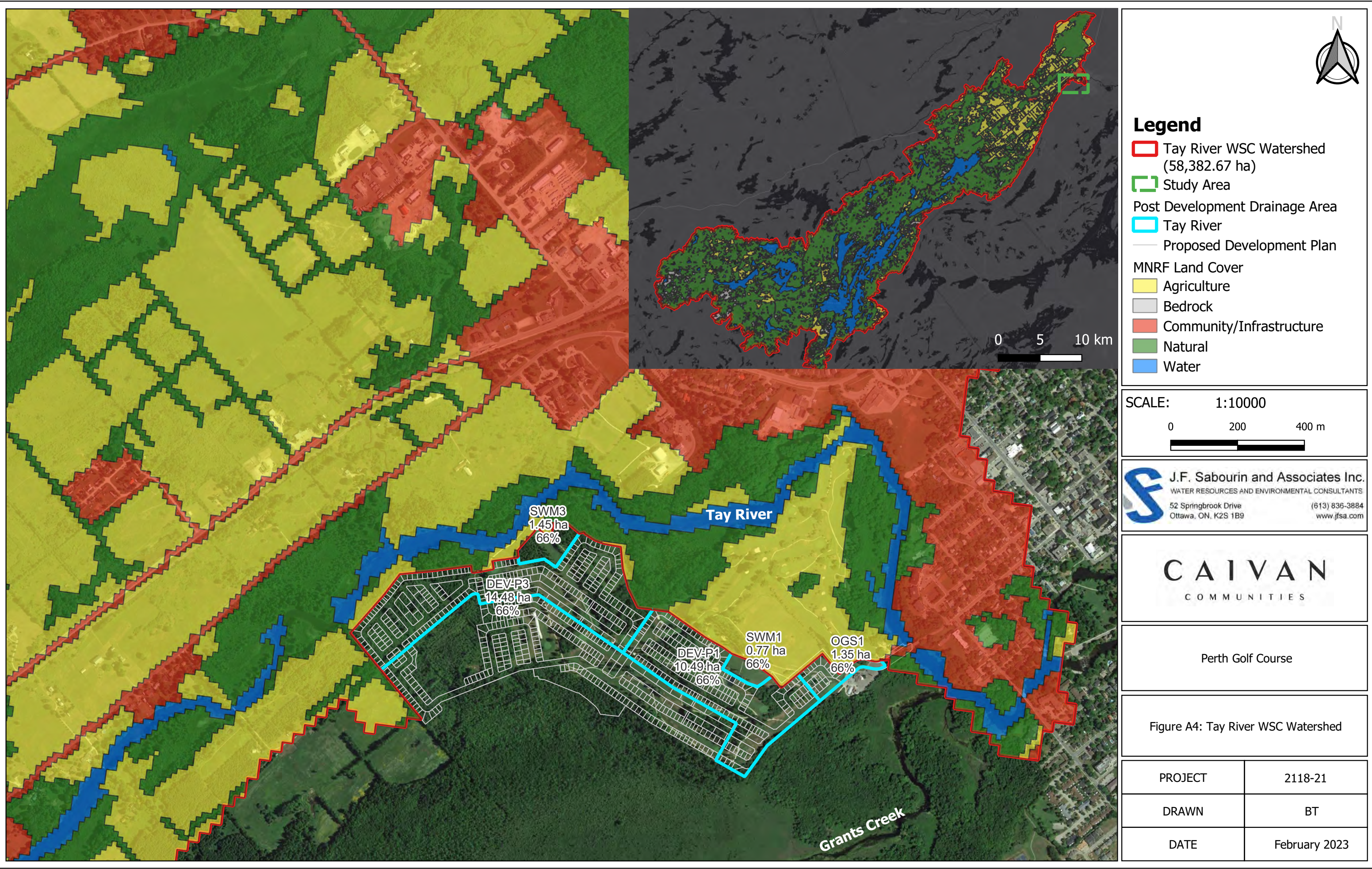
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Figure A3: Grants Creek Watershed

PROJECT	2118-21
DRAWN	BT
DATE	February 2023




Legend

- Tay River WSC Watershed (58,382.67 ha)
- Study Area
- Post Development Drainage Area
- Tay River
- Proposed Development Plan
- MNRF Land Cover
- Agriculture
- Bedrock
- Community/Infrastructure
- Natural
- Water

SCALE: 1:10000

0 200 400 m

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Figure A4: Tay River WSC Watershed

PROJECT	2118-21
DRAWN	BT
DATE	February 2023

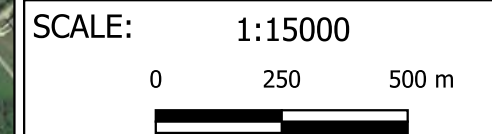
Appendix B

Surface Water Monitoring



Legend

- Rain Gauge Site
- WSC Site
- Grant's Creek Logger Site
- Glen Tay Road Logger Site
- Watercourses



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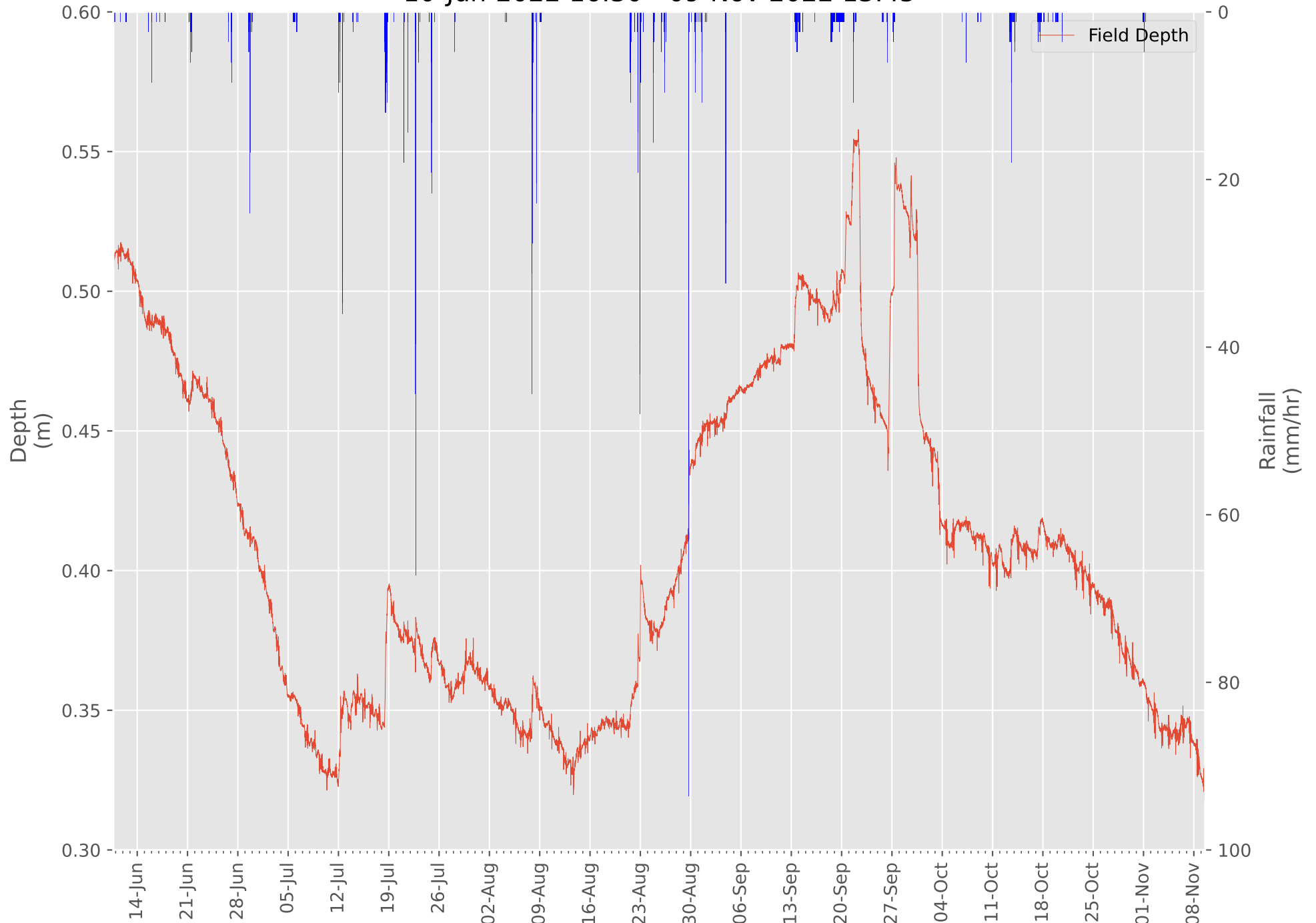
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Figure B1:
 2022 Monitoring Locations

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DRAWN	MP
DATE	FEB 2023

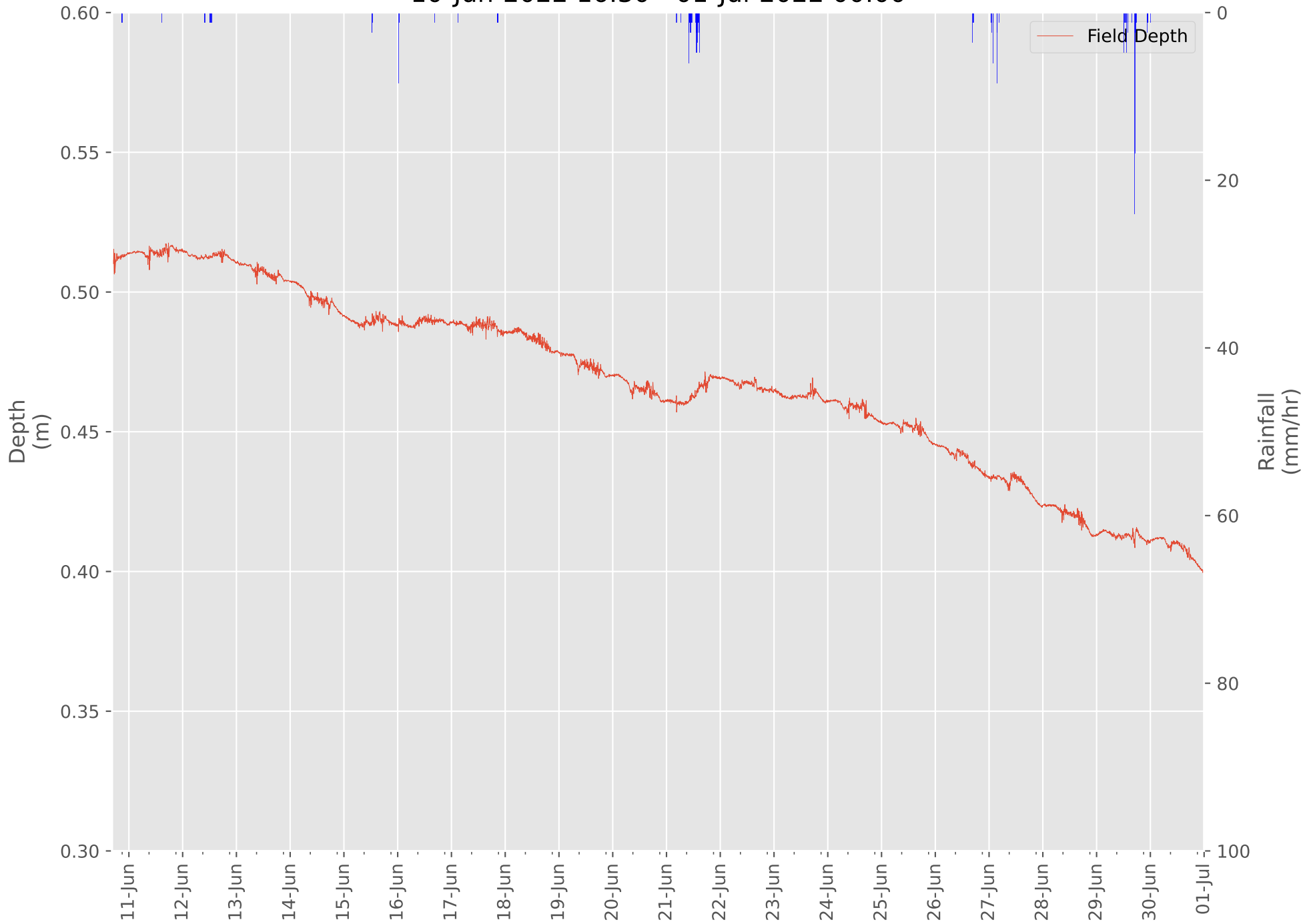
Glen Tay Crossing

10-Jun-2022 16:30 - 09-Nov-2022 13:45



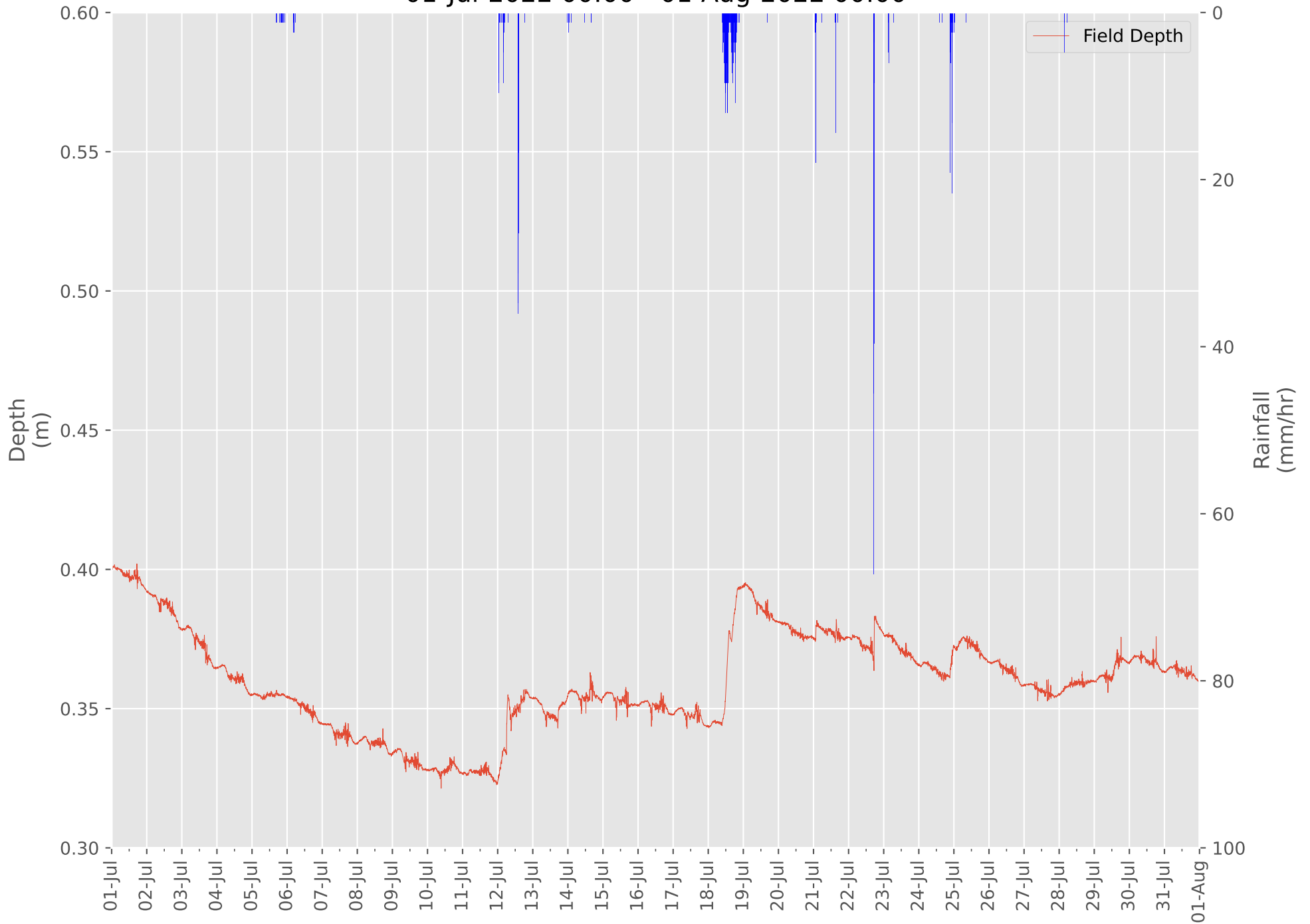
Glen Tay Crossing

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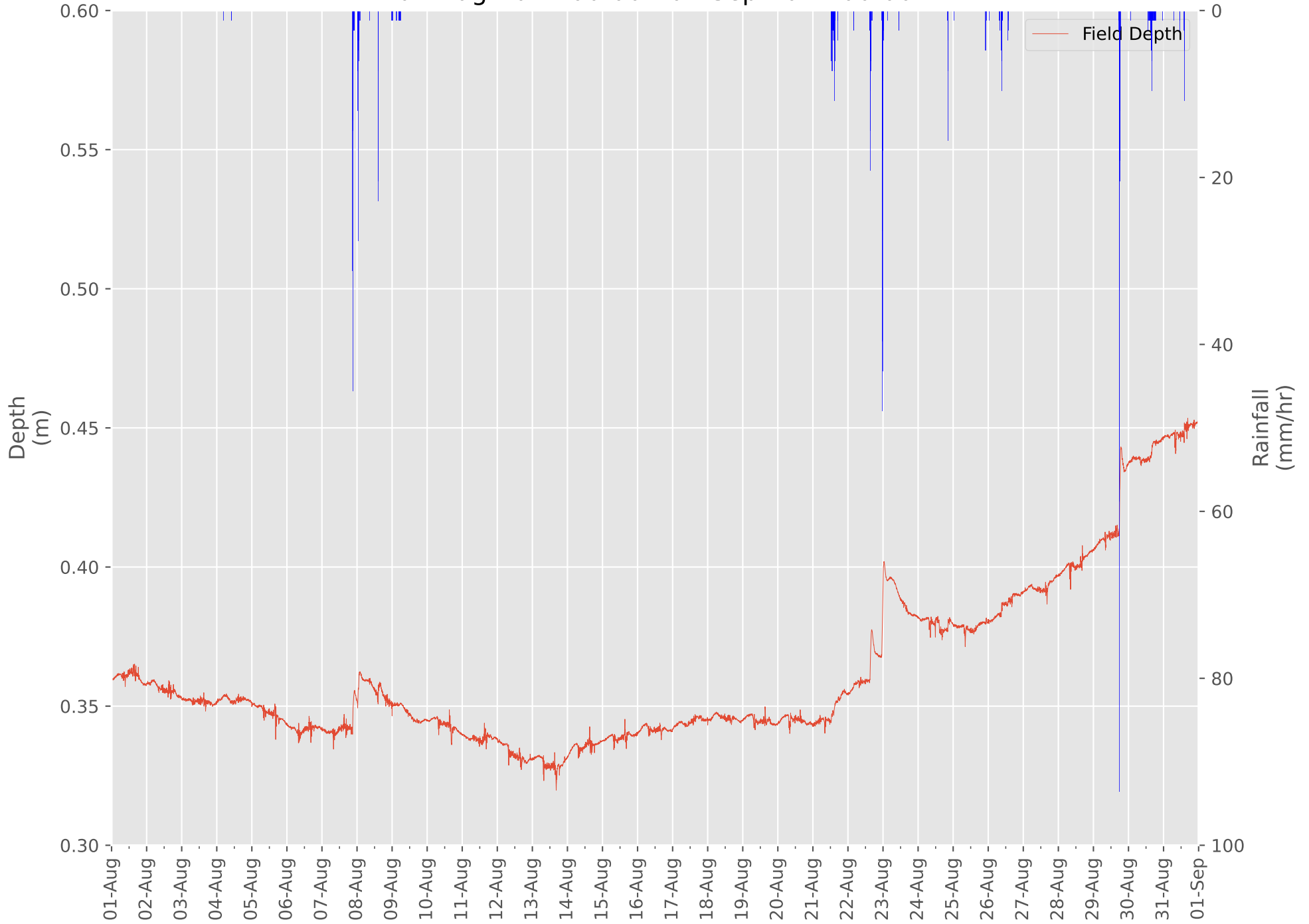
Glen Tay Crossing

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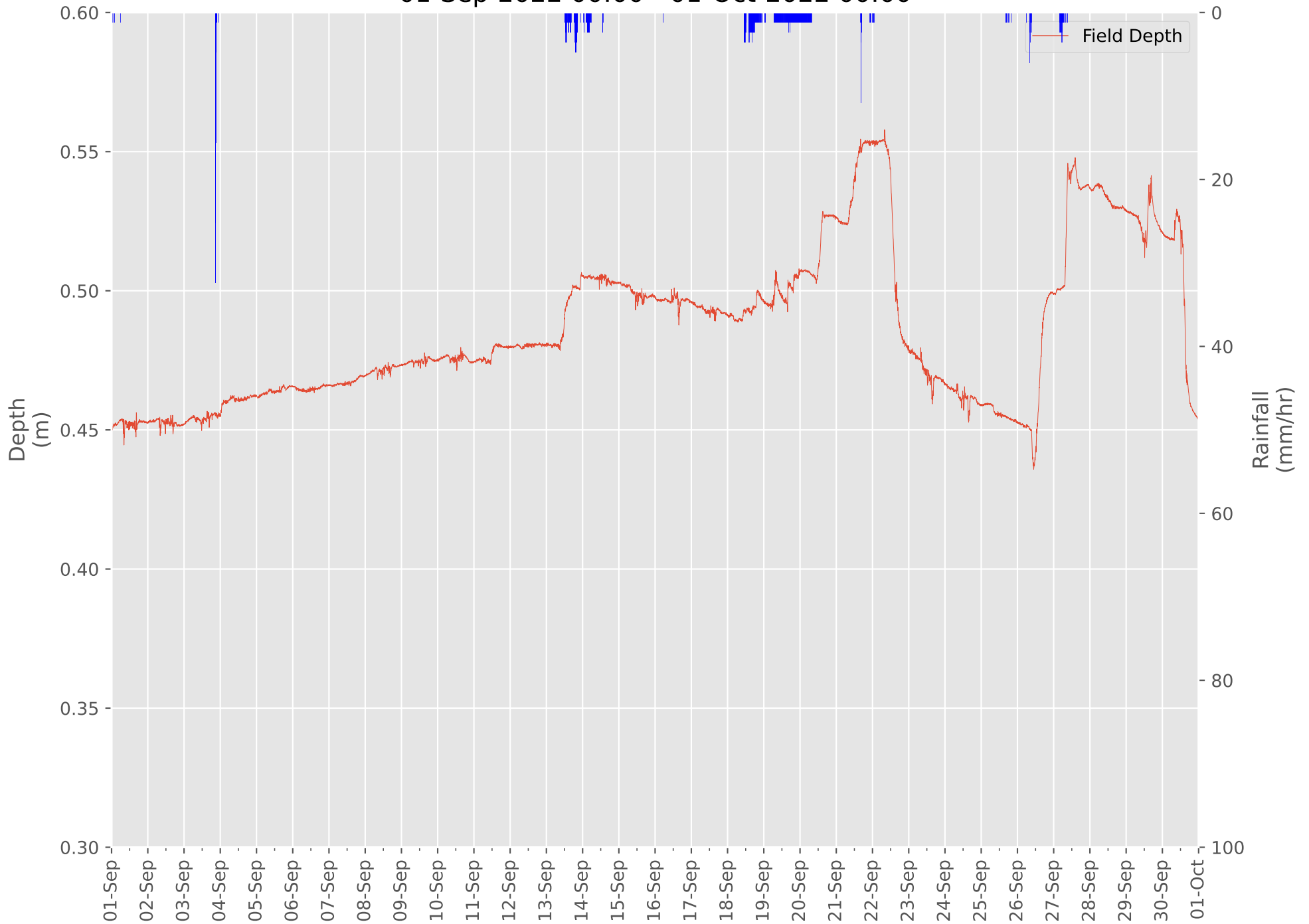
Glen Tay Crossing

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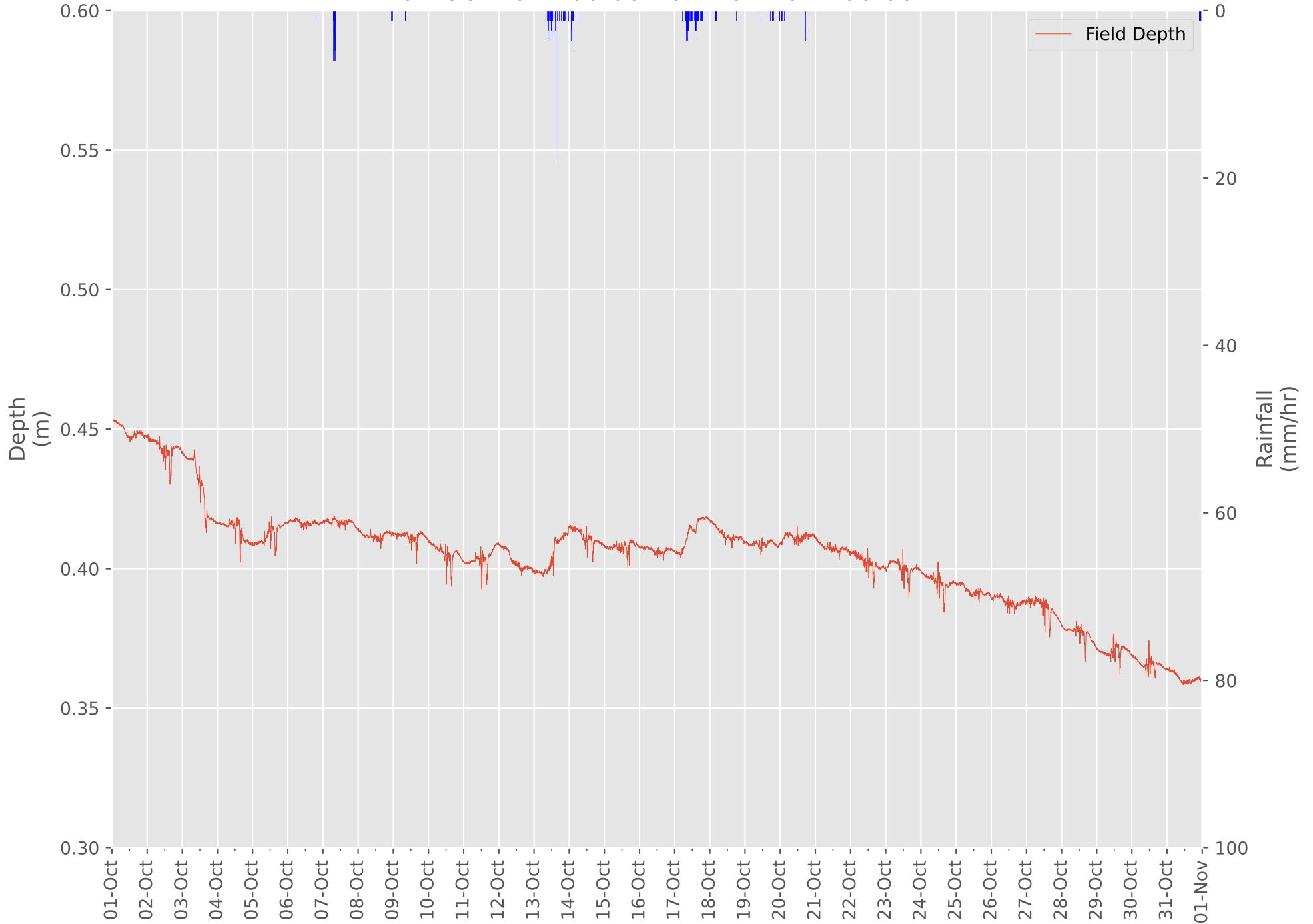
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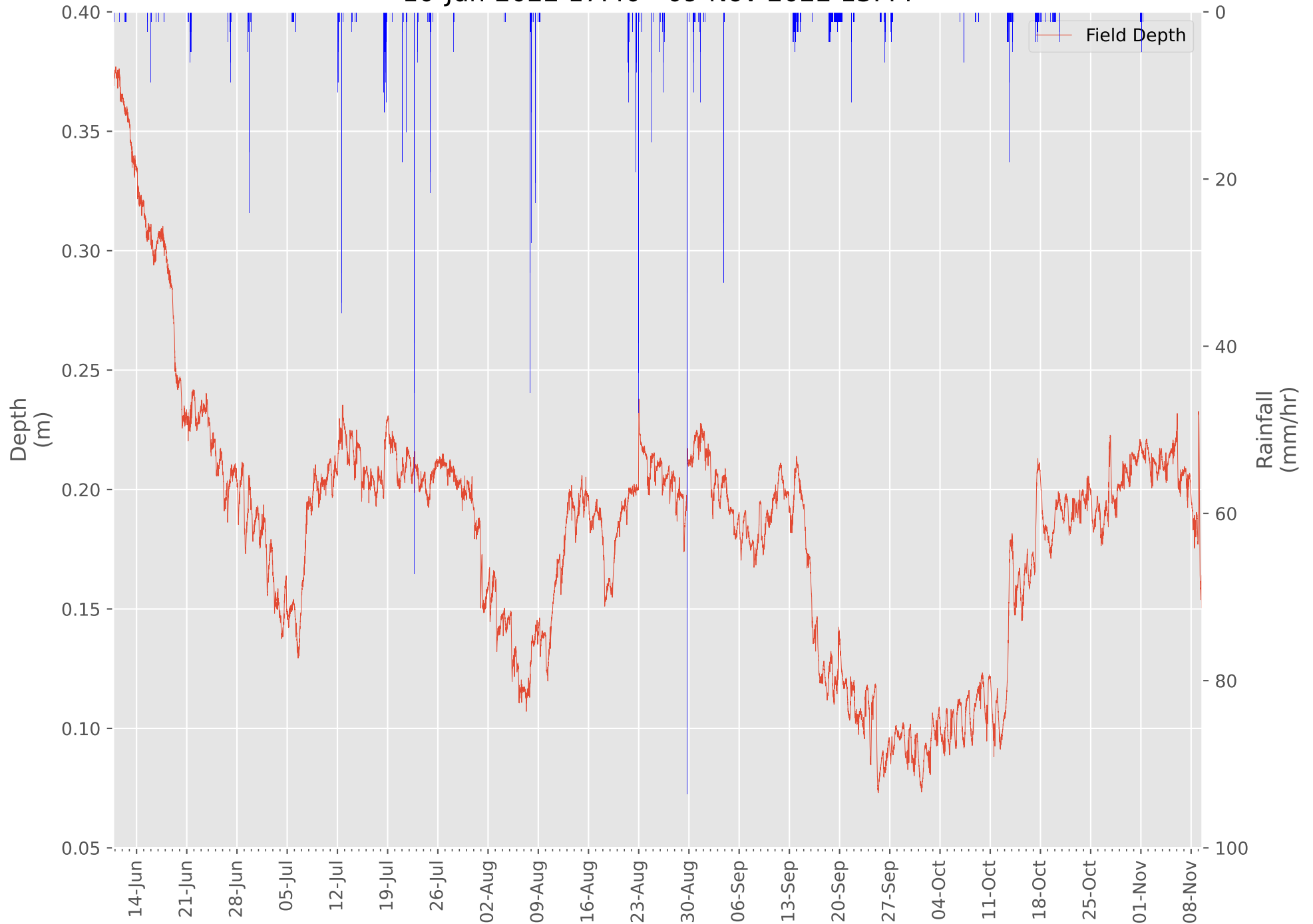
Glen Tay Crossing

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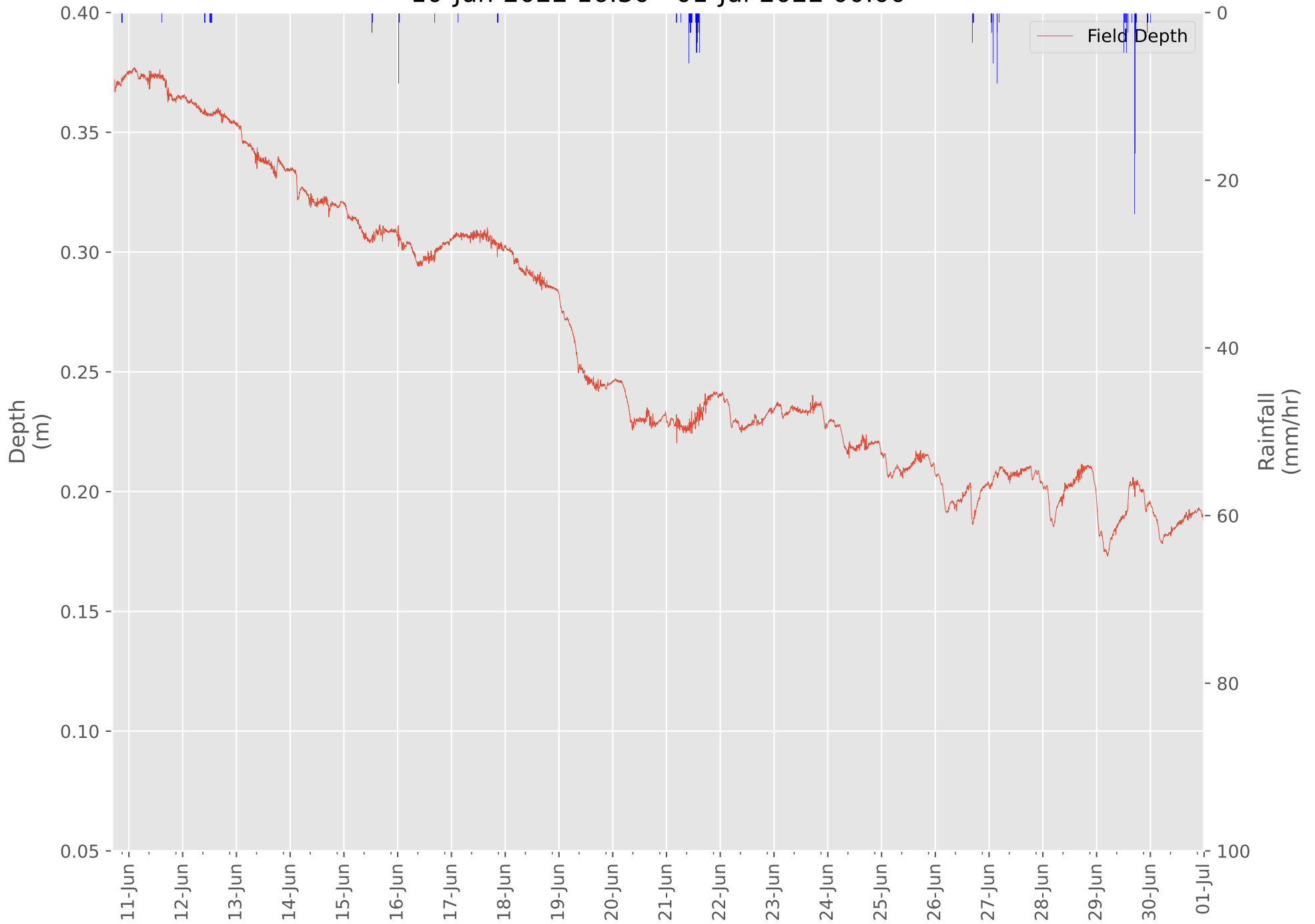
Grants Creek Wetland

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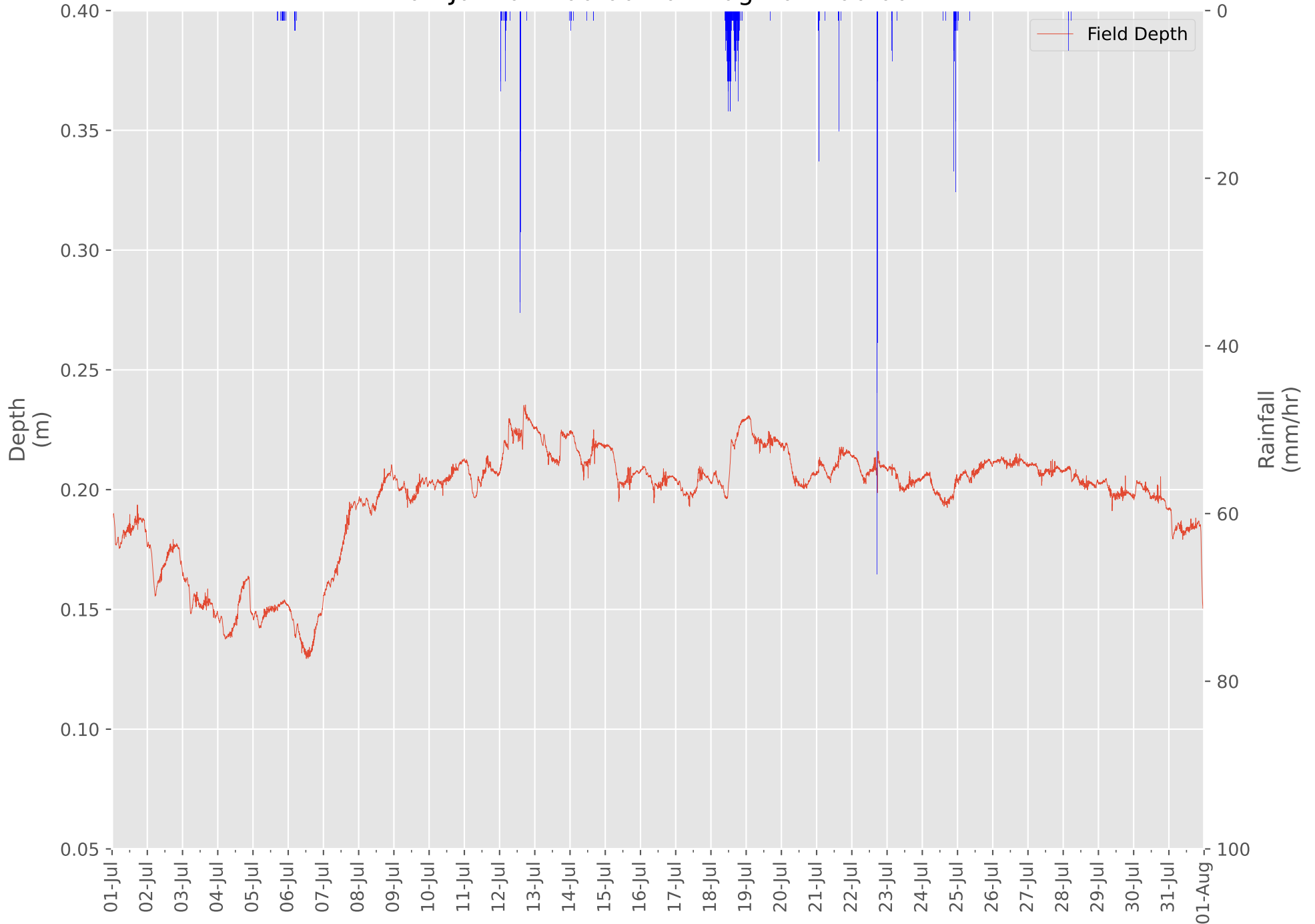
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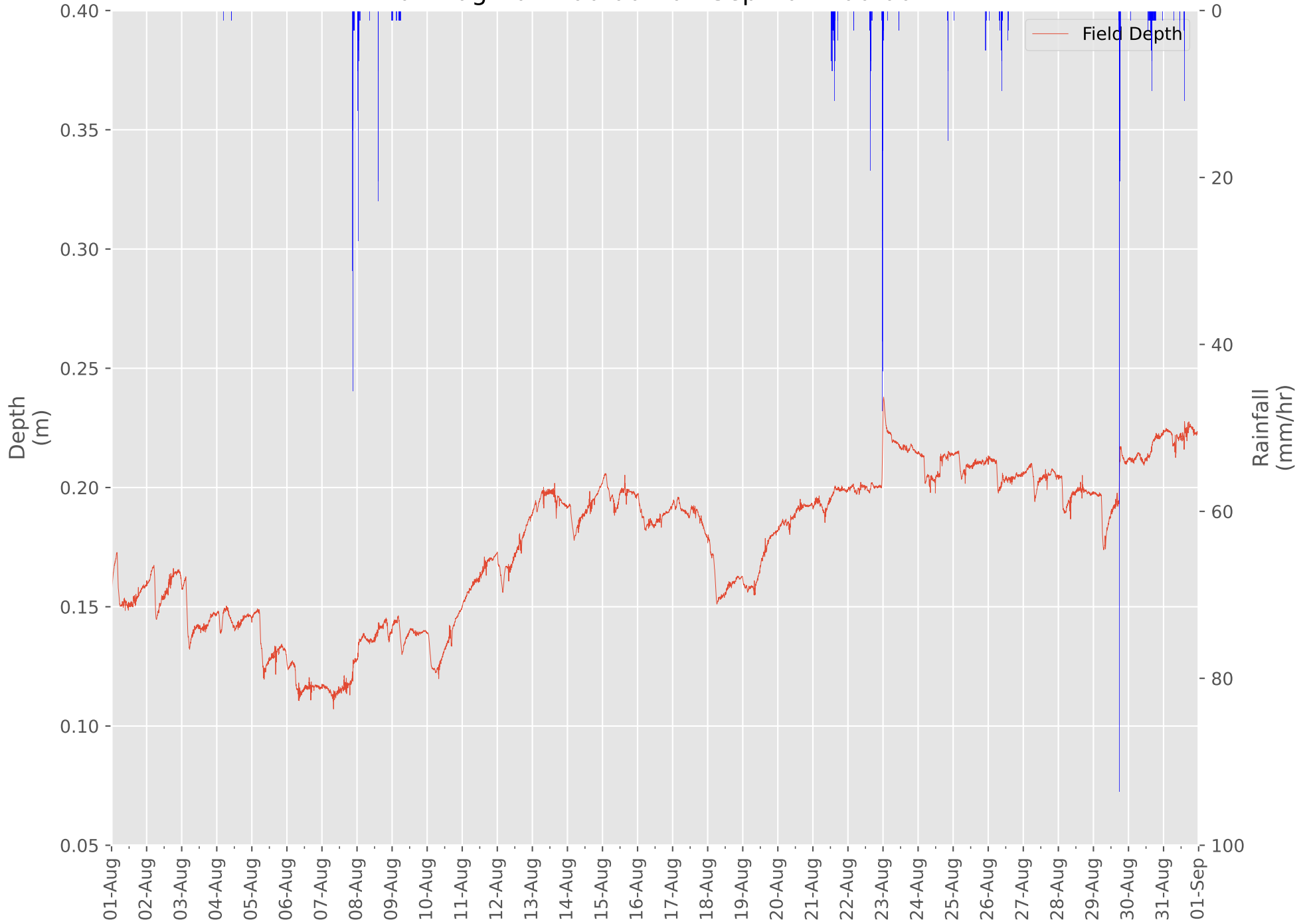
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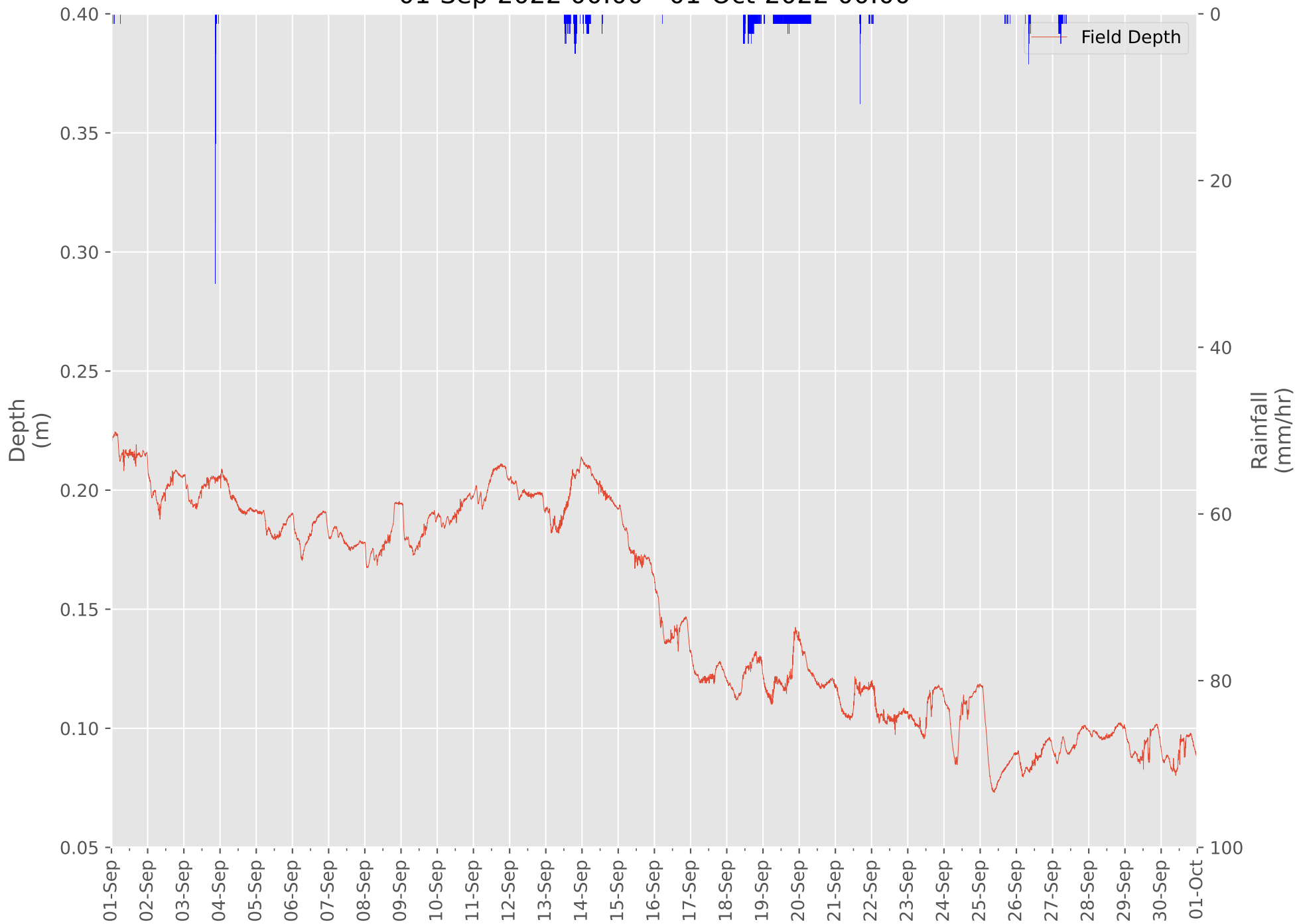
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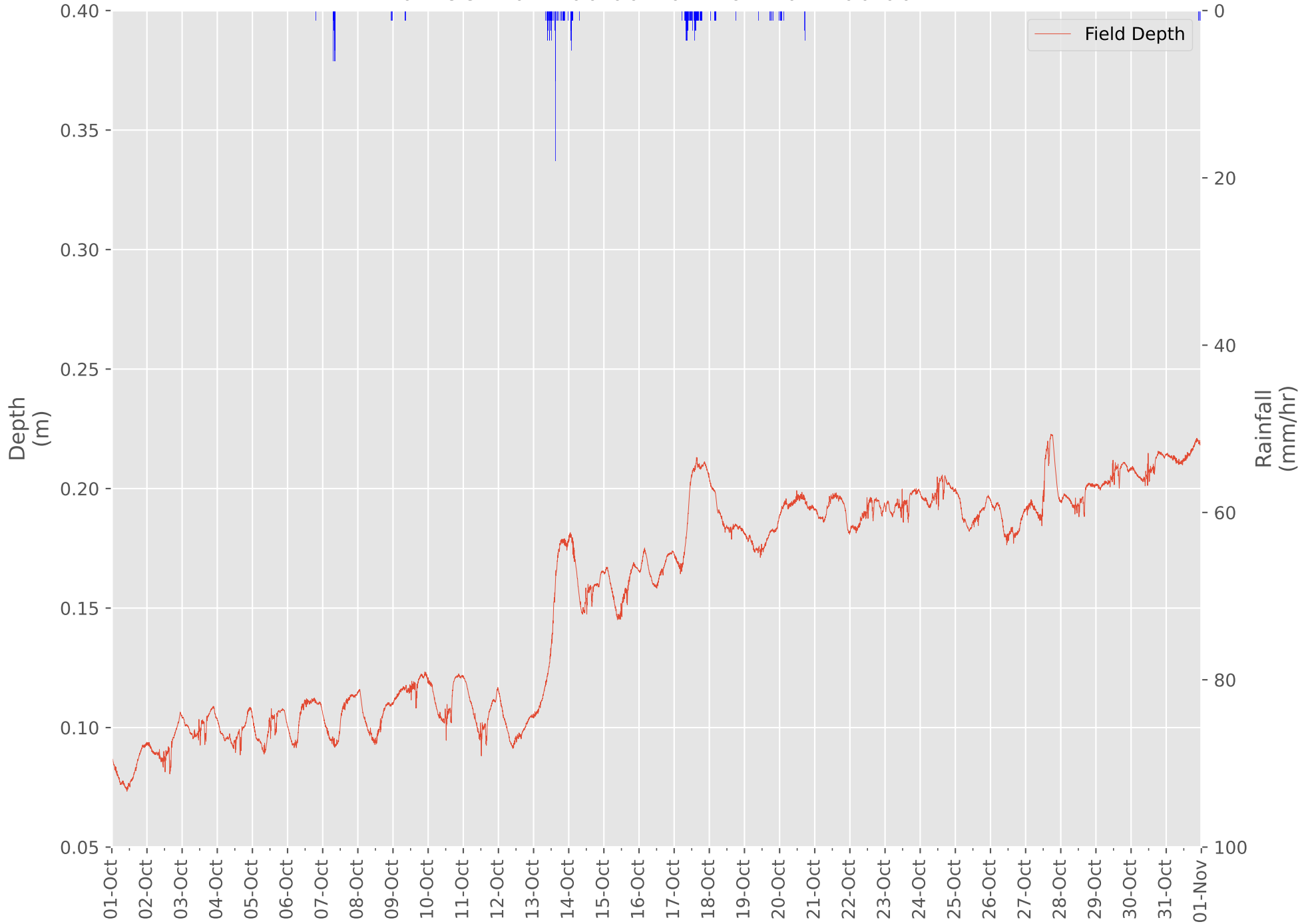
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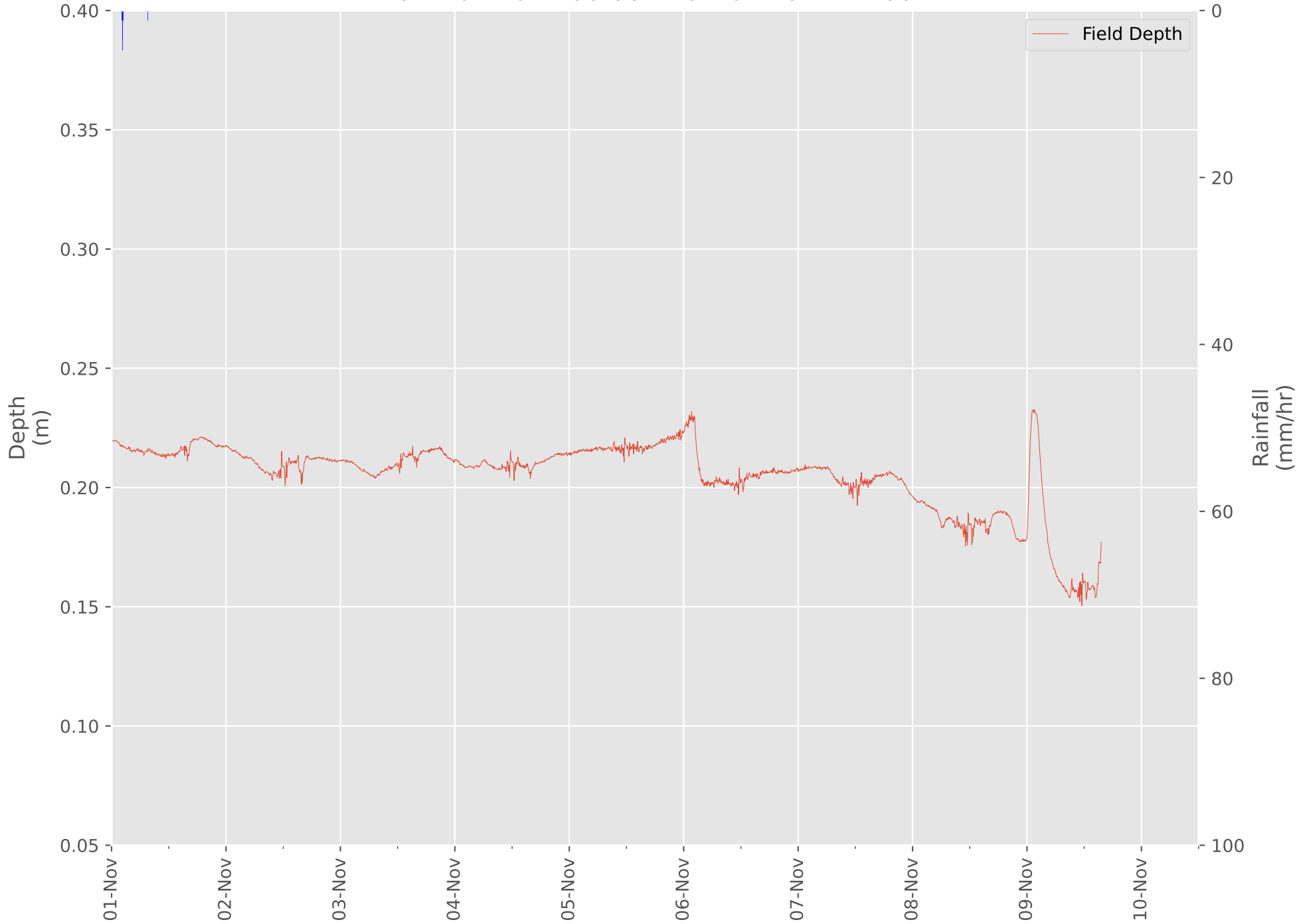
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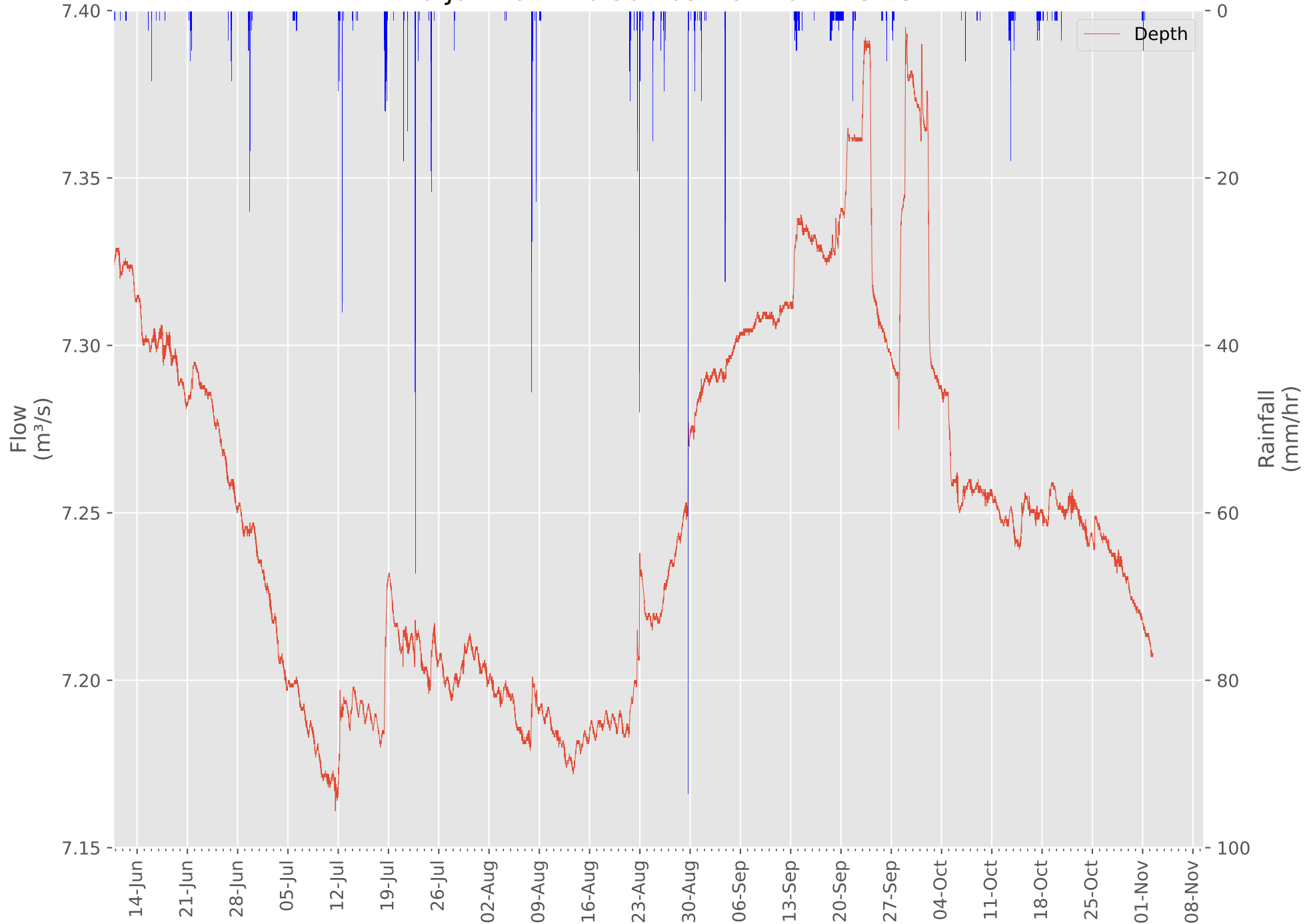
Grants Creek Wetland

01-Nov-2022 00:00 - 10-Nov-2022 12:00



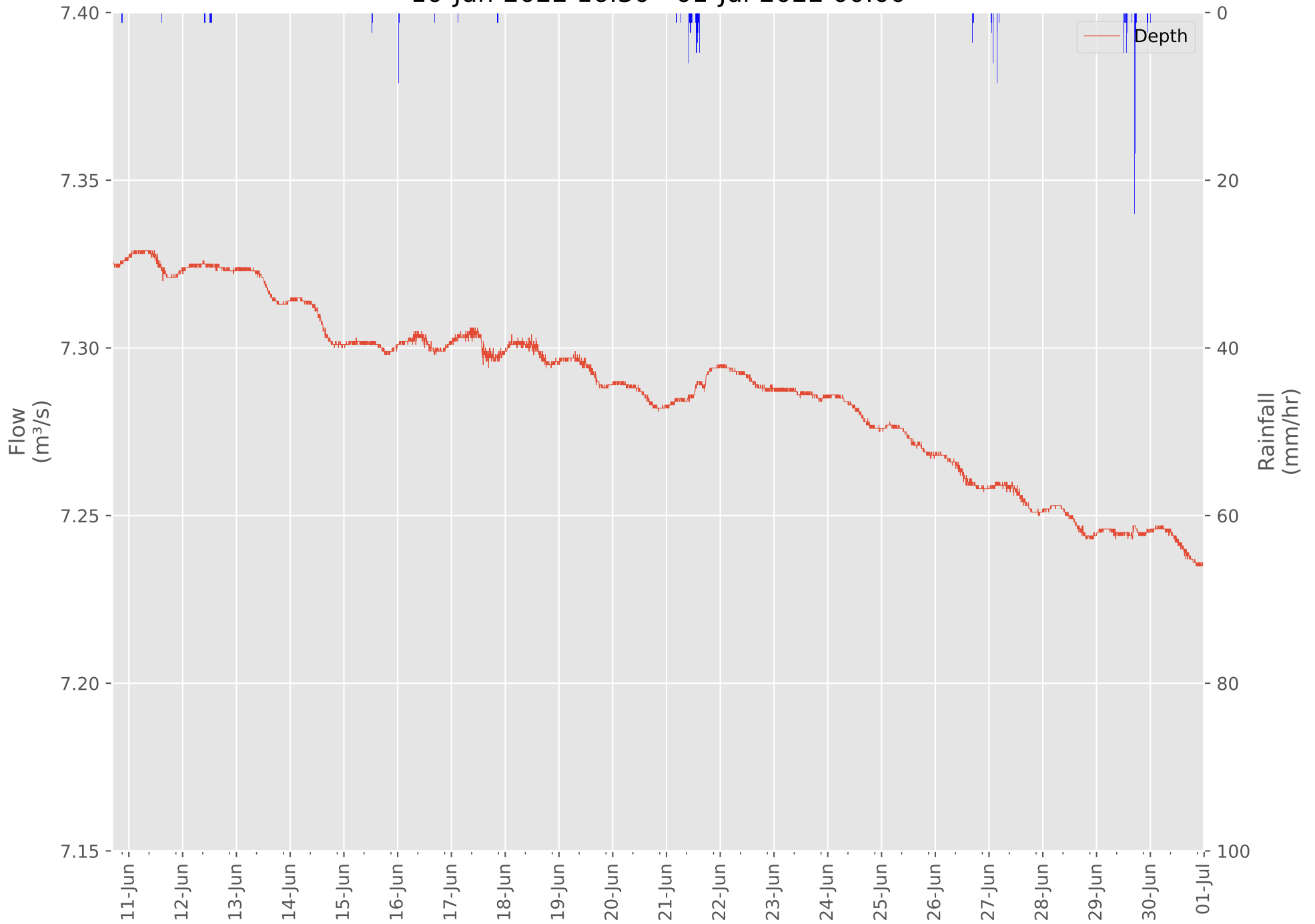
Tay River WSC Gauge Level

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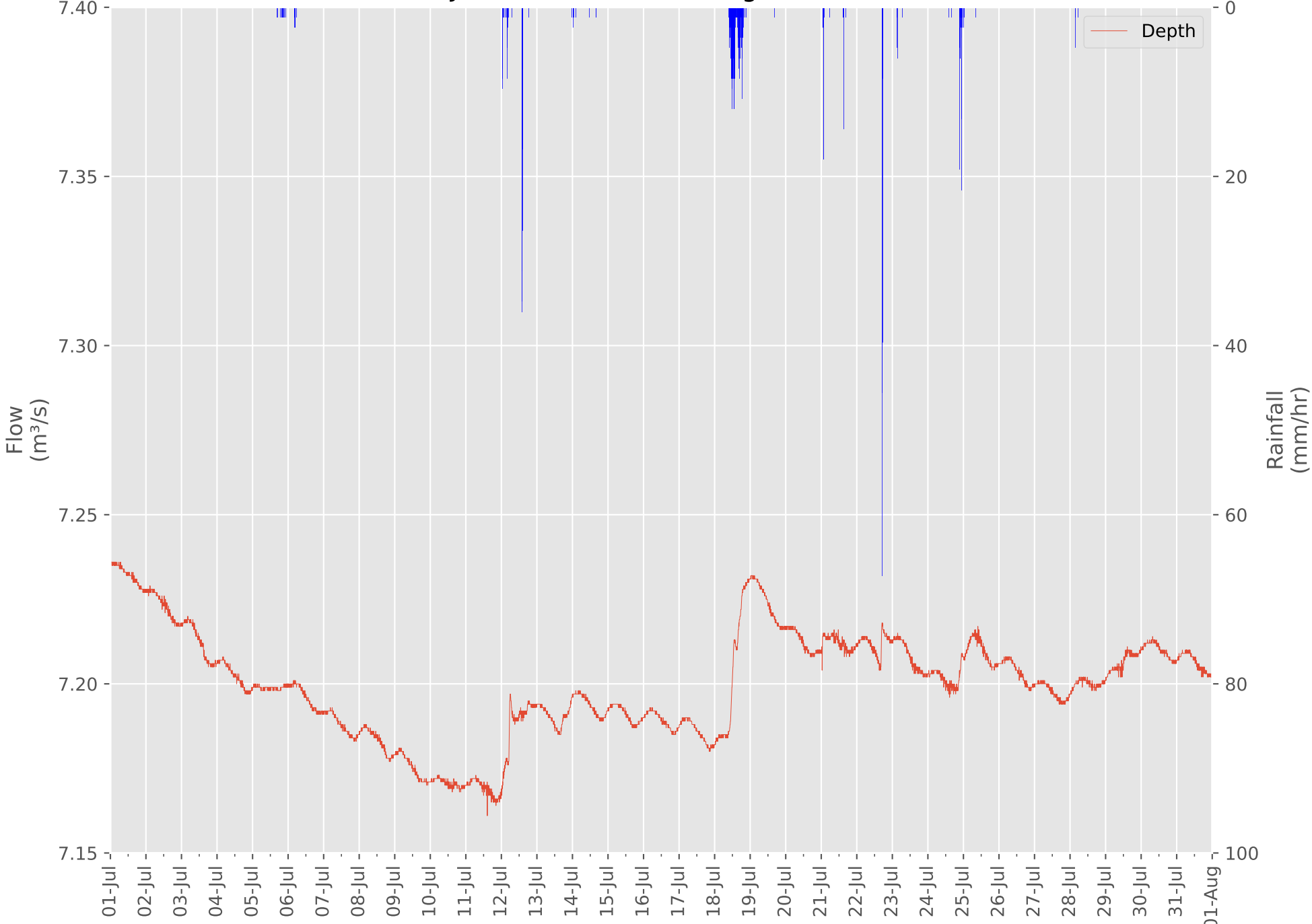


Tay River WSC Gauge Level

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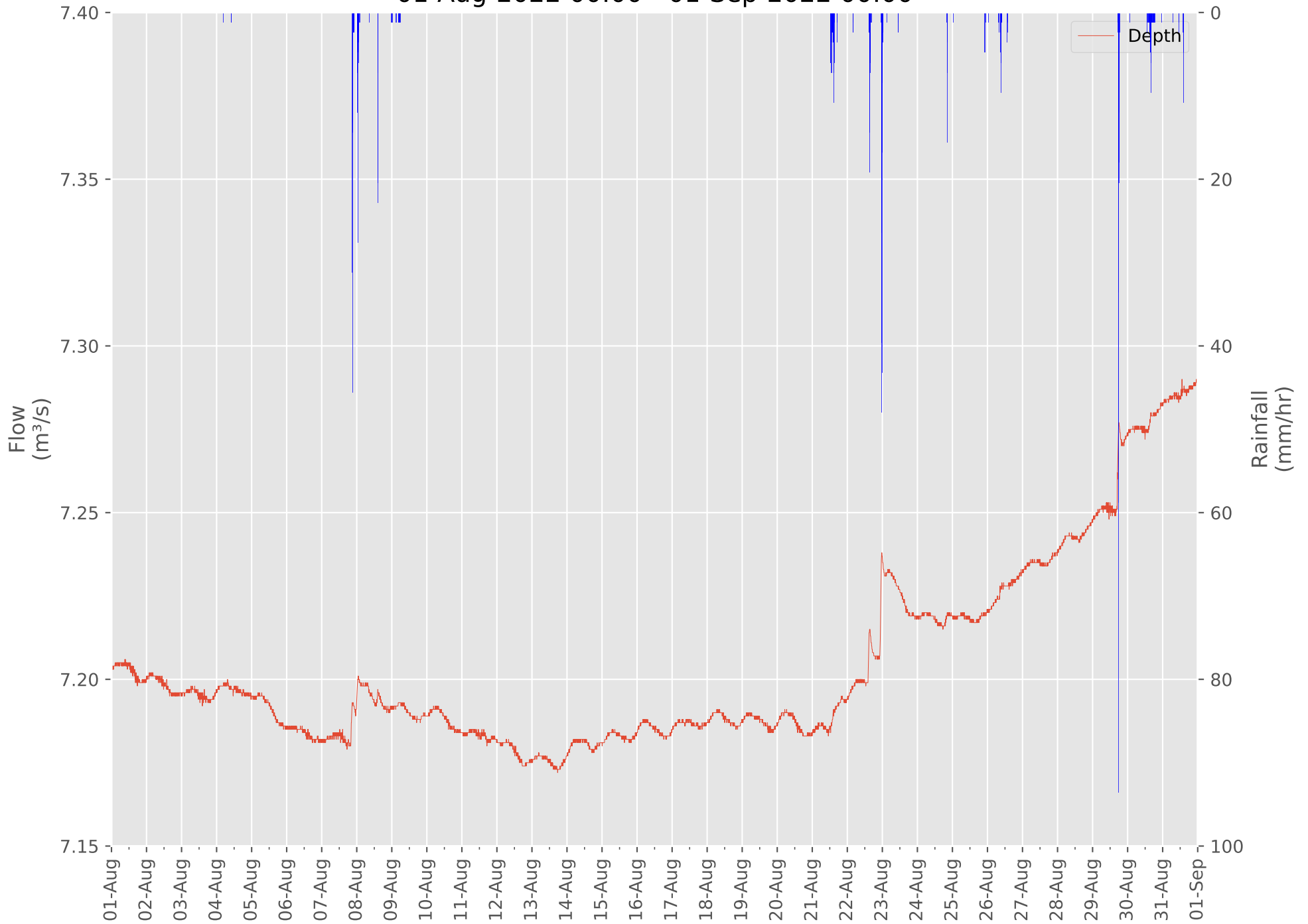


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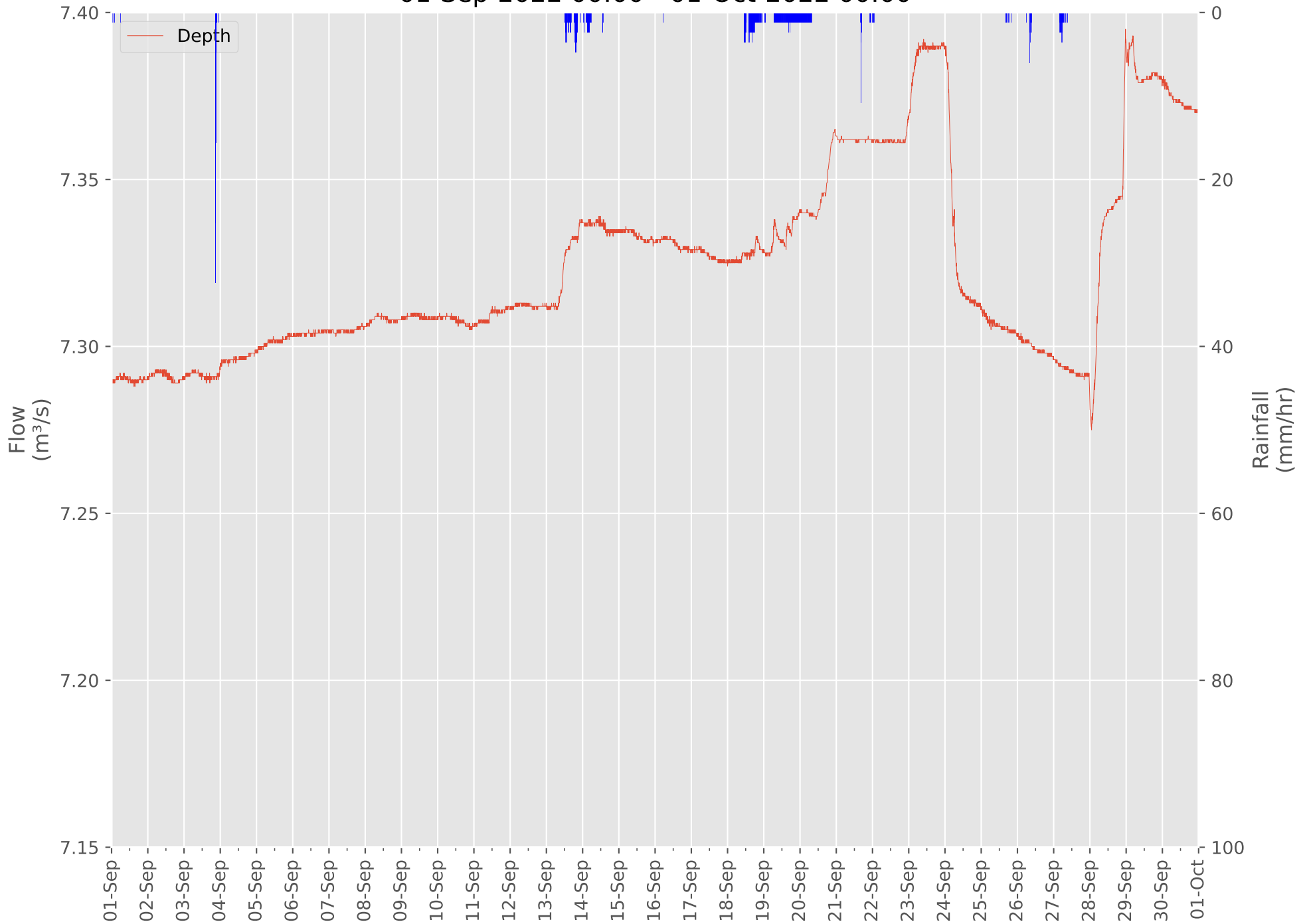
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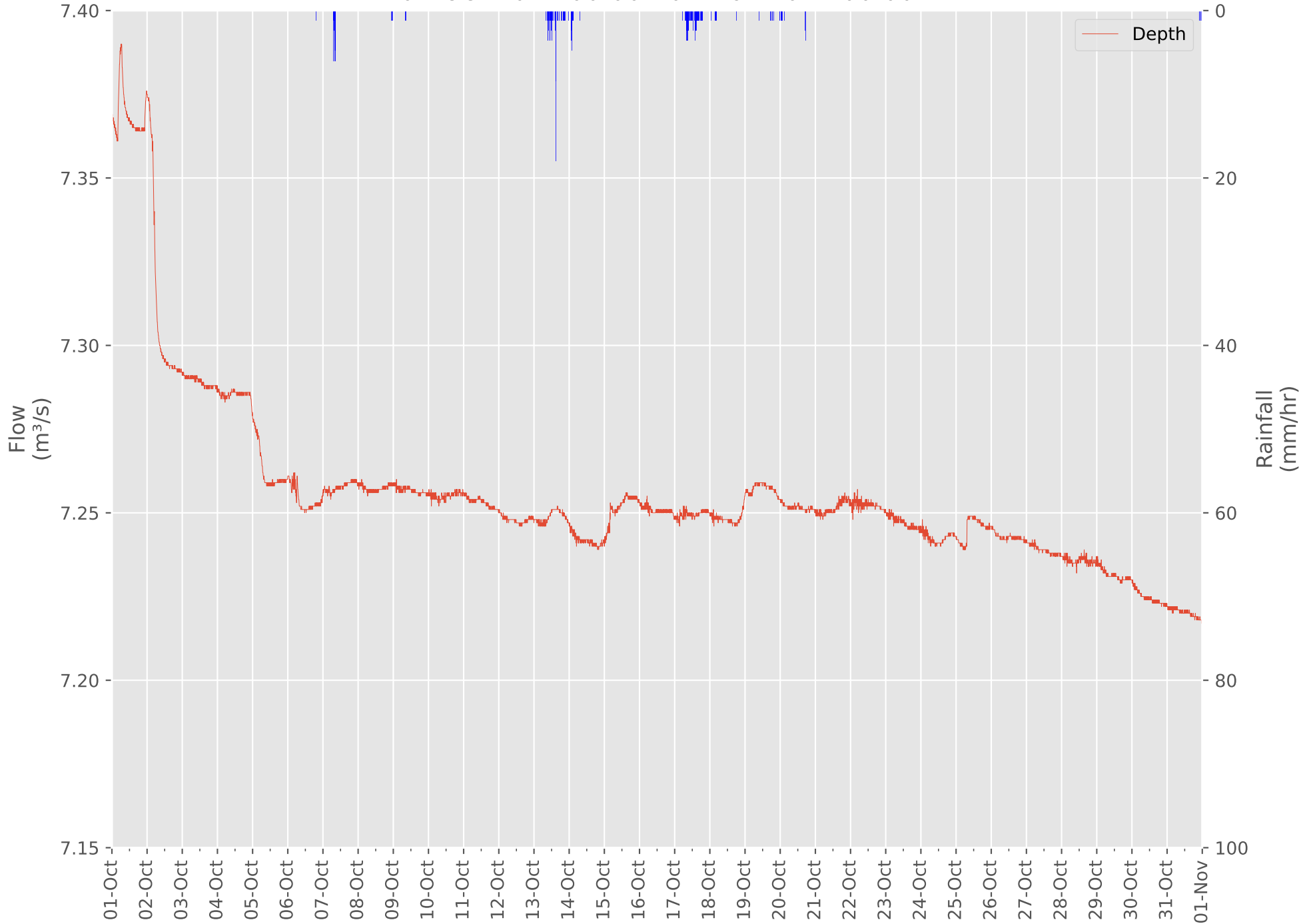
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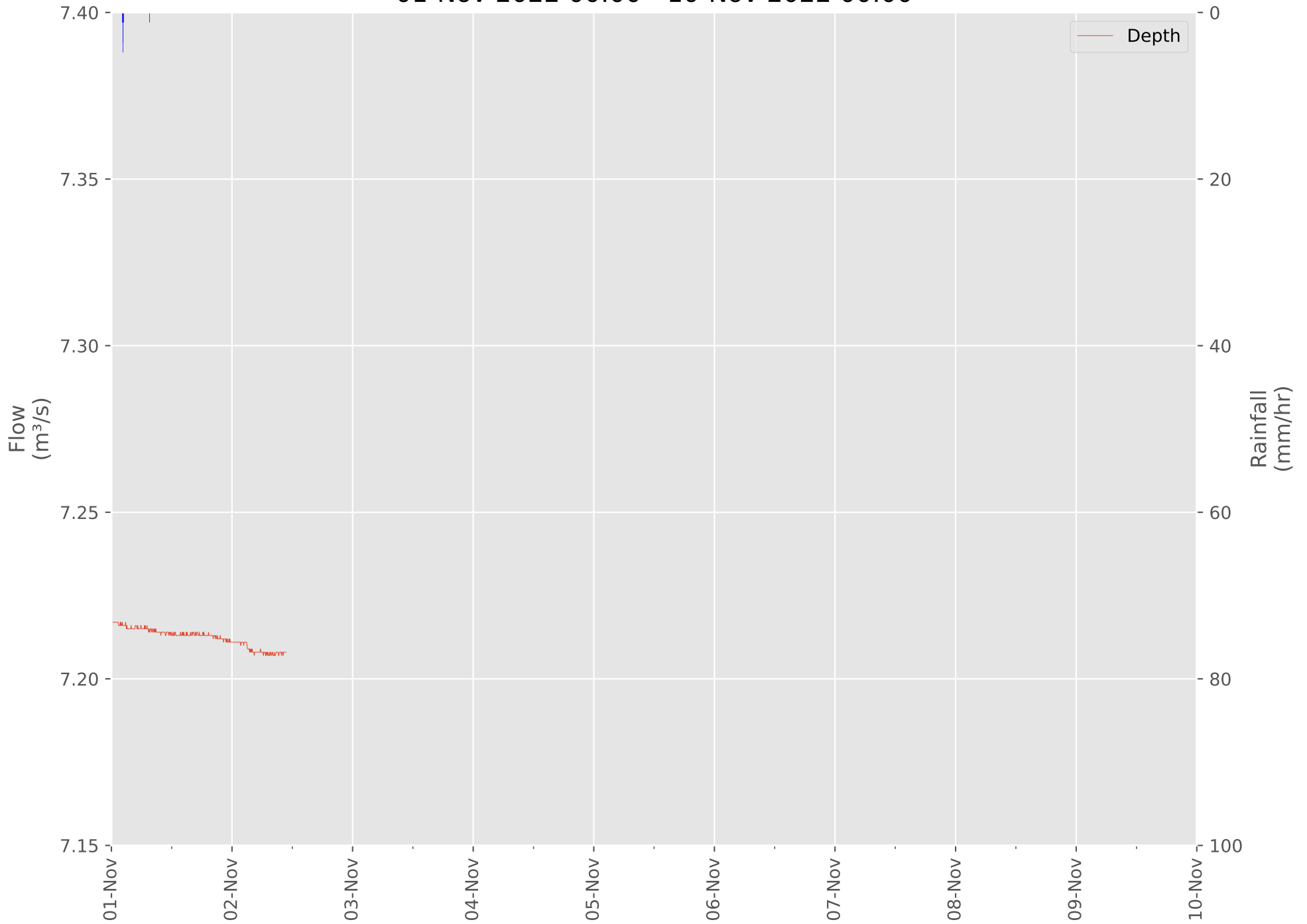
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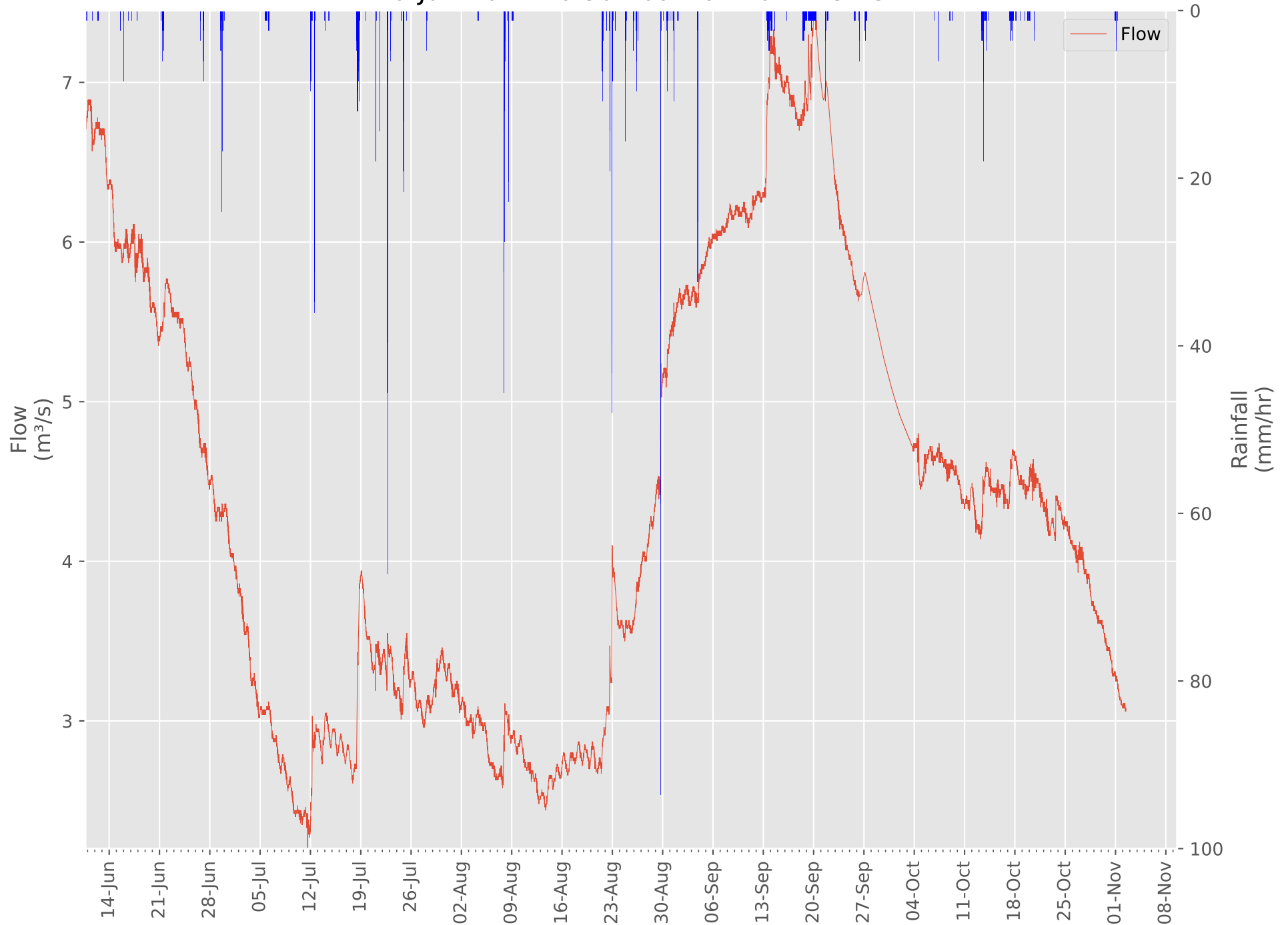
Tay River WSC Gauge Level

01-Nov-2022 00:00 - 10-Nov-2022 00:00



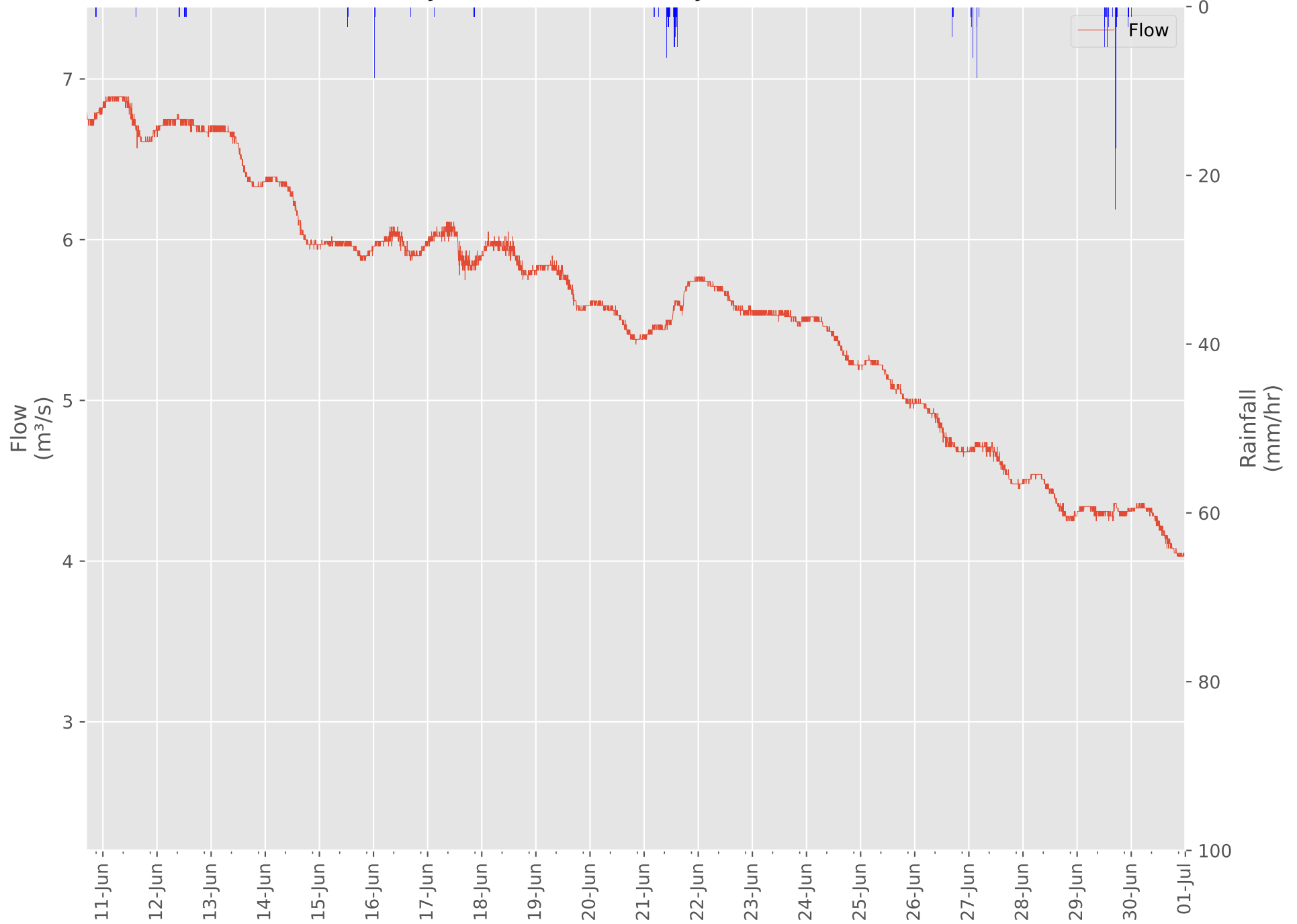
Tay River WSC Gauge Flow

10-Jun-2022 16:30 - 09-Nov-2022 13:45



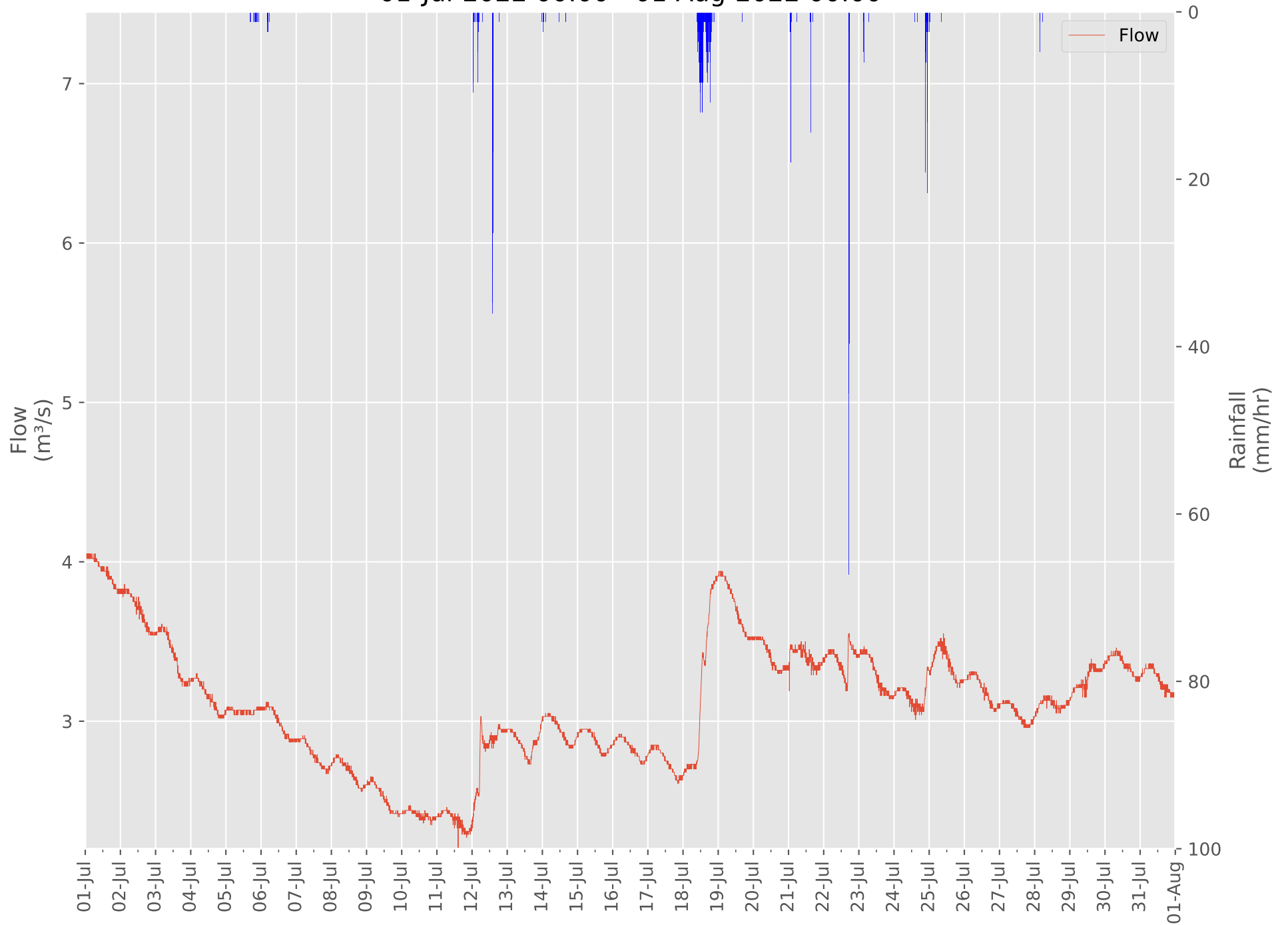
Tay River WSC Gauge Flow

10-Jun-2022 16:30 - 01-Jul-2022 00:00



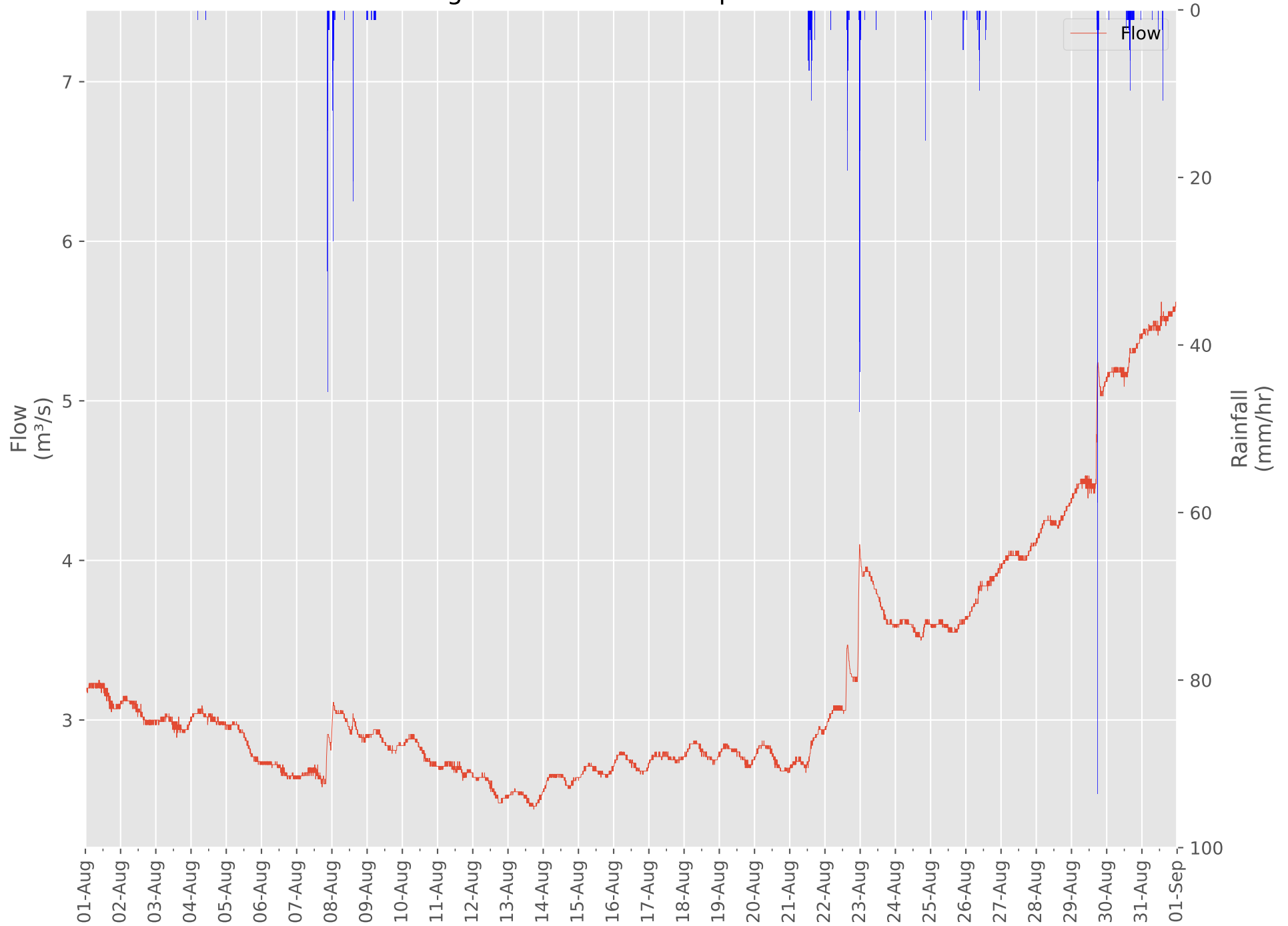
Tay River WSC Gauge Flow

01-Jul-2022 00:00 - 01-Aug-2022 00:00



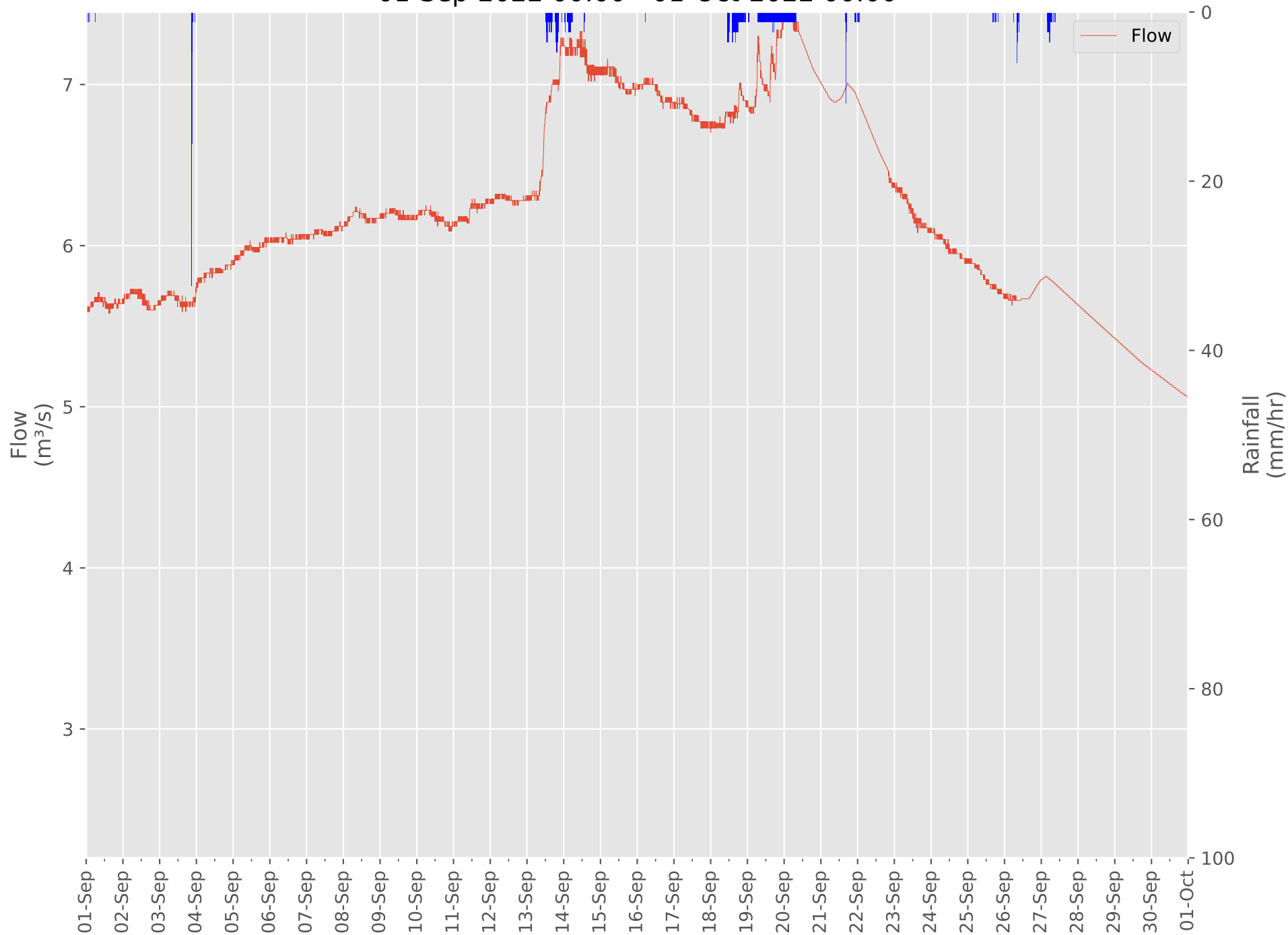
Tay River WSC Gauge Flow

01-Aug-2022 00:00 - 01-Sep-2022 00:00



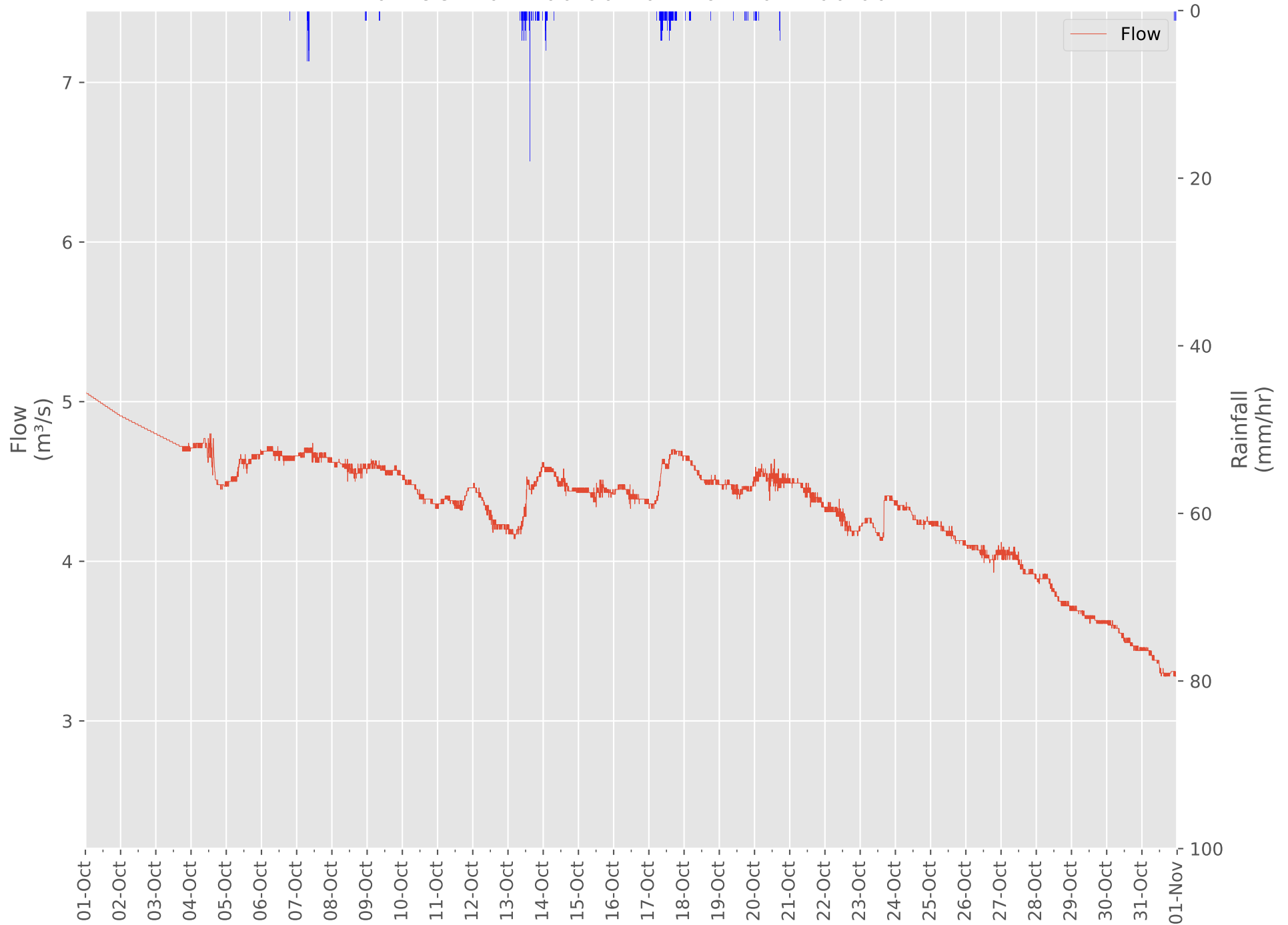
Tay River WSC Gauge Flow

01-Sep-2022 00:00 - 01-Oct-2022 00:00



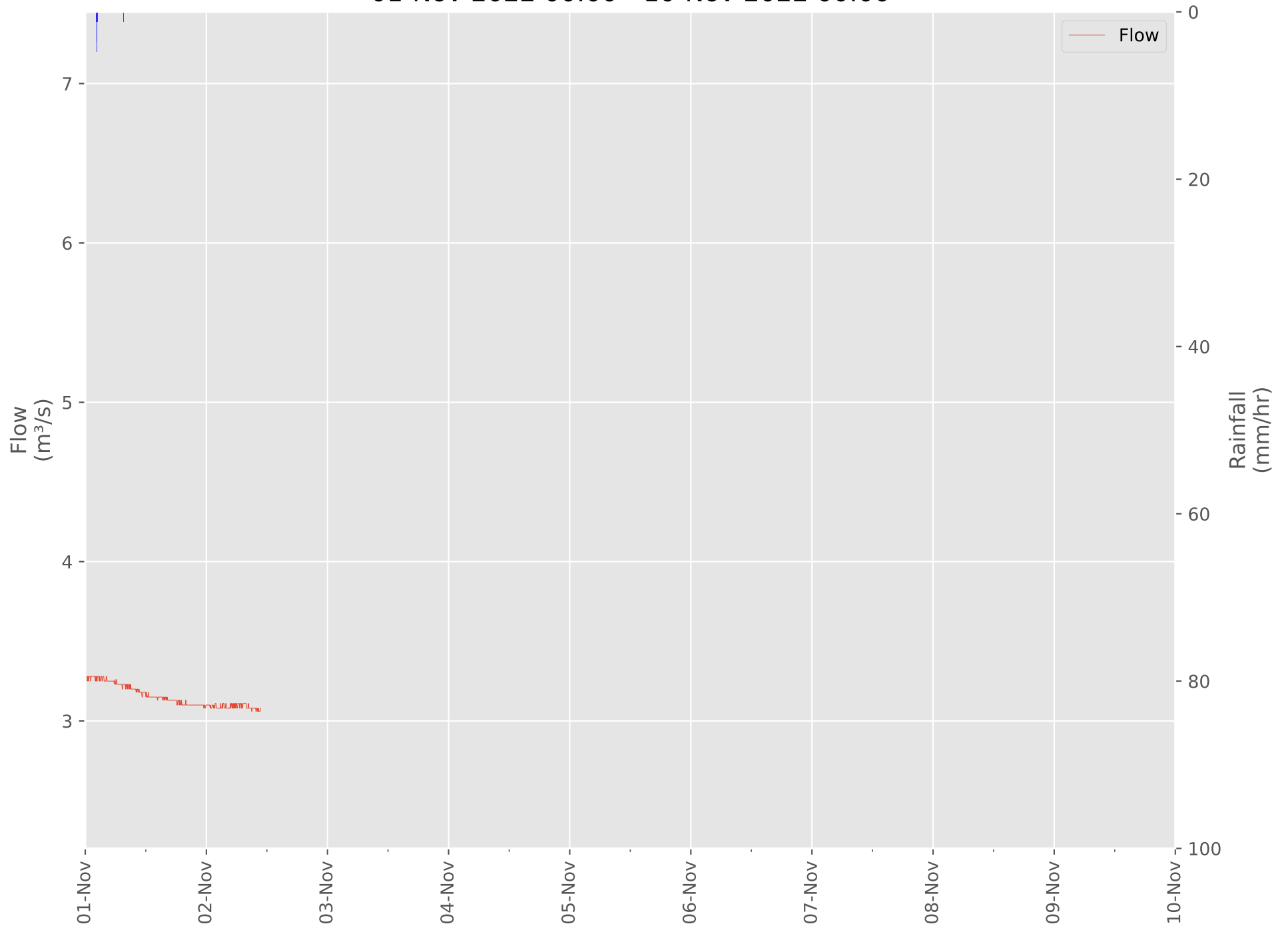
Tay River WSC Gauge Flow

01-Oct-2022 00:00 - 01-Nov-2022 00:00



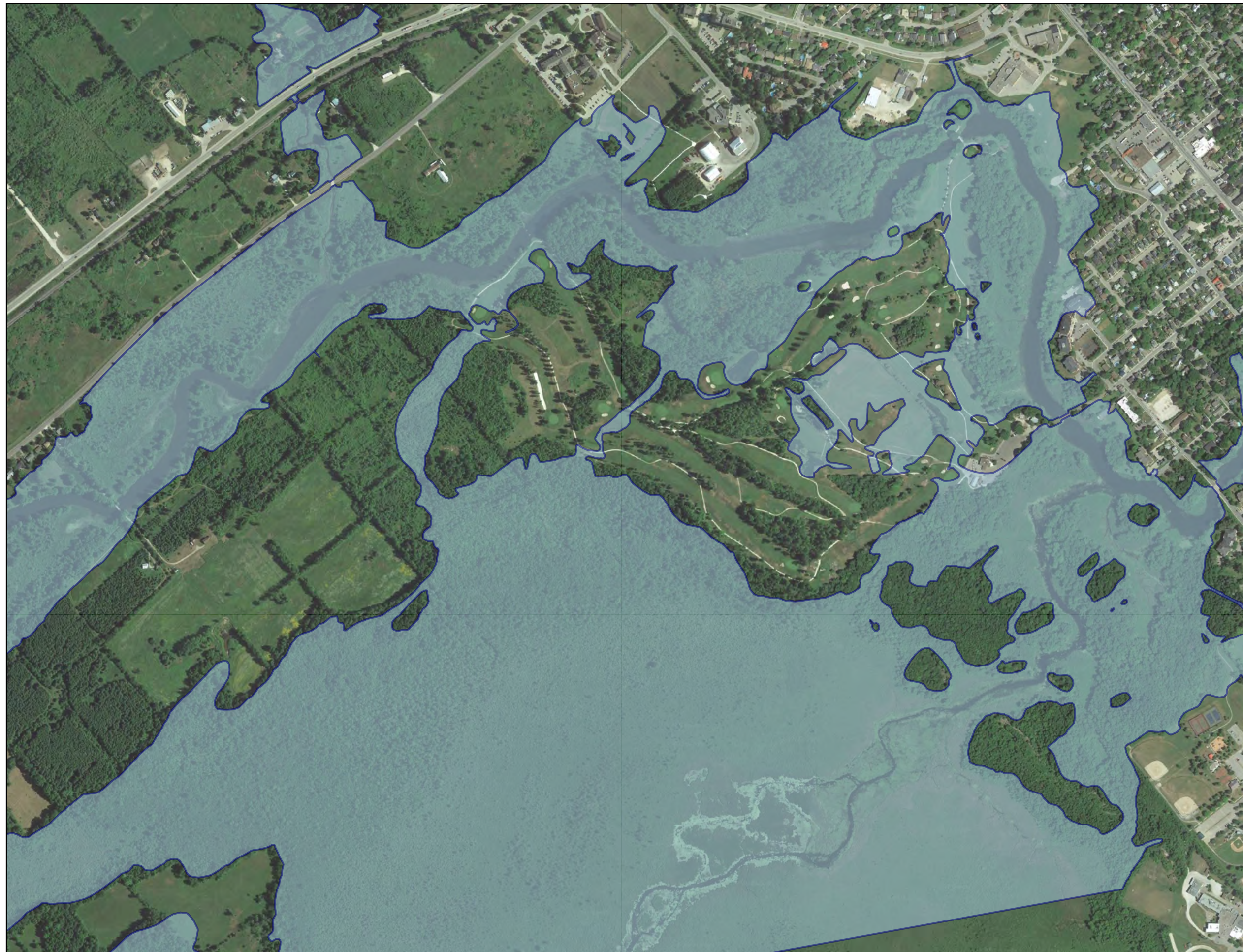
Tay River WSC Gauge Flow

01-Nov-2022 00:00 - 10-Nov-2022 00:00



Appendix C

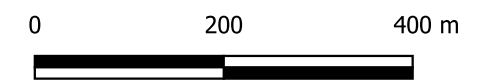
Floodplain Mapping




Legend

 RVCA Floodplain

SCALE: 1:8000



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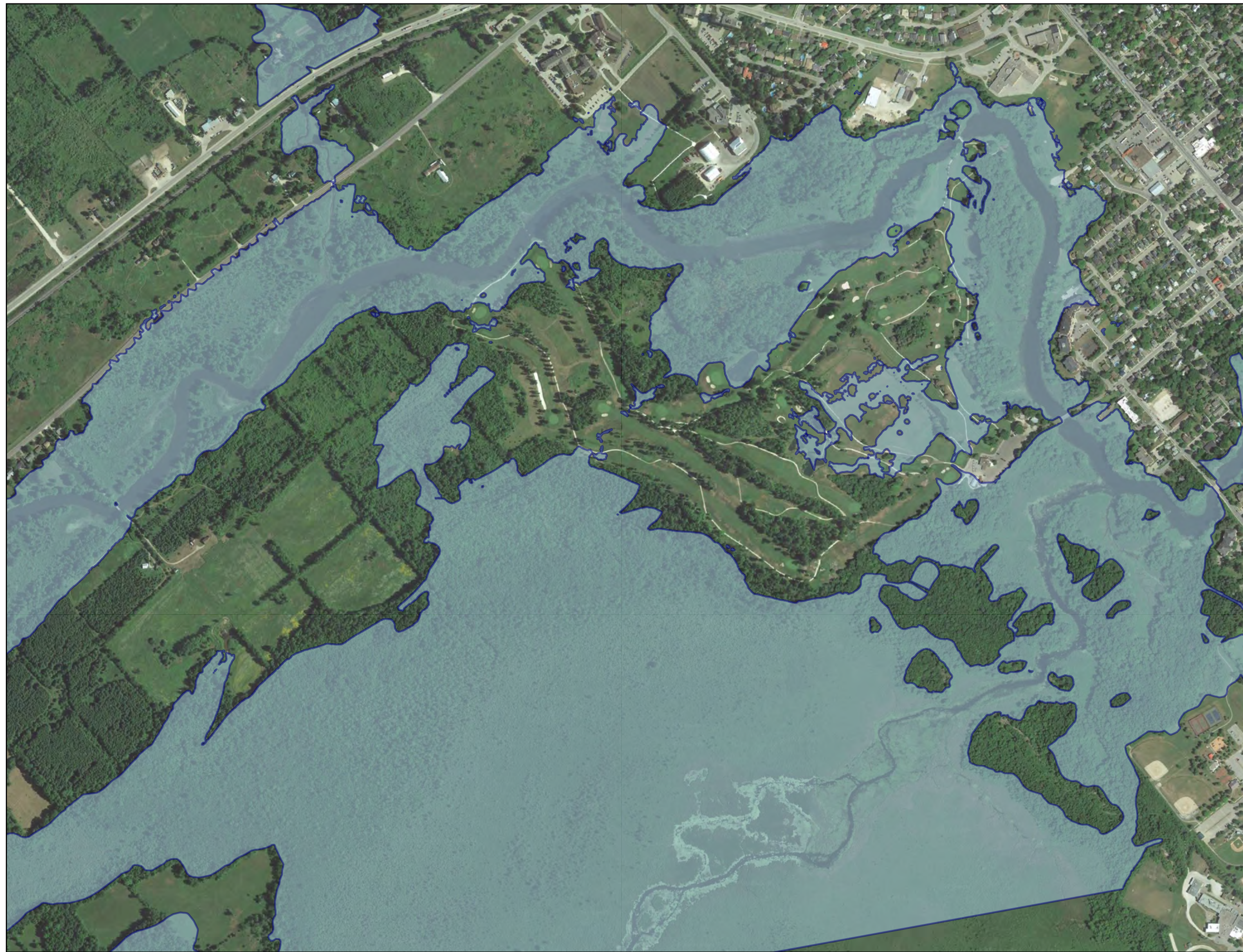
Perth Golf Course

Figure C1: RVCA Floodplain Boundary

PROJECT	2118-21
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DRAWN	MP
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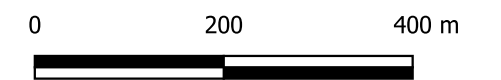
DATE	MAR 2022
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Legend

 2022 JFSA Floodplain

SCALE: 1:8000



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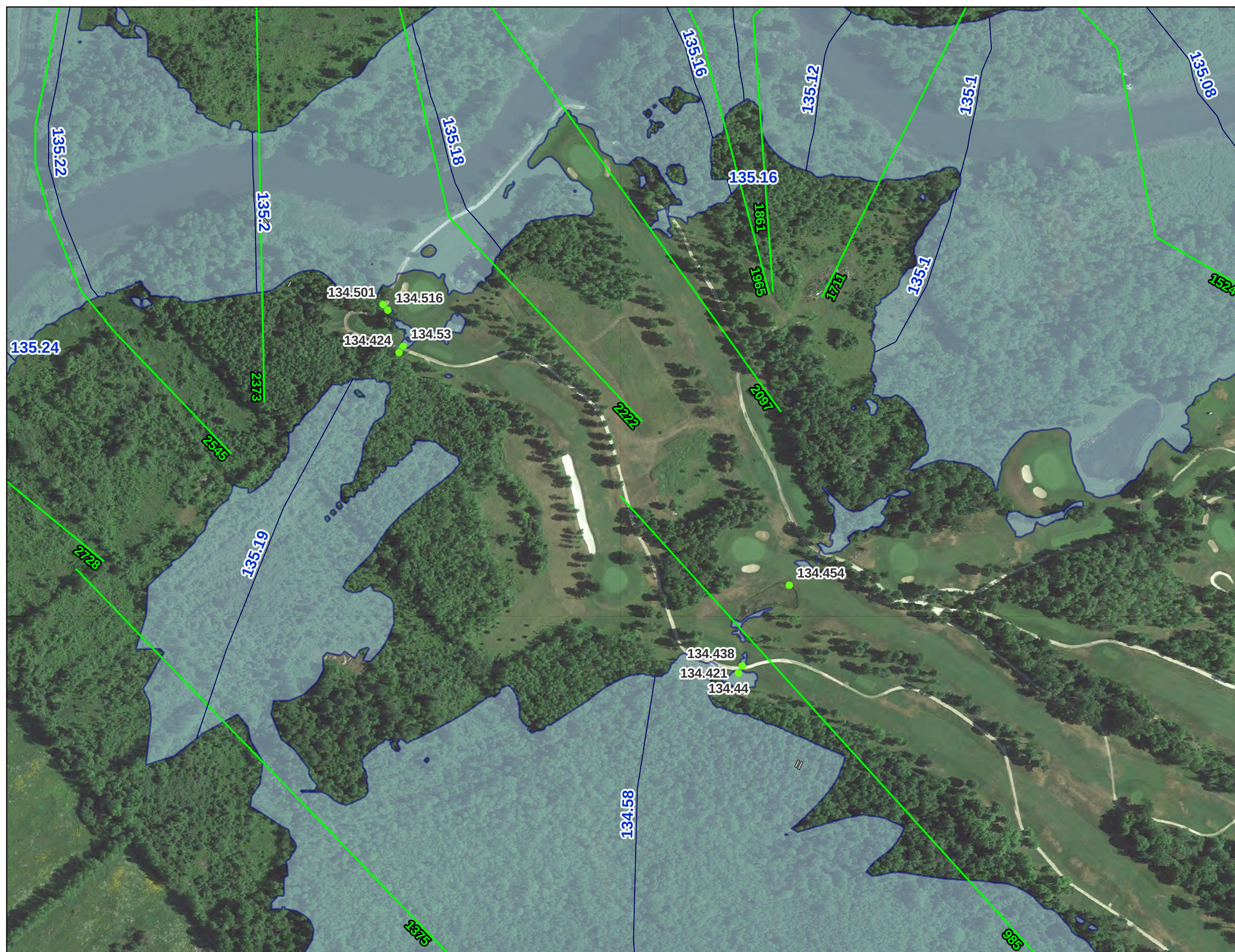
Perth Golf Course

Figure C2: JFSA Floodplain Boundary

PROJECT	2118-21
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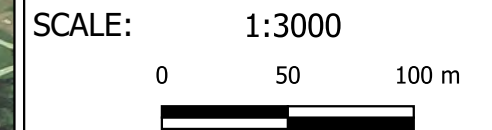
DRAWN	MP
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DATE	MAR 2022
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Legend

- 2022 JFSA Floodplain
- 2 cm Contours (m)
- RVCA Cross Sections
- Culvert Invert Elevation (m)



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Figure C3: JFSA Flood Plain and Culvert Invert Elevations

PROJECT	2118-21
DRAWN	MP
DATE	MAR 2022

Appendix D

Existing Water Budget

Table D1 - Grants Creek - Pre Development Water Budget

Year	[1]	[1]-[2]-[4]		[2]		[3]	
	Precipitation (mm)	Total Evaporation (mm)	(%)	Total Infiltration (mm)	(%)	Total Runoff (mm)	(%)
1967	386.9	200.9	52%	54.3	14%	131.8	34%
1968	592.8	355.2	60%	85.9	14%	151.7	26%
1969	569.8	343.6	60%	84.4	15%	141.9	25%
1970	558.9	348.8	62%	82.1	15%	128.0	23%
1971	522.1	357.3	68%	72.4	14%	92.4	18%
1972	784.3	423.7	54%	118.1	15%	242.6	31%
1973	744.9	424.3	57%	113.8	15%	206.9	28%
1974	386.2	276.8	72%	48.5	13%	60.9	16%
1975	535.5	339.0	63%	78.1	15%	118.3	22%
1976	492.4	332.9	68%	65.6	13%	93.9	19%
1977	677.6	418.9	62%	99.9	15%	158.8	23%
1978	638.8	400.5	63%	110.6	17%	127.7	20%
1979	866.5	454.5	52%	136.8	16%	275.2	32%
1980	622	395.7	64%	91.6	15%	134.7	22%
1981	936.4	525.2	56%	120.4	13%	290.8	31%
1982	596.1	408.4	69%	83.3	14%	104.5	18%
1983	587.3	401.7	68%	73.4	13%	112.2	19%
1984	459.4	262.8	57%	76.5	17%	120.1	26%
1985	559.9	332.9	59%	105.9	19%	121.1	22%
1986	849.4	478.6	56%	117.8	14%	252.9	30%
1987	639.9	418.9	65%	79.1	12%	141.9	22%
1988	643.2	404.0	63%	87.4	14%	151.8	24%
1989	522.5	351.1	67%	70.9	14%	100.6	19%
1990	727.8	455.5	63%	98.7	14%	173.6	24%
1991	555.8	388.9	70%	62.8	11%	104.2	19%
1992	730.2	446.8	61%	103.3	14%	180.1	25%
1993	721.1	469.7	65%	97.4	14%	154.0	21%
1994	527	312.4	59%	81.8	16%	132.8	25%
1995	321.6	161.3	50%	48.6	15%	111.7	35%
1996	512.2	333.2	65%	66.4	13%	112.7	22%
1997	433.2	283.4	65%	72.0	17%	77.8	18%
1998	440.3	287.8	65%	66.9	15%	85.6	19%
1999	424.4	267.4	63%	71.6	17%	85.4	20%
2000	535.9	336.4	63%	76.1	14%	123.5	23%
2002	551.5	273.2	50%	86.7	16%	191.6	35%
2003	554.6	331.3	60%	76.9	14%	146.4	26%
Average	589.1	361.2	62%	85.2	15%	142.8	24%
Min	321.6	161.3	50%	48.5	11%	60.9	16%
Max	936.4	525.2	72%	136.8	19%	290.8	35%

Table D2 - Tay River - Pre Development Water Budget

Year	[1]	[1]-[2]-[4]		[2]		[3]	
	Precipitation (mm)	Total Evaporation (mm)	(%)	Total Infiltration (mm)	(%)	Total Runoff (mm)	(%)
1967	386.9	200.9	52%	60.6	16%	125.4	32%
1968	592.8	355.2	60%	95.2	16%	142.5	24%
1969	569.8	343.6	60%	93.4	16%	132.8	23%
1970	558.9	348.8	62%	90.5	16%	119.7	21%
1971	522.1	357.3	68%	78.9	15%	85.9	16%
1972	784.3	423.7	54%	131.3	17%	229.4	29%
1973	744.9	424.3	57%	126.5	17%	194.1	26%
1974	386.2	276.8	72%	52.8	14%	56.6	15%
1975	535.5	339.0	63%	85.8	16%	110.7	21%
1976	492.4	332.9	68%	72.0	15%	87.5	18%
1977	677.6	418.9	62%	109.9	16%	148.8	22%
1978	638.8	400.5	63%	120.6	19%	117.7	18%
1979	866.5	454.5	52%	152.2	18%	259.8	30%
1980	622	395.7	64%	100.3	16%	126.0	20%
1981	936.4	525.2	56%	134.0	14%	277.1	30%
1982	596.1	408.4	69%	90.7	15%	97.1	16%
1983	587.3	401.7	68%	80.7	14%	105.0	18%
1984	459.4	262.8	57%	84.3	18%	112.3	24%
1985	559.9	332.9	59%	115.6	21%	111.4	20%
1986	849.4	478.6	56%	130.7	15%	240.1	28%
1987	639.9	418.9	65%	87.2	14%	133.8	21%
1988	643.2	404.0	63%	96.8	15%	142.4	22%
1989	522.5	351.1	67%	77.8	15%	93.6	18%
1990	727.8	455.5	63%	109.2	15%	163.1	22%
1991	555.8	388.9	70%	69.3	12%	97.6	18%
1992	730.2	446.8	61%	114.3	16%	169.1	23%
1993	721.1	469.7	65%	107.2	15%	144.2	20%
1994	527	312.4	59%	90.7	17%	123.9	24%
1995	321.6	161.3	50%	53.8	17%	106.5	33%
1996	512.2	333.2	65%	73.2	14%	105.9	21%
1997	433.2	283.4	65%	78.2	18%	71.6	17%
1998	440.3	287.8	65%	73.2	17%	79.4	18%
1999	424.4	267.4	63%	78.3	18%	78.7	19%
2000	535.9	336.4	63%	83.8	16%	115.7	22%
2002	551.5	273.2	50%	96.3	17%	182.0	33%
2003	554.6	331.3	60%	85.0	15%	138.3	25%
Average	589.1	361.2	62%	93.9	16%	134.0	22%
Min	321.6	161.3	50%	52.8	12%	56.6	15%
Max	936.4	525.2	72%	152.2	21%	277.1	33%

```

20 Metric units / ID Numbers OFF
21 *#*****
22 *# SWMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
23 *#*****
24 *# Project Name : [Caivan Perth properties]
25 *# Project Number: [2118]
26 *# Date : [2023 JAN 26]
27 *# Modeller : [JB]
28 *# Company : J.F. Sabourin and Associates
29 *# License # : 2549237
30 *#*****
31 *#*****
32 *# Model developed to simulate pre-development water budget
33 *#*****
34
35 START TZERO=[1967.0101], METOUT=[2], NSTORM=[0], NRUN=[67]
36 *% [ " ] <--storm filename, one per line for NSTORM time
37 *%-----|-----
38 *# Ottawa International Airport (1967 - 2003)
39 READ AES DATA AES_FILENAME=[ "6106000.123" ],
40 IELEM=[123], START_DATE=[0], END_DATE=[-364]
41 *%-----|-----
42 COMPUTE API APII=[50], APIK=[0.90]/day
43 *#####
44 *# Pre Development Condition - Using NASHHYD and CN
45 *#####
46 CONTINUOUS NASHYD NHYD=[ "GrantsPre" ], DT=[5]min, AREA=[22.01](ha),
47 DWF=[0](cms), CN/C=[92], IA=[8.75](mm),
48 N=[3], TP=[0.24]hrs,
49 Continuous simulation parameters:
50 IaRECper=[24](hrs),SMIN=[ -1 ](mm), SMAX=[ -1 ](mm),
51 SK=[0.3]/(mm), InterEventTime=[ 12 ](hrs)
52 END=-1
53 *%-----|-----
54
55 CONTINUOUS NASHYD NHYD=[ "TayPre" ], DT=[5]min, AREA=[22.86](ha),
56 DWF=[0](cms), CN/C=[90], IA=[8.75](mm),
57 N=[3], TP=[0.36]hrs,
58 Continuous simulation parameters:
59 IaRECper=[24](hrs),SMIN=[ -1 ](mm), SMAX=[ -1 ](mm),
60 SK=[0.3]/(mm), InterEventTime=[ 12 ](hrs)
61 END=-1
62 *%-----|-----
63 *ADD HYD NHYDsum=[ "Pre" ], NHYDs to add=[ "GrantsPre"+"TayPre" ]
64 *#####
65 *# Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
66 *#####
67 CONTINUOUS NASHYD NHYD=[ "InfGrantsPre" ], DT=[5]min, AREA=[22.01](ha),
68 DWF=[0](cms), CN/C=[99.99], IA=[8.75](mm),
69 N=[3], TP=[0.24]hrs,
70 Continuous simulation parameters:
71 IaRECper=[24](hrs),SMIN=[ 0 ](mm), SMAX=[ 0 ](mm), SK=[0.3]/(mm),
72 InterEventTime=[ 12 ](hrs)
73 END=-1
74 *%-----|-----
75 CONTINUOUS NASHYD NHYD=[ "InfTayPre" ], DT=[5]min, AREA=[22.86](ha),
76 DWF=[0](cms), CN/C=[99.99], IA=[8.75](mm),
77 N=[3], TP=[0.36]hrs,
78 Continuous simulation parameters:
79 IaRECper=[24](hrs),SMIN=[ 0 ](mm), SMAX=[ 0 ](mm), SK=[0.3]/(mm),
80 InterEventTime=[ 12 ](hrs)
81 END=-1
82 *%-----|-----
83 *ADD HYD NHYDsum=[ "InfPre" ], NHYDs to add=[ "InfGrantsPre"+"InfTayPre" ]
84 *%-----|-----
85 *#####
86 *# STORMS
87 *#####

```

```

64  START           TZERO=[1968.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[68]
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66  START           TZERO=[1969.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[69]
67  *%-----|-----|-----|-----|-----|-----|-----|-----|
68  START           TZERO=[1970.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[70]
69  *%-----|-----|-----|-----|-----|-----|-----|-----|
70  START           TZERO=[1971.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[71]
71  *%-----|-----|-----|-----|-----|-----|-----|-----|
72  START           TZERO=[1972.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[72]
73  *%-----|-----|-----|-----|-----|-----|-----|-----|
74  START           TZERO=[1973.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[73]
75  *%-----|-----|-----|-----|-----|-----|-----|-----|
76  START           TZERO=[1974.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[74]
77  *%-----|-----|-----|-----|-----|-----|-----|-----|
78  START           TZERO=[1975.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[75]
79  *%-----|-----|-----|-----|-----|-----|-----|-----|
80  START           TZERO=[1976.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[76]
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82  START           TZERO=[1977.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[77]
83  *%-----|-----|-----|-----|-----|-----|-----|-----|
84  START           TZERO=[1978.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[78]
85  *%-----|-----|-----|-----|-----|-----|-----|-----|
86  START           TZERO=[1979.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[79]
87  *%-----|-----|-----|-----|-----|-----|-----|-----|
88  START           TZERO=[1980.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[80]
89  *%-----|-----|-----|-----|-----|-----|-----|-----|
90  START           TZERO=[1981.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[81]
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92  START           TZERO=[1982.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[82]
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96  START           TZERO=[1984.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[84]
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104 START           TZERO=[1988.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[88]
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106 START           TZERO=[1989.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[89]
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110 START           TZERO=[1991.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[91]
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112 START           TZERO=[1992.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[92]
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114 START           TZERO=[1993.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[93]
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120 START           TZERO=[1996.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[96]
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126 START           TZERO=[1999.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[99]
127 *%-----|-----|-----|-----|-----|-----|-----|-----|
128 START           TZERO=[2000.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[100]
129 *%-----|-----|-----|-----|-----|-----|-----|-----|
130 *% MISSING FROM AES RAINFALL DATA
131 *%START           TZERO=[2001.0101],  METOUT=[2],   NSTORM=[0],   NRUN=[101]
132 *%-----|-----|-----|-----|-----|-----|-----|-----|

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133  START                TZERO=[2002.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[102]
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135  START                TZERO=[2003.0101],  METOUT=[2],  NSTORM=[0],  NRUN=[103]
136  *%-----|-----|
137  FINISH
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00361) *****
00362) RW07-CO006-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00363) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.01 2.074 1970.0926,21:00 210.12 376 .000
00364) [CN=100.0; M= 3.00; Tp=.24]
00365) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00366) [InterEventTime= 12.00]
00367) RW07-CO007-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00368) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.86 1.824 1970.0926,21:05 210.12 376 .000
00369) [CN=100.0; M= 3.00; Tp=.38]
00370) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00371) [InterEventTime= 12.00]
00372) *****
00373) # STORMS
00374) *****
00375) ** END OF RUN : 71
00376) *****
00377) *****
00378) *****
00379) *****
00380) *****
00381) *****
00382) *****
00383) RUN:COMMANDS
00384) RW07-CO001-----
00385) START
00386) [TZRO = .00 hrs on 1970101]
00387) [NETOUT= 2 (1=Imperial, 2=metric output)]
00388) [NETFORM= 0]
00389) [NRUN = 071]
00390) *****
00391) # SWMHYM Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00392) *****
00393) # Project Name : [Calvan Perth properties]
00394) # Project Number: [2118]
00395) # Date : [2023 JAN 26]
00396) # Modeller : [JB]
00397) # Company : J.F. Sabourin and Associates
00398) # License # : 2549237
00399) *****
00400) # Model developed to simulate pre-development water budget
00401) *****
00402) # Ottawa International Airport (1967 - 2003)
00403) *****
00404) RW07-CO002-----
00405) * READ AES DATA
00406) [Filename = 610600.123]
00407) [Start_date= 1973.0101; End_date= 1973.1231]
00408) [DTr= 60.min; Length= 8760.hrs; WetHrs= 549; DryHrs= 8211; PTOF= 744.90]
00409) Maximum average rainfall intensities over
00410) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00411) 24.60 16.40 11.67 6.13 3.09 1.56 1.06 79 .54 mm/hr
00412) 24.60 33.20 35.00 36.80 37.10 37.40 38.00 38.00 38.90 mm
00413) 19710810 19710810 19710810 19710810 19710810 19710810 19710812 19710813 date
00414) Number of rainfall events per following interval time
00415) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00416) 156 123 113 93 72 61 52 42 33
00417) Number of events with at least the following durations
00418) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00419) 159 123 113 93 72 61 52 42 33
00420) RW07-CO003-----
00421) COMPUTE API
00422) [APIIn= 50.00; APIkdy= 9000; APIkdt= .9956]
00423) [APImax= 62.22; APIave= 14.84; APImin= .36]
00424) *****
00425) # Pre Development Condition - Using NASHYD and CN
00426) *****
00427) RW07-CO004-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00428) CONTINUOUS NASHYD 5.0 01:GransPre 22.01 1.094 1971.0810,15:05 92.41 177 .000
00429) [CN= 92.0; M= 3.00; Tp=.24]
00430) [IAREC=24.0; SMIN= 10.51; SMAK= 70.09; SK=.300]
00431) [InterEventTime= 12.00]
00432) RW07-CO005-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00433) CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 .890 1971.0810,15:10 85.91 165 .000
00434) [CN= 90.0; M= 3.00; Tp=.38]
00435) [IAREC=24.0; SMIN= 12.64; SMAK= 84.28; SK=.300]
00436) [InterEventTime= 12.00]
00437) *****
00438) # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
00439) *****
00440) RW07-CO006-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00441) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.01 1.456 1971.0810,15:00 164.77 316 .000
00442) [CN=100.0; M= 3.00; Tp=.24]
00443) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00444) [InterEventTime= 12.00]
00445) RW07-CO007-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00446) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.86 1.314 1971.0810,15:05 164.77 316 .000
00447) [CN=100.0; M= 3.00; Tp=.38]
00448) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00449) [InterEventTime= 12.00]
00450) *****
00451) # STORMS
00452) *****
00453) ** END OF RUN : 71
00454) *****
00455) *****
00456) *****
00457) *****
00458) *****
00459) *****
00460) *****
00461) RUN:COMMANDS
00462) RW07-CO001-----
00463) START
00464) [TZRO = .00 hrs on 1970101]
00465) [NETOUT= 2 (1=Imperial, 2=metric output)]
00466) [NETFORM= 0]
00467) [NRUN = 071]
00468) *****
00469) # SWMHYM Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00470) *****
00471) # Project Name : [Calvan Perth properties]
00472) # Project Number: [2118]
00473) # Date : [2023 JAN 26]
00474) # Modeller : [JB]
00475) # Company : J.F. Sabourin and Associates
00476) # License # : 2549237
00477) *****
00478) # Model developed to simulate pre-development water budget
00479) *****
00480) # Ottawa International Airport (1967 - 2003)
00481) *****
00482) RW07-CO002-----
00483) * READ AES DATA
00484) [Filename = 610600.123]
00485) [Start_date= 1973.0101; End_date= 1973.1230]
00486) [DTr= 60.min; Length= 8760.hrs; WetHrs= 489; DryHrs= 8271; PTOF= 784.30]
00487) Maximum average rainfall intensities over
00488) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00489) 37.30 19.15 12.97 8.15 4.50 2.53 2.00 1.71 1.17 mm/hr
00490) 31.30 38.30 38.90 48.90 54.00 60.70 72.10 82.20 84.20
00491) 19720712 19720712 19720807 19720808 19720808 19720808 19720713 19720714 19720715 date
00492) Number of rainfall events per following interval time
00493) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00494) 170 133 122 86 76 60 45 41 31
00495) Number of events with at least the following durations
00496) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00497) 169 96 58 22 5 0 0 0 0
00498) RW07-CO003-----
00499) COMPUTE API
00500) [APIIn= 50.00; APIkdy= 9000; APIkdt= .9956]
00501) [APImax= 108.88; APIave= 21.70; APImin= .00]
00502) *****
00503) # Pre Development Condition - Using NASHYD and CN
00504) *****
00505) RW07-CO004-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00506) CONTINUOUS NASHYD 5.0 01:GransPre 22.01 1.807 1972.0712, 4:00 242.59 309 .000
00507) [CN= 92.0; M= 3.00; Tp=.24]
00508) [IAREC=24.0; SMIN= 10.51; SMAK= 70.09; SK=.300]
00509) [InterEventTime= 12.00]
00510) RW07-CO005-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00511) CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 1.431 1972.0712, 4:05 229.36 292 .000
00512) [CN= 90.0; M= 3.00; Tp=.38]
00513) [IAREC=24.0; SMIN= 12.64; SMAK= 84.28; SK=.300]
00514) [InterEventTime= 12.00]
00515) *****
00516) # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
00517) *****
00518) RW07-CO006-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00519) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.01 2.197 1972.0712, 4:00 360.64 460 .000
00520) [CN=100.0; M= 3.00; Tp=.24]
00521) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00522) [InterEventTime= 12.00]
00523) RW07-CO007-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00524) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.86 1.942 1972.0712, 4:05 360.64 460 .000
00525) [CN=100.0; M= 3.00; Tp=.38]
00526) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00527) [InterEventTime= 12.00]
00528) *****
00529) # STORMS
00530) *****
00531) ** END OF RUN : 72
00532) *****
00533) *****
00534) *****
00535) *****
00536) *****
00537) *****
00538) *****
00539) RUN:COMMANDS
00540) RW07-CO001-----
00541) START
00542) [TZRO = .00 hrs on 1970101]
00543) [NETOUT= 2 (1=Imperial, 2=metric output)]
00544) [NETFORM= 0]
00545) [NRUN = 071]
00546) *****
00547) # SWMHYM Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00548) *****
00549) # Project Name : [Calvan Perth properties]
00550) # Project Number: [2118]
00551) # Date : [2023 JAN 26]
00552) # Modeller : [JB]
00553) # Company : J.F. Sabourin and Associates
00554) # License # : 2549237
00555) *****
00556) # Model developed to simulate pre-development water budget
00557) *****
00558) # Ottawa International Airport (1967 - 2003)
00559) *****
00560) RW07-CO002-----
00561) * READ AES DATA
00562) [Filename = 610600.123]
00563) [Start_date= 1973.0101; End_date= 1973.1231]
00564) [DTr= 60.min; Length= 8760.hrs; WetHrs= 549; DryHrs= 8211; PTOF= 744.90]
00565) Maximum average rainfall intensities over
00566) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00567) 20.00 17.25 12.33 7.10 3.63 1.89 1.28 .96 56 mm/hr
00568) 30.00 34.50 37.00 42.60 43.60 45.40 46.00 48.00 69.20 mm
00569) 19730611 19730608 19730608 19730608 19730608 19730616 19730616 19730616 date
00570) Number of rainfall events per following interval time
00571) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00572) 200 144 143 108 79 62 61 54 43 37
00573) Number of events with at least the following durations
00574) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00575) 200 102 66 20 4 0 0 0 0
00576) RW07-CO003-----
00577) COMPUTE API
00578) [APIIn= 50.00; APIkdy= 9000; APIkdt= .9956]
00579) [APImax= 78.26; APIave= 20.56; APImin= .00]
00580) *****
00581) # Pre Development Condition - Using NASHYD and CN
00582) *****
00583) RW07-CO004-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00584) CONTINUOUS NASHYD 5.0 01:GransPre 22.01 1.431 1973.0808,20:00 206.87 278 .000
00585) [CN= 92.0; M= 3.00; Tp=.24]
00586) [IAREC=24.0; SMIN= 10.51; SMAK= 70.09; SK=.300]
00587) [InterEventTime= 12.00]
00588) RW07-CO005-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00589) CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 1.309 1973.0808,20:05 194.10 261 .000
00590) [CN= 90.0; M= 3.00; Tp=.38]
00591) [IAREC=24.0; SMIN= 12.64; SMAK= 84.28; SK=.300]
00592) [InterEventTime= 12.00]
00593) *****
00594) # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
00595) *****
00596) RW07-CO006-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00597) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.01 1.738 1973.0611,17:00 320.63 430 .000
00598) [CN=100.0; M= 3.00; Tp=.24]
00599) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00600) [InterEventTime= 12.00]
00601) RW07-CO007-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00602) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.86 1.547 1973.0808,20:00 320.63 430 .000
00603) [CN=100.0; M= 3.00; Tp=.38]
00604) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00605) [InterEventTime= 12.00]
00606) *****
00607) # STORMS
00608) *****
00609) ** END OF RUN : 73
00610) *****
00611) *****
00612) *****
00613) *****
00614) *****
00615) *****
00616) *****
00617) RUN:COMMANDS
00618) RW07-CO001-----
00619) START
00620) [TZRO = .00 hrs on 1970101]
00621) [NETOUT= 2 (1=Imperial, 2=metric output)]
00622) [NETFORM= 0]
00623) [NRUN = 074]
00624) *****
00625) # SWMHYM Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00626) *****
00627) # Project Name : [Calvan Perth properties]
00628) # Project Number: [2118]
00629) # Date : [2023 JAN 26]
00630) # Modeller : [JB]
00631) # Company : J.F. Sabourin and Associates
00632) # License # : 2549237
00633) *****
00634) # Model developed to simulate pre-development water budget
00635) *****
00636) # Ottawa International Airport (1967 - 2003)
00637) *****
00638) RW07-CO002-----
00639) * READ AES DATA
00640) [Filename = 610600.123]
00641) [Start_date= 1974.0101; End_date= 1974.1231]
00642) [DTr= 60.min; Length= 8760.hrs; WetHrs= 320; DryHrs= 8440; PTOF= 386.20]
00643) Maximum average rainfall intensities over
00644) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00645) 20.40 15.40 10.37 5.18 2.98 1.63 1.08 .81 54 mm/hr
00646) 20.40 30.80 31.10 31.10 35.70 39.00 39.00 39.00 39.00 mm
00647) 19740719 19740719 19740719 19740719 19740719 19740719 19740719 19740719 19740719 date
00648) Number of rainfall events per following interval time
00649) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00650) 129 100 83 60 42 30 28 19 23
00651) Number of events with at least the following durations
00652) 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00653) 128 66 32 10 3 0 0 0 0
00654) RW07-CO003-----
00655) COMPUTE API
00656) [APIIn= 50.00; APIkdy= 9000; APIkdt= .9956]
00657) [APImax= 51.91; APIave= 11.36; APImin= .00]
00658) *****
00659) # Pre Development Condition - Using NASHYD and CN
00660) *****
00661) RW07-CO004-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00662) CONTINUOUS NASHYD 5.0 01:GransPre 22.01 .790 1974.0719, 0:10 60.86 158 .000
00663) [CN= 92.0; M= 3.00; Tp=.24]
00664) [IAREC=24.0; SMIN= 10.51; SMAK= 70.09; SK=.300]
00665) [InterEventTime= 12.00]
00666) RW07-CO005-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00667) CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 .611 1974.0719, 0:15 56.58 147 .000
00668) [CN= 90.0; M= 3.00; Tp=.38]
00669) [IAREC=24.0; SMIN= 12.64; SMAK= 84.28; SK=.300]
00670) [InterEventTime= 12.00]
00671) *****
00672) # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
00673) *****
00674) RW07-CO006-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00675) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.01 1.128 1974.0719, 0:10 309.40 283 .000
00676) [CN=100.0; M= 3.00; Tp=.24]
00677) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00678) [InterEventTime= 12.00]
00679) RW07-CO007-----Dtn-ID:HVND-----AREHA-QPEACms-TpeakDate_hh:mm-----RvM-R.C-----DWFms
00680) CONTINUOUS NASHYD 5.0 01:InfGransPre 22.86 .970 1974.0719, 0:15 309.40 283 .000
00681) [CN=100.0; M= 3.00; Tp=.38]
00682) [IAREC=24.0; SMIN=.00; SMAK=.00; SK=.300]
00683) [InterEventTime= 12.00]
00684) *****
00685) # STORMS
00686) *****
00687) ** END OF RUN : 74
00688) *****
00689) *****
00690) *****
00691) *****
00692) *****
00693) *****
00694) *****
00695) RUN:COMMANDS
00696) RW07-CO001-----
00697) START
00698) [TZRO = .00 hrs on 1970101]
00699) [NETOUT= 2 (1=Imperial, 2=metric output)]
00700) [NETFORM= 0]
00701) [NRUN = 075]
00702) *****
00703) # SWMHYM Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00704) *****
00705) # Project Name : [Calvan Perth properties]
00706) # Project Number: [2118]
00707) # Date : [2023 JAN 26]
00708) # Modeller : [JB]
00709) # Company : J.F. Sabourin and Associates
00710) # License # : 2549237
00711) *****
00712) # Model developed to simulate pre-development water budget
00713) *****
00714) # Ottawa International Airport (1967 - 2003)
00715) *****
00716) RW07-CO002-----
00717) * READ AES DATA
00718) [Filename = 610600.123]
00719) [Start_date= 1975.0101; End_date= 1975.1231]
00720) [DTr= 60.min; Length= 8760.hrs; WetHrs= 344; DryHrs= 8416; PTOF= 535.50]

00721: Maximum average rainfall intensities over
00722: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00723: 34.80 18.40 12.53 6.32 3.33 1.73 1.15 .87 .62 mm/hr
00724: 34.80 36.80 37.60 37.90 40.00 41.50 41.50 41.80 44.00
00725: 1976028 1976028 1976028 1976028 1976028 1976028 1976028 1976028 1976028 date
00726: Number of rainfall events per following interval time
00727: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00728: 136 118 99 78 61 49 40 33 25
00729: Number of events with at least the following durations
00730: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00731: 135 70 40 17 1 0 0 0 0
00732: R0075-C0003
00733: COMPUTE API
00734: [APIIn: 50.00; APIkdy: 9000; APIkdt: .9956]
00735: [APIave: 73.23; APIave: .03]
00736: *****
00737: # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
00738: *****
00739: R0075-C0004
00740: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 1.764 1975.0708.17:05 118.31 221 .000
00741: [CN: 92.0; N: 3.00; Tp: .24]
00742: [IAREC:24.00; SMIN: 10.51; SMAK: 70.09; SK: 300]
00743: [InterEventTime: 12.00]
00744: R0075-C0005
00745: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 1.133 1975.0708.17:10 110.48 207 .000
00746: [CN: 90.0; N: 3.00; Tp: .38]
00747: [IAREC:24.00; SMIN: 12.64; SMAK: 84.28; SK: 300]
00748: [InterEventTime: 12.00]
00749: *****
00750: # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
00751: *****
00752: R0075-C0006
00753: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 2.042 1975.0708.17:00 196.46 367 .000
00754: [CN:100.0; N: 3.00; Tp: .24]
00755: [IAREC:24.00; SMIN: .00; SMAK: .00; SK: 300]
00756: [InterEventTime: 12.00]
00757: R0075-C0007
00758: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 1.792 1975.0708.17:05 196.46 367 .000
00759: [CN:100.0; N: 3.00; Tp: .38]
00760: [IAREC:24.00; SMIN: .00; SMAK: .00; SK: 300]
00761: [InterEventTime: 12.00]
00762: *****
00763: # STORMS
00764: *****
00765: ** END OF RUN : 77
00766: *****
00767: *****
00768: *****
00769: *****
00770: *****
00771: *****
00772: *****
00773: RUN:COMMANDS
00774: R0076-C0001
00775: START
00776: [TZERO = .00 hrs on 19760101]
00777: [METOUT = 2 (Imperial, 2-metric output)]
00778: [NFORM = 0]
00779: [NIN = 007]
00780: *****
00781: # SMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00782: *****
00783: # Project Name : [Caivan Perth properties]
00784: # Project Number: [2118]
00785: # Date : [2023 JAN 26]
00786: # Modeller : [JF]
00787: # Company : [J.F. Sabourin and Associates]
00788: # License # : [2549237]
00789: *****
00790: # Model developed to simulate pre-development water budget
00791: *****
00792: # Ottawa International Airport (1967 - 2003)
00793: *****
00794: R0076-C0002
00795: * READ AES DATA
00796: [Filename = 610600.123]
00797: [Dfr: 60.min; Length= 8046.hrs; WetHrs= 3891; DryHrs= 7675; PTOF= 492.40]
00798: *****
00799: Maximum average rainfall intensities over
00800: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00801: 14.00 8.90 6.43 4.65 2.35 1.39 .97 .97 .80 mm/hr
00802: 14.00 27.80 19.30 27.90 28.20 33.30 35.10 46.60 57.50
00803: 1976028 1976028 1976028 1976028 1976028 1976028 1976028 1976028 1976028 date
00804: Number of rainfall events per following interval time
00805: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00806: 173 139 123 96 76 59 44 38 28
00807: Number of events with at least the following durations
00808: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00809: 172 80 46 13 1 0 0 0 0
00810: R0076-C0003
00811: COMPUTE API
00812: [APIIn: 50.00; APIkdy: 9000; APIkdt: .9956]
00813: [APIave: 59.67; APIave: 15.32; APIave: .02]
00814: *****
00815: # Pre Development Condition - Using NASHHYD and CN
00816: *****
00817: R0075-C0004
00818: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 .270 1976.0828.19:10 93.86 191 .000
00819: [CN: 92.0; N: 3.00; Tp: .24]
00820: [IAREC:24.00; SMIN: 10.51; SMAK: 70.09; SK: 300]
00821: [InterEventTime: 12.00]
00822: R0076-C0005
00823: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 .246 1976.0828.22:05 87.52 178 .000
00824: [CN: 90.0; N: 3.00; Tp: .38]
00825: [IAREC:24.00; SMIN: 12.64; SMAK: 84.28; SK: 300]
00826: [InterEventTime: 12.00]
00827: *****
00828: # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
00829: *****
00830: R0076-C0006
00831: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 .652 1976.0828.19:05 159.47 324 .000
00832: [CN:100.0; N: 3.00; SMAK: .00; SK: 300]
00833: [InterEventTime: 12.00]
00834: R0076-C0007
00835: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 .506 1976.0828.19:15 159.47 324 .000
00836: [CN:100.0; N: 3.00; SMAK: .00; SK: 300]
00837: [InterEventTime: 12.00]
00838: *****
00839: # STORMS
00840: *****
00841: ** END OF RUN : 76
00842: *****
00843: *****
00844: *****
00845: *****
00846: *****
00847: *****
00848: *****
00849: *****
00850: *****
00851: RUN:COMMANDS
00852: R0077-C0001
00853: START
00854: [TZERO = .00 hrs on 19770101]
00855: [METOUT = 2 (Imperial, 2-metric output)]
00856: [NFORM = 0]
00857: [NIN = 007]
00858: *****
00859: # SMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00860: *****
00861: # Project Name : [Caivan Perth properties]
00862: # Project Number: [2118]
00863: # Date : [2023 JAN 26]
00864: # Modeller : [JF]
00865: # Company : [J.F. Sabourin and Associates]
00866: # License # : [2549237]
00867: *****
00868: # Model developed to simulate pre-development water budget
00869: *****
00870: # Ottawa International Airport (1967 - 2003)
00871: *****
00872: R0077-C0002
00873: * READ AES DATA
00874: [Filename = 610600.123]
00875: [Start_date = 1977.0101; End_date = 1977.1231]
00876: [Dfr: 60.min; Length= 8014.hrs; WetHrs= 511; DryHrs= 7505; PTOF= 677.60]
00877: *****
00878: Maximum average rainfall intensities over
00879: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00880: 21.30 15.20 10.40 6.53 3.30 1.65 1.38 1.06 7.75 mm/hr
00881: 21.30 30.40 31.20 39.20 39.60 39.60 48.60 51.00 52.40
00882: 1977017 1977017 1977017 1977017 1977017 1977017 1977017 1977017 1977017 date
00883: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00884: 188 156 139 107 82 61 52 41 28
00885: Number of events with at least the following durations
00886: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00887: 187 89 49 28 17 10 7 4 2
00888: R0077-C0003
00889: COMPUTE API
00890: [APIIn: 50.00; APIkdy: 9000; APIkdt: .9956]
00891: [APIave: 74.28; APIave: 20.42; APIave: 1.62]
00892: *****
00893: # Pre Development Condition - Using NASHHYD and CN
00894: *****
00895: R0077-C0004
00896: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 1.041 1977.0901.23:00 158.78 234 .000
00897: [CN: 92.0; N: 3.00; Tp: .24]
00898: [IAREC:24.00; SMIN: 10.51; SMAK: 70.09; SK: 300]
00899: [InterEventTime: 12.00]
00900: R0077-C0005
00901: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 1.884 1976.0616.17:05 238.27 373 .000
00902: [CN:100.0; N: 3.00; SMAK: .00; SK: 300]
00903: [InterEventTime: 12.00]
00904: *****
00905: # STORMS
00906: *****
00907: *****
00908: *****
00909: *****
00910: *****
00911: *****
00912: *****
00913: *****
00914: *****
00915: *****
00916: *****
00917: *****
00918: *****
00919: *****
00920: *****
00921: *****
00922: *****
00923: *****
00924: *****
00925: *****
00926: *****
00927: *****
00928: *****
00929: RUN:COMMANDS
00930: R0078-C0001
00931: START
00932: [TZERO = .00 hrs on 19780101]
00933: [METOUT = 2 (Imperial, 2-metric output)]
00934: [NFORM = 0]
00935: [NIN = 007]
00936: *****
00937: # SMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
00938: *****
00939: # Project Name : [Caivan Perth properties]
00940: # Project Number: [2118]
00941: # Date : [2023 JAN 26]
00942: # Modeller : [JF]
00943: # Company : [J.F. Sabourin and Associates]
00944: # License # : [2549237]
00945: *****
00946: # Model developed to simulate pre-development water budget
00947: *****
00948: # Ottawa International Airport (1967 - 2003)
00949: *****
00950: R0078-C0002
00951: * READ AES DATA
00952: [Filename = 610600.123]
00953: [Start_date = 1978.0101; End_date = 1978.1231]
00954: [Dfr: 60.min; Length= 8040.hrs; WetHrs= 407; DryHrs= 7633; PTOF= 638.80]
00955: *****
00956: Maximum average rainfall intensities over
00957: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00958: 36.00 20.15 12.10 6.05 3.04 1.62 1.12 .87 58 mm/hr
00959: 36.00 36.30 36.30 36.30 36.30 36.30 36.30 36.30 36.30
00960: 1978018 1978018 1978018 1978018 1978018 1978018 1978018 1978018 1978018 date
00961: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00962: 167 160 145 110 85 60 53 45 31
00963: Number of events with at least the following durations
00964: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
00965: 166 78 48 17 1 0 0 0 0
00966: R0078-C0003
00967: COMPUTE API
00968: [APIIn: 50.00; APIkdy: 9000; APIkdt: .9956]
00969: [APIave: 65.36; APIave: 19.25; APIave: .25]
00970: *****
00971: # Pre Development Condition - Using NASHHYD and CN
00972: *****
00973: R0078-C0004
00974: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 1.719 1978.0616.17:00 127.68 200 .000
00975: [CN: 92.0; N: 3.00; Tp: .24]
00976: [IAREC:24.00; SMIN: 10.51; SMAK: 70.09; SK: 300]
00977: [InterEventTime: 12.00]
00978: R0078-C0005
00979: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 1.864 1978.0616.17:05 117.69 184 .000
00980: [CN: 90.0; N: 3.00; Tp: .38]
00981: [IAREC:24.00; SMIN: 12.64; SMAK: 84.28; SK: 300]
00982: [InterEventTime: 12.00]
00983: *****
00984: # Pre Development Condition - Using NASHHYD and CN - No INFILTRATION
00985: *****
00986: R0078-C0006
00987: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 2.117 1978.0616.17:00 238.27 373 .000
00988: [CN:100.0; N: 3.00; Tp: .24]
00989: [IAREC:24.00; SMIN: .00; SMAK: .00; SK: 300]
00990: [InterEventTime: 12.00]
00991: R0078-C0007
00992: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 1.864 1978.0616.17:05 238.27 373 .000
00993: [CN:100.0; N: 3.00; Tp: .38]
00994: [IAREC:24.00; SMIN: .00; SMAK: .00; SK: 300]
00995: [InterEventTime: 12.00]
00996: *****
00997: # STORMS
00998: *****
00999: *****
01000: *****
01001: *****
01002: *****
01003: *****
01004: *****
01005: *****
01006: *****
01007: *****
01008: RUN:COMMANDS
01009: R0079-C0001
01010: START
01011: [TZERO = .00 hrs on 19790101]
01012: [METOUT = 2 (Imperial, 2-metric output)]
01013: [NFORM = 0]
01014: [NIN = 007]
01015: *****
01016: # SMHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
01017: *****
01018: # Project Name : [Caivan Perth properties]
01019: # Project Number: [2118]
01020: # Date : [2023 JAN 26]
01021: # Modeller : [JF]
01022: # Company : [J.F. Sabourin and Associates]
01023: # License # : [2549237]
01024: *****
01025: # Model developed to simulate pre-development water budget
01026: *****
01027: # Ottawa International Airport (1967 - 2003)
01028: R0079-C0002
01029: * READ AES DATA
01030: [Filename = 610600.123]
01031: [Start_date = 1979.0101; End_date = 1979.1231]
01032: [Dfr: 60.min; Length= 8760.hrs; WetHrs= 546; DryHrs= 8214; PTOF= 865.50]
01033: *****
01034: Maximum average rainfall intensities over
01035: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01036: 34.90 22.00 14.67 7.33 5.14 2.63 1.76 1.31 48 mm/hr
01037: 34.90 44.00 44.00 44.00 44.00 44.00 44.00 44.00 44.00
01038: 1979016 1979016 1979016 1979016 1979016 1979016 1979016 1979016 1979016 date
01039: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01040: 205 160 146 114 92 61 52 43 35
01041: Number of events with at least the following durations
01042: 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01043: 204 98 68 23 4 0 0 0 0
01044: R0079-C0003
01045: COMPUTE API
01046: [APIIn: 50.00; APIkdy: 9000; APIkdt: .9956]
01047: [APIave: 78.45; APIave: 23.23; APIave: .13]
01048: *****
01049: # Pre Development Condition - Using NASHHYD and CN
01050: *****
01051: R0079-C0004
01052: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 1.888 1979.0616.14:00 275.16 318 .000
01053: [CN: 92.0; N: 3.00; Tp: .24]
01054: [IAREC:24.00; SMIN: 10.51; SMAK: 70.09; SK: 300]
01055: [InterEventTime: 12.00]
01056: R0079-C0005
01057: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 1.645 1979.0616.14:00 259.82 300 .000
01058: [CN: 90.0; N: 3.00; Tp: .38]
01059: [IAREC:24.00; SMIN: 12.64; SMAK: 84.28; SK: 300]
01060: [InterEventTime: 12.00]
01061: *****
01062: # STORMS
01063: *****
01064: R0079-C0006
01065: CONTINUOUS NASHHYD 5.0 0.0:GrantPre 22.01 2.118 1979.0616.14:00 411.97 475 .000
01066: [CN:100.0; N: 3.00; Tp: .24]
01067: [IAREC:24.00; SMIN: .00; SMAK: .00; SK: 300]
01068: [InterEventTime: 12.00]
01069: R0079-C0007
01070: CONTINUOUS NASHHYD 5.0 0.0:InfTrape 22.86 2.026 1979.0616.14:00 411.97 475 .000
01071: [CN:100.0; N: 3.00; Tp: .38]
01072: [IAREC:24.00; SMIN: .00; SMAK: .00; SK: 300]
01073: [InterEventTime: 12.00]
01074: *****
01075: # STORMS
01076: *****
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01078: *****
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01081: *****
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01261 # Ottawa International Airport (1967 - 2003)
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01441 R0884C0004-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01442 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.01 .724 1984.0812, 7:00 120.12 .261 .000
01443 [Cm 90.0: N# 3.00: Tp# .24]
01444 [IARC#24.00: SMIN# 10.51: SMAX# 70.09: SK# .300]
01445 [InterEventTime# 12.00]
01446 R0884C0005-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01447 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.86 .990 1984.0812,16:05 112.33 .245 .000
01448 [Cm 90.0: N# 3.00: Tp# .38]
01449 [IARC#24.00: SMIN# 12.64: SMAX# 84.28: SK# .300]
01450 [InterEventTime# 12.00]
01451 *****
01452 # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
01453 *****
01454 R0884C0006-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01455 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.01 1.059 1984.0812, 7:00 196.65 .428 .000
01456 [Cm 100.0: N# 3.00: Tp# .24]
01457 [IARC#24.00: SMIN# .00: SMAX# .00: SK# .300]
01458 [InterEventTime# 12.00]
01459 R0884C0007-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01460 CONTINUOUS NASHYD 5.0 01:InfTrayPre 22.86 .954 1984.0812, 7:05 196.65 .428 .000
01461 [Cm 100.0: N# 3.00: Tp# .38]
01462 [IARC#24.00: SMIN# .00: SMAX# .00: SK# .300]
01463 [InterEventTime# 12.00]
01464 *****
01465 # STORMS
01466 *****
01467 ** END OF RUN : 84
01468 *****
01469 *****
01470 *****
01471 *****
01472 *****
01473 *****
01474 *****
01475 RUN#COMMAND#
01476 R0885C0001-----
01477 START
01478 [TZERO = .00 hrs on 19850101]
01479 [NETOUT# 2 (1=Imperial, 2=metric output)]
01480 [NETFORM# 0]
01481 [NRUN = 0089]
01482 *****
01483 # SWHMYO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
01484 *****
01485 # Project Name : [Caivan Perth properties]
01486 # Project Number : [2118]
01487 # Date : [2023 JAN 26]
01488 # Modeller : [JFB]
01489 # Company : [J.F. Sabourin and Associates]
01490 # License # : [2549237]
01491 *****
01492 # Model developed to simulate pre-development water budget
01493 *****
01494 # Ottawa International Airport (1967 - 2003)
01495 *****
01496 R0885C0002-----
01497 READ A&S DATA
01498 [Filename = 6106000.123]
01499 [Start_date= 1985.0101; End_date= 1985.1231]
01500 [Dtr= 60.min; Length# 8760.hrs; WetRcs= 884; DryRcs= 8406; PTO# 559.90]
01501 *****
01502 Maximum average rainfall intensities over
01503 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01504 19.00 13.60 9.07 5.27 2.63 1.35 1.10 .82 .60 mm/hr
01505 19850724 19850617 19850617 19850618 19850619 19850619 19850619 19850619 19850625 date
01506 Number of rainfall events per following interevent time
01507 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01508 .124 .99 .96 .85 .77 .57 .49 .41 .31
01509 Number of events with at least the following durations
01510 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01511 .123 .70 .40 .14 .5 0 0 0 0
01512 *****
01513 R0885C0003-----
01514 COMPUTE API
01515 [APIIn# 50.00: APIkdy# .9000: APIkdt# .9956]
01516 [APIOut# 52.83: APIAv# 21.40: APIDim# 1.18]
01517 *****
01518 # Pre Development Condition - Using NASHYD and CN
01519 *****
01520 R0885C0004-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01521 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.01 .719 1985.0617,23:00 121.13 .216 .000
01522 [Cm 90.0: N# 3.00: Tp# .24]
01523 [IARC#24.00: SMIN# 10.51: SMAX# 70.09: SK# .300]
01524 [InterEventTime# 12.00]
01525 R0885C0005-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01526 CONTINUOUS NASHYD 5.0 01:InfTrayPre 22.86 .498 1985.0617,23:05 111.44 .199 .000
01527 [Cm 90.0: N# 3.00: Tp# .38]
01528 [IARC#24.00: SMIN# 12.64: SMAX# 84.28: SK# .300]
01529 [InterEventTime# 12.00]
01530 *****
01531 # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
01532 *****
01533 R0885C0006-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01534 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.01 1.026 1985.0716,14:00 227.04 .406 .000
01535 [Cm 100.0: N# 3.00: Tp# .24]
01536 [IARC#24.00: SMIN# .00: SMAX# .00: SK# .300]
01537 [InterEventTime# 12.00]
01538 R0885C0007-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01539 CONTINUOUS NASHYD 5.0 01:InfTrayPre 22.86 .899 1985.0617,23:00 227.04 .406 .000
01540 [Cm 100.0: N# 3.00: Tp# .38]
01541 [IARC#24.00: SMIN# .00: SMAX# .00: SK# .300]
01542 [InterEventTime# 12.00]
01543 *****
01544 # STORMS
01545 *****
01546 ** END OF RUN : 87
01547 *****
01548 *****
01549 *****
01550 *****
01551 *****
01552 *****
01553 *****
01554 R0886C0001-----
01555 START
01556 [TZERO = .00 hrs on 19850101]
01557 [NETOUT# 2 (1=Imperial, 2=metric output)]
01558 [NETFORM# 0]
01559 [NRUN = 0089]
01560 *****
01561 # SWHMYO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
01562 *****
01563 # Project Name : [Caivan Perth properties]
01564 # Project Number : [2118]
01565 # Date : [2023 JAN 26]
01566 # Modeller : [JFB]
01567 # Company : [J.F. Sabourin and Associates]
01568 # License # : [2549237]
01569 *****
01570 # Model developed to simulate pre-development water budget
01571 *****
01572 # Ottawa International Airport (1967 - 2003)
01573 *****
01574 R0886C0002-----
01575 READ A&S DATA
01576 [Filename = 6106000.123]
01577 [Start_date= 1986.0101; End_date= 1986.1231]
01578 [Dtr= 60.min; Length# 8040.hrs; WetRcs= 820; DryRcs= 7520; PTO# 849.40]
01579 *****
01580 Maximum average rainfall intensities over
01581 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01582 18.30 17.80 13.57 7.07 4.93 2.93 2.32 1.82 1.31 mm/hr
01583 18.30 18.30 36.40 40.70 42.40 59.20 70.40 83.50 87.20 94.40 mm
01584 19860729 19860729 19860729 19860729 19860729 19860729 19860729 19860729 19860729 date
01585 Number of rainfall events per following interevent time
01586 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01587 .201 .161 .149 .118 .88 .61 .51 .47 .30
01588 Number of events with at least the following durations
01589 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01590 .200 .107 .73 .22 1 0 0 0 0
01591 *****
01592 R0886C0003-----
01593 COMPUTE API
01594 [APIIn# 50.00: APIkdy# .9000: APIkdt# .9956]
01595 [APIOut# 52.83: APIAv# 21.40: APIDim# 1.18]
01596 *****
01597 # Pre Development Condition - Using NASHYD and CN
01598 *****
01599 R0886C0004-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01600 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.01 .994 1986.0729,15:00 252.94 .298 .000
01601 [Cm 90.0: N# 3.00: Tp# .24]
01602 [IARC#24.00: SMIN# 10.51: SMAX# 70.09: SK# .300]
01603 [InterEventTime# 12.00]
01604 R0886C0005-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01605 CONTINUOUS NASHYD 5.0 01:InfTrayPre 22.86 .932 1986.0729,15:05 240.11 .283 .000
01606 [Cm 90.0: N# 3.00: Tp# .38]
01607 [IARC#24.00: SMIN# 12.64: SMAX# 84.28: SK# .300]
01608 [InterEventTime# 12.00]
01609 *****
01610 # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
01611 *****
01612 R0886C0006-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01613 CONTINUOUS NASHYD 5.0 01:InfGrantPre 22.01 1.118 1986.0729,15:00 370.77 .437 .000
01614 [Cm 100.0: N# 3.00: Tp# .24]
01615 [IARC#24.00: SMIN# .00: SMAX# .00: SK# .300]
01616 [InterEventTime# 12.00]
01617 *****
01618 R0886C0007-----DRAIN-ID:INHVD-----AREHA-QPEARcms-TpeakDate_hh:mm-----Rvum-R.C.--DWfms
01619 CONTINUOUS NASHYD 5.0 01:InfTrayPre 22.86 1.146 1986.0729,15:00 370.77 .437 .000
01620 [Cm 100.0: N# 3.00: Tp# .38]
01621 [IARC#24.00: SMIN# .00: SMAX# .00: SK# .300]
01622 [InterEventTime# 12.00]
01623 *****
01624 *****
01625 *****
01626 *****
01627 *****
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01800 *****

01801 # Company : J.F. Sabourin and Associates
01802 # License # : 2549237
01803 # *****
01804 # Model developed to simulate pre-development water budget
01805 # Ottawa International Airport (1967 - 2003)
01806 # *****
01807 # *****
01808 # *****
01809 # READ AES DATA
01810 [Filename = 6106000.123
01811 [Start_date= 1989.0101; End_date= 1989.1231]
01812 [Dw= 60;min; Length= 8040;hrs; WetHrs= 421; DryHrs= 7619; PTO= 522.50]
01813 Maximum average rainfall intensities over
01814 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01815 22.70 12.60 8.60 5.76 3.03 1.67 1.14 .86 .59 mm/hr
01816 22.70 25.20 26.80 34.50 36.30 40.20 40.50 41.50 42.50 mm
01817 1989072 1989072 1989072 1989072 1989072 1989072 1989072 1989072 1989072 date
01818 Number of rainfall events per following interevent time
01819 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01820 170 137 101 71 52 40 36 29
01821 Number of events with at least the following durations
01822 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
01823 169 81 49 17 4 0 0 0 0
01824 # *****
01825 # *****
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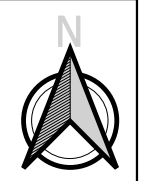
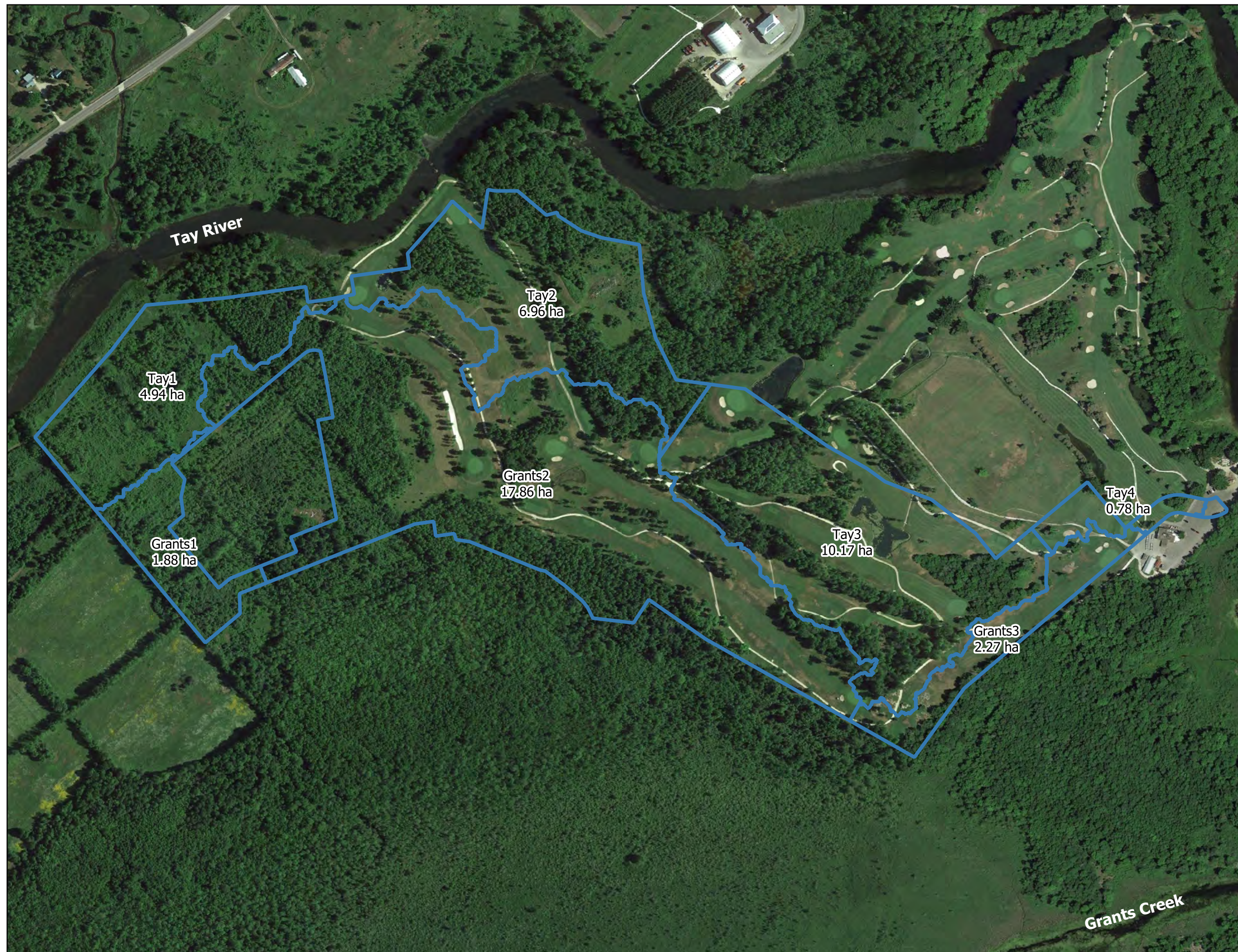
05211 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
05212 126 62 42 37 32 21
05223 Number of events with at least the following durations
05224 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
05225 126 64 43 31 0 0 0
05226 R0598:CO0003-----
05227 COMPUTE API
05228 [APInum=50.00:APIKey=9000:APIdate=1995]
05229 [APInum=57.22:APIKey=21.28:APIdate=1995]
05230 *****
05231 # Pre Development Condition - Using NASHYD and CN
05232 *****
05233 R0598:CO0004-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05234 CONTINUOUS NASHYD 5.0 01:GrantPre 22.01 .626 1998.0927.14:00 85.62 1294 .000
05235 [CM=92.0: N=3.00: Tp= .24]
05236 [IAR=24.00: SMIN= 10.51: SMAX= 70.09: SK= .300]
05237 [InterEventTime= 12.00]
05238 R0598:CO0005-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05239 CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 .844 1998.0927.14:05 79.35 1180 .000
05240 [CM= 92.0: N= 3.00: Tp= .24]
05241 [IAR=24.00: SMIN= 12.64: SMAX= 84.28: SK= .300]
05242 *****
05243 *****
05244 # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
05245 *****
05246 R0598:CO0006-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05247 CONTINUOUS NASHYD 5.0 01:GrantPre 22.01 .917 1998.0627.1:00 152.32 1346 .000
05248 [CM=100.0: N= 3.00: Tp= .24]
05249 [IAR=24.00: SMIN= .00: SMAX= .00: SK= .300]
05250 [InterEventTime= 12.00]
05251 R0598:CO0007-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05252 CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 .794 1998.0627.1:05 152.32 1346 .000
05253 [CM=100.0: N= 3.00: Tp= .38]
05254 [IAR=24.00: SMIN= .00: SK= .300]
05255 [InterEventTime= 12.00]
05256 *****
05257 # STORMS
05258 *****
05259 ** END OF RUN : 98
05260 *****
05261 *****
05262 *****
05263 *****
05264 *****
05265 *****
05266 *****
05267 R0598:CO0008-----
05268 R0598:CO0001-----
05269 START
05270 [TZERO = .00 hrs on 20000101]
05271 [MRTYPE= 2 (1=Imperial, 2=metric output)]
05272 [MRTIME= 0]
05273 [MUNIT= 0999]
05274 *****
05275 # SWHYMO Ver:5.02/Jan 2001 <BETA> / INPUT DATA FILE
05276 *****
05277 # Project Name : [Caivan Perth properties]
05278 # Project Number : [2118]
05279 # Date : [2023 JAN 26]
05280 # Modeller : [JB]
05281 # Company : J.F. Sabourin and Associates
05282 # License # : 2549237
05283 *****
05284 # Model developed to simulate pre-development water budget
05285 # Ottawa International Airport (1967 - 2003)
05286 R0598:CO0002-----
05287 *****
05288 *****
05289 * READ AFS DATA
05290 [Filename = 6106000.123 ]
05291 [Start_date= 1999.0101: End_date= 1999.1231 ]
05292 [DT= 60.min: Length= 4440.hrs: WetHrs= 247: DryHrs= 4193: PTO= 424.40 ]
05293 *****
05294 Maximum average rainfall intensities over
05295 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
05296 17.50 10.10 9.03 6.97 3.31 1.65 1.45 1.22 .87 mm/hr
05297 17.50 20.20 27.10 39.40 39.70 39.70 52.20 56.60 69.50 mm
05298 19990717 19990717 19990906 19990906 19990908 19990908 19990908 19990908 19990908 date
05299 Number of rainfall events per following interval time
05300 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
05301 102 80 70 63 56 49 31 28 18
05302 *****
05303 Number of events with at least the following durations
05304 1 hr 2 hrs 3 hrs 6 hrs 12 hrs 24 hrs 36 hrs 48 hrs 72 hrs
05305 102 80 70 63 56 49 31 28 18
05306 *****
05307 *****
05308 R0598:CO0003-----
05309 COMPUTE API
05310 [APInum=50.00:APIKey=9000:APIdate=1995]
05311 [APInum=69.51:APIKey=23.97:APIdate=1995]
05312 *****
05313 # Pre Development Condition - Using NASHYD and CN
05314 *****
05315 R0598:CO0004-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05316 CONTINUOUS NASHYD 5.0 01:GrantPre 22.01 .592 1999.0717.15:05 85.38 1201 .000
05317 [CM=92.0: N=3.00: Tp= .24]
05318 [IAR=24.00: SMIN= 10.51: SMAX= 70.09: SK= .300]
05319 [InterEventTime= 12.00]
05320 R0598:CO0005-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05321 CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 .917 1999.0906.8:05 78.74 1186 .000
05322 [CM= 90.0: N= 3.00: Tp= .38]
05323 [IAR=24.00: SMIN= 12.64: SMAX= 84.28: SK= .300]
05324 [InterEventTime= 12.00]
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05327 # Pre Development Condition - Using NASHYD and CN - No INFILTRATION
05328 *****
05329 R0598:CO0006-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05330 CONTINUOUS NASHYD 5.0 01:GrantPre 22.01 .993 1999.0717.15:00 157.02 370 .000
05331 [CM=100.0: N= 3.00: Tp= .24]
05332 [IAR=24.00: SMIN= .00: SK= .300]
05333 [InterEventTime= 12.00]
05334 R0598:CO0007-----DTmin-ID:INHYD-----AREHA-QPEARCS-TPeakDate_hh:mm-----RvM-R.C-----DWFCms
05335 CONTINUOUS NASHYD 5.0 01:TrayPre 22.86 .833 1999.0717.15:05 157.02 370 .000
05336 [CM=100.0: N= 3.00: Tp= .38]
05337 [IAR=24.00: SMIN= .00: SK= .300]
05338 [InterEventTime= 12.00]
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02881 *** WARNING: Missing rainfall increments were set to 0.
02882 *** WARNING: Requested start date is less than start date in file.
02883 *** WARNING: Missing rainfall increments were set to 0.
02884 *** WARNING: Missing rainfall increments were set to 0.
02885 *** WARNING: Missing rainfall increments were set to 0.
02886 *** WARNING: Missing rainfall increments were set to 0.
02887 *** WARNING: Missing rainfall increments were set to 0.
02888 *** WARNING: Missing rainfall increments were set to 0.
02889 *** WARNING: Missing rainfall increments were set to 0.
02890 *** WARNING: Requested start date is less than start date in file.
02891 *** WARNING: Missing rainfall increments were set to 0.
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02901 *** WARNING: Requested start date is less than start date in file.
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02903 *** WARNING: Missing rainfall increments were set to 0.
02904 *** WARNING: Missing rainfall increments were set to 0.
02905 *** WARNING: Requested start date is less than start date in file.
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02912 *** WARNING: Requested start date is less than start date in file.
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02915 *** WARNING: Requested start date is less than start date in file.
02916 *** WARNING: Missing rainfall increments were set to 0.
02917 *** WARNING: Requested start date is less than start date in file.
02918 *** WARNING: Missing rainfall increments were set to 0.
02919 *** WARNING: Requested start date is less than start date in file.
02920 *** WARNING: Missing rainfall increments were set to 0.
02921 *** WARNING: Requested start date is less than start date in file.
02922 *** WARNING: Missing rainfall increments were set to 0.
02923 *** WARNING: Requested start date is less than start date in file.
02924 *** WARNING: Specified end date is beyond the end date in file.
02925 *** WARNING: Missing rainfall increments were set to 0.
02926 Simulation ended on 2023-02-14 at 17:44:01
02927 *****
02928

Appendix E

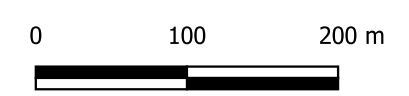
Preliminary SWM Pond Sizing



Legend

- Drainage Area
[Name]
[Area]

SCALE: 1:5000



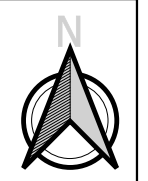
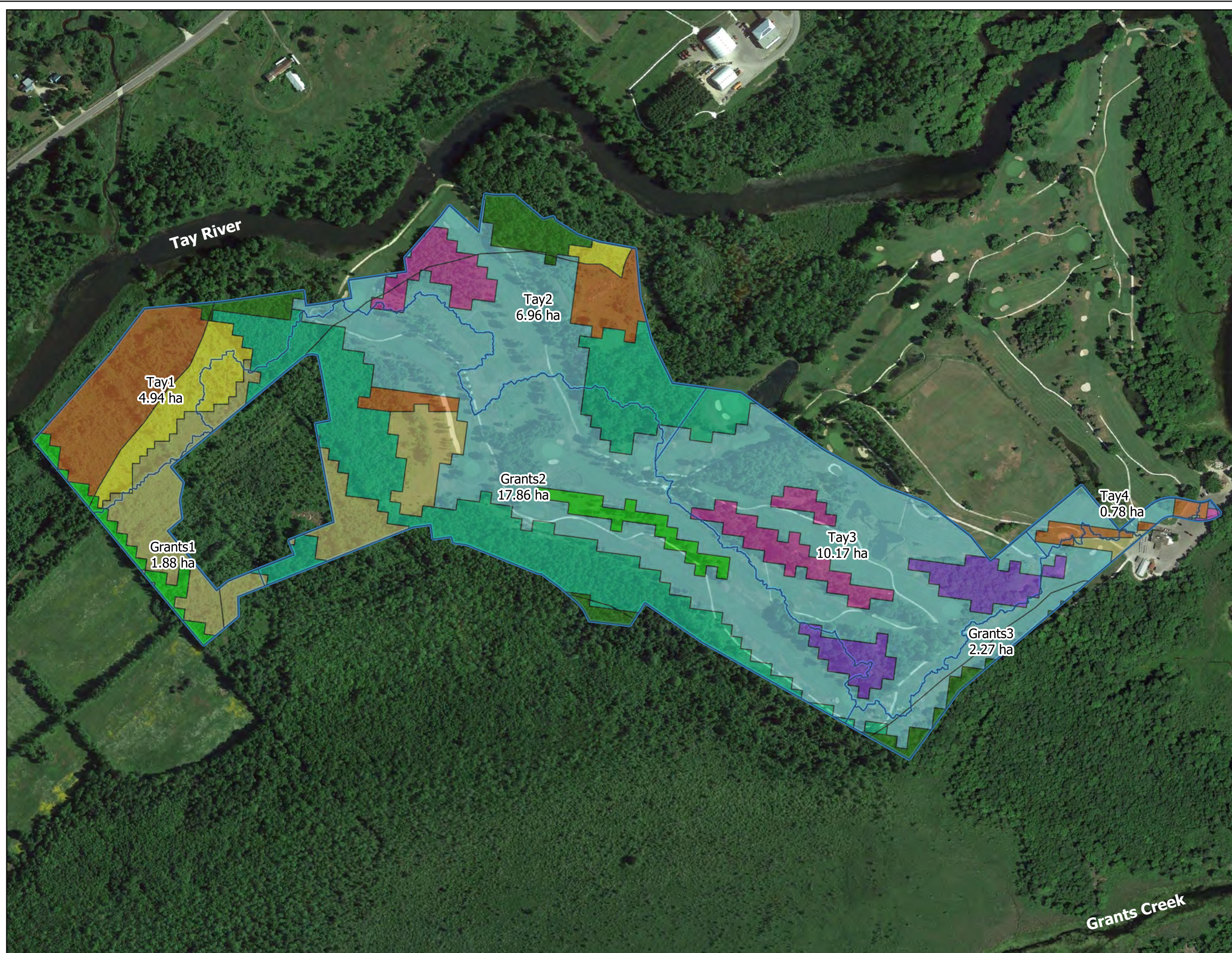
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Figure E1: Pre-Development Drainage Area

PROJECT	2118-21
DRAWN	BT
DATE	February 2023



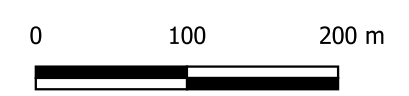
Legend

Drainage Area
[Name]
[Area]

Land Use

- Built Up Area - Pervious
- Deciduous Forest
- Fallow
- Forest
- Hedge Rows
- Mixed Forest
- Plantation
- Tilled
- Transportation
- Treed Swamp

SCALE: 1:5000



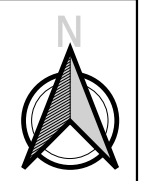
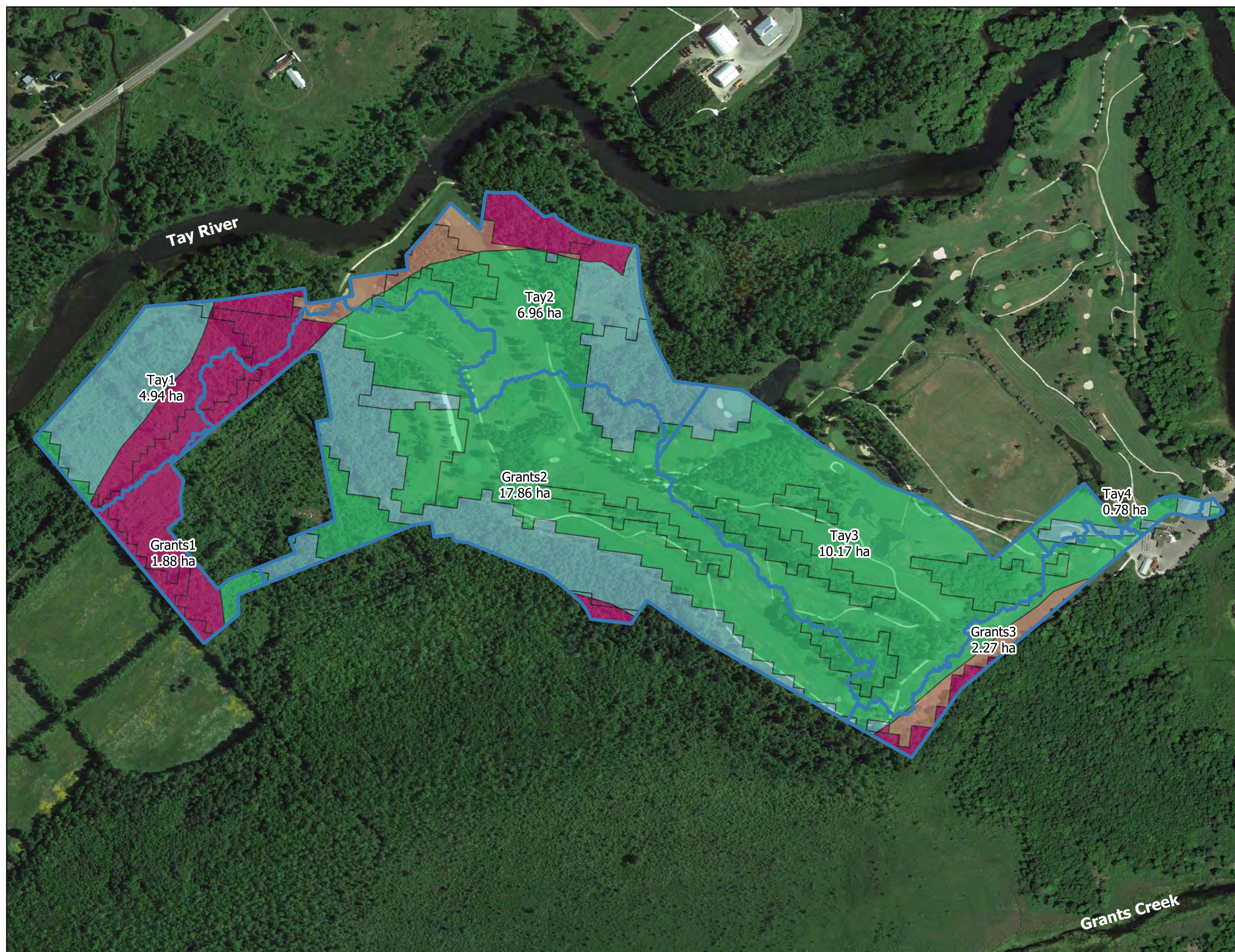
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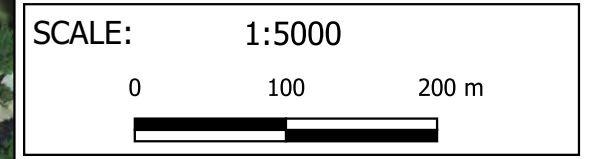
Figure E2: Pre-Development Land Use

PROJECT	2118-21
DRAWN	BT
DATE	February 2023



Legend

- Drainage Area
[Name]
[Area]
- Soil Types
- Monteagle (SCS Type B)
- Monteagle Sandy Loam (SCS Type B)
- Muck (SCS Type D)
- North Gower (SCS Type D)
- Water (N)



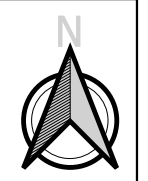
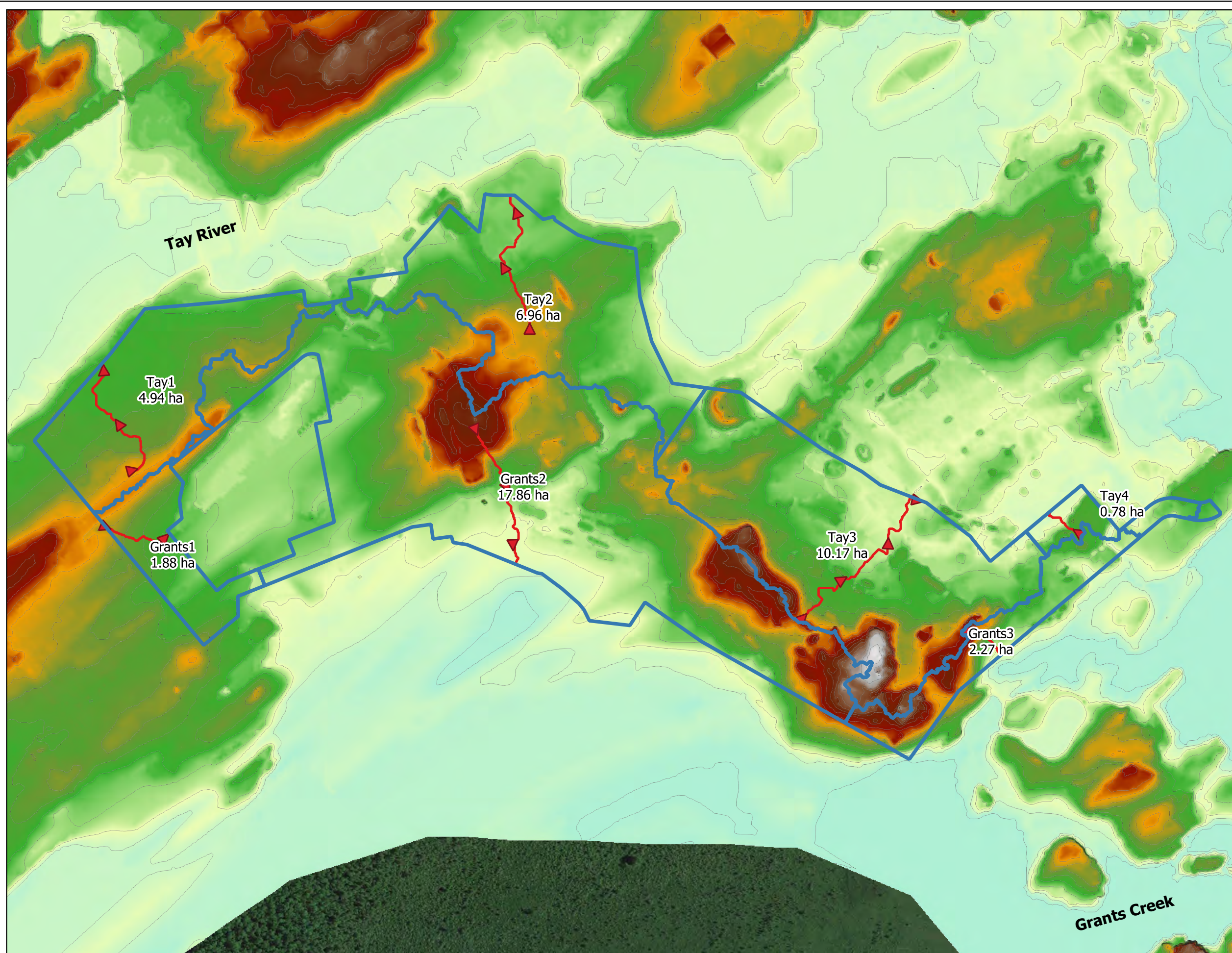
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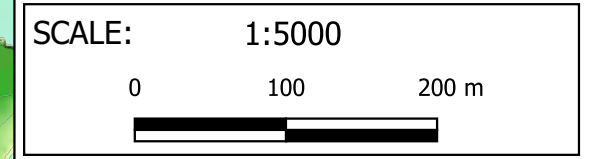
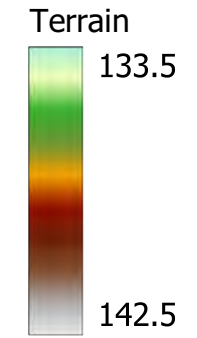
Figure E3: Pre-Development Soils

PROJECT	2118-21
DRAWN	BT
DATE	February 2023



Legend

- Drainage Area
[Name]
[Ares]
- ▶ Flow Lengths



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Figure E4: Pre-Development Flow Paths

PROJECT	2118-21
DRAWN	BT
DATE	February 2023

Table E1: Calculation of SCS Curve Number (CN)

Tay River

Area (ha)	Land Type	Soil Name	Soil		CN	% of Catchment	Weighted CN
			Condition	Soil Group			
0.608	Built Up Area - Pervious	NORTH GOWER	D	Fair	84	2.7%	2.2
0.407	Plantation	MUCK	D	Fair	79	1.8%	1.4
0.138	Hedge Rows	MONTEAGLE	B	Fair	56	0.6%	0.3
1.266	Fallow	MUCK	D	Fair	94	5.5%	5.2
0.023	Hedge Rows	MUCK	D	Fair	77	0.1%	0.1
0.468	Deciduous Forest	MUCK	D	Fair	79	2.0%	1.6
0.056	Treed Swamp	MONTEAGLE SANDY LOAM	B	Fair	50	0.2%	0.1
3.558	Tilled	MONTEAGLE SANDY LOAM	B	Fair	78	15.6%	12.1
0.943	Treed Swamp	MUCK	D	Fair	50	4.1%	2.1
1.626	Forest	MONTEAGLE	B	Fair	60	7.1%	4.3
0.344	Forest	NORTH GOWER	D	Fair	79	1.5%	1.2
1.640	Deciduous Forest	MONTEAGLE SANDY LOAM	B	Fair	60	7.2%	4.3
10.611	Built Up Area - Pervious	MONTEAGLE	B	Fair	69	46.4%	32.0
1.143	Mixed Forest	MONTEAGLE	B	Fair	60	5.0%	3.0
0.025	Plantation	MONTEAGLE	B	Fair	60	0.1%	0.1
						CN	70

Grants Creek

Area (ha)	Land Type	Soil Name	Soil		CN	% of Catchment	Weighted CN
			Condition	Soil Group			
1.550	Plantation	MUCK	D	Fair	79	7.0%	5.6
2.093	Plantation	MONTEAGLE	B	Fair	60	9.5%	5.7
0.322	Fallow	MUCK	D	Fair	94	1.5%	1.4
0.354	Hedge Rows	MUCK	D	Fair	77	1.6%	1.2
5.266	Deciduous Forest	MONTEAGLE SANDY LOAM	B	Fair	60	23.9%	14.4
0.041	Forest	MONTEAGLE	B	Fair	60	0.2%	0.1
0.781	Built Up Area - Pervious	NORTH GOWER	D	Fair	84	3.5%	3.0
0.341	Mixed Forest	MONTEAGLE	B	Fair	60	1.5%	0.9
0.631	Hedge Rows	MONTEAGLE	B	Fair	56	2.9%	1.6
0.375	Deciduous Forest	MUCK	D	Fair	79	1.7%	1.3
0.032	Treed Swamp	MONTEAGLE SANDY LOAM	B	Fair	50	0.1%	0.1
0.384	Tilled	MONTEAGLE SANDY LOAM	B	Fair	78	1.7%	1.4
9.425	Built Up Area - Pervious	MONTEAGLE	B	Fair	69	42.8%	29.5
0.403	Treed Swamp	MUCK	D	Fair	50	1.8%	0.9
0.002	Forest	WATER	N	Fair	98	0.0%	0.0
0.008	Transportation	MONTEAGLE SANDY LOAM	B	Fair	98	0.0%	0.0
						CN	67

Table E2: Time to Peak Calculations

Parameter	Units	Grants	Tay
Area	ha	22.01	22.86
CN	-	67	70
Ptotal to calc C from CN, use 2 yr 3 hr Chicago stom	P(mm)	31.9	31.9
	la(mm)	5.00	5.00
	RV(mm)	4.75	5.3
Ptotal to calc C from CN, use 2 yr 24 hr SCS stom	P(mm)	48.46	48.46
	RV(mm)	11.2	12.4
C (From Chicago storm)	-	0.149	0.167
C (From SCS storm)	-	0.231	0.256
Length of Channel	m	231	304
	ft	758	996
Elevation of Head Water	m	139.35	139.46
	ft	457	458
Elevation of Outlet	m	134.30	135.42
	ft	441	444
Average Slope	m/m	2.18%	1.33%
	ft/ft	2.18%	1.33%
Kirpich			
Time of Concentration	mins	6	8
Time to Peak	min	4	6
Time to Peak	Hours	0.06	0.09
FAA (From Chicago storm)			
Time of Concentration	mins	36	48
Time to Peak	mins	24	32
Time to Peak	Hours	0.40	0.54
FAA (From SCS storm)			
Time of Concentration	mins	33	44
Time to Peak	mins	22	29
Time to Peak	Hours	0.37	0.48
Barnsby Williams			
Time of Concentration	mins	8	12
Time to Peak	mins	6	8
Time to Peak	Hours	0.09	0.13
SCS			
Time of Concentration	mins	25	37
Time to Peak	mins	17	24
Time to Peak	Hours	0.28	0.41
Selected Method			
FAA (From SCS storm)			
Time to Peak	min	22	29
Time to Peak	Hours	0.37	0.48

Note:

All methods calculated as per Appendix A of the SWMHYMO manual

Time to Peak calculated as 2/3 Time of concentration

```

1  20    Metric units / ID numbers OFF
2  *#*****
   *****
3  *#  SWMHYMO  /  INPUT DATA FILE
4  *#*****
   *****
5  *#  Project Name  : [Caivan Perth properties]
6  *#  Project Number: [2118]
7  *#  Date          : [2023 JAN 26]
8  *#  Modeller     : [JB]
9  *#  Company      : J.F. Sabourin and Associates
10 *#  License #    : 2549237
11 *#*****
   *****
12 *# Model developed to simulate runoff from subcatchments under pre development conditions
13 *#*****
   *****
14 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
15 [ "25MMC3H.stm" ] <--storm filename, one per line for NSTORM time
16 *%-----|-----|
17 READ STORM     STORM_FILENAME=[ "STORM.001" ]
18 *%-----|-----|
19 *#*****
   *****
20 *# Grants Creek
21 *#*****
   *****
22 CALIB NASHYD   NHYD=[ "Grants" ], DT=[1]min, AREA=[22.01](ha),
23 DWF=[0](cms), CN/C=[67], IA=[5.0](mm),
24 N=[3], TP=[0.37]hrs,
25 RAINFALL[ , , -1]
26 *#*****
   *****
27 *# TAY RIVER
28 *#*****
   *****
29 CALIB NASHYD   NHYD=[ "Tay" ], DT=[1]min, AREA=[22.86](ha),
30 DWF=[0](cms), CN/C=[70], IA=[5.0](mm),
31 N=[3], TP=[0.48]hrs,
32 RAINFALL[ , , -1]
33
34 *%-----|-----|
35 ADD HYD       NHYDsum=[ "Total" ], NHYDs to add=[ "Grants"+"Tay" ]
36 *#####
37 *# STORMS
38 *#####
39 *% 25 mm Storm based on 2-Year, 3-Hour Chicago Storm
40 *%START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
41 *%              [ "25MMC3H.stm" ] <--storm filename, one per line for NSTORM time
42 *%-----|-----|
43 *% 2-Year, 3-Hour Chicago Storm
44 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[002]
45 [ "002YRCHI3HR.stm" ] <--storm filename, one per line for NSTORM time
46 *%-----|-----|
47 *% 5-Year, 3-Hour Chicago Storm
48 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[005]
49 [ "005YRCHI3HR.stm" ] <--storm filename, one per line for NSTORM time
50 *%-----|-----|
51 *% 10-Year, 3-Hour Chicago Storm
52 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[010]
53 [ "010YRCHI3HR.stm" ] <--storm filename, one per line for NSTORM time
54 *%-----|-----|
55 *% 25-Year, 3-Hour Chicago Storm
56 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[025]
57 [ "025YRCHI3HR.stm" ] <--storm filename, one per line for NSTORM time
58 *%-----|-----|
59 *% 50-Year, 3-Hour Chicago Storm
60 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[050]
61 [ "050YRCHI3HR.stm" ] <--storm filename, one per line for NSTORM time

```

```

62  *%-----|-----
63  *% 100-Year, 3-Hour Chicago Storm
64  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
65             ["100YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
66  *%-----|-----
67  *% 2-Year, 24-Hour SCS Storm
68  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
69             ["002YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
70  *%-----|-----
71  *% 5-Year, 24-Hour SCS Storm
72  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[105]
73             ["005YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
74  *%-----|-----
75  *% 10-Year, 24-Hour SCS Storm
76  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[110]
77             ["010YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
78  *%-----|-----
79  *% 25-Year, 24-Hour SCS Storm
80  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[125]
81             ["025YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
82  *%-----|-----
83  *% 50-Year, 24-Hour SCS Storm
84  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[150]
85             ["050YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
86  *%-----|-----
87  *% 100-Year, 24-Hour SCS Storm
88  START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[199]
89             ["100YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
90  *%-----|-----
91  *% 100-Year, 24-Hour SCS Storm + 20%
92  *START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[998]
93  *%          ["SC24100x+.stm"] <--storm filename, one per line for NSTORM time
94  *%-----|-----
95  *% 100-Year, 3-Hour Chicago Storm + 20%
96  *START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
97  *           ["100YRCHI3HR+.stm"] <--storm filename, one per line for NSTORM time
98  *%-----|-----
99  FINISH
100

```

```

00001 .....
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00003 SSSS W M M M H H Y Y M M OOO 222 000 11 5555 .....
00004 S W M M M M M H H Y Y M M M O O 2 0 0 11 5 .....
00005 SSSS W M M M M H H Y Y M M M O O 2 0 0 11 5 Ver 5.500 .....
00006 S W M M M M H H Y Y M M O O 222 0 0 11 555 FEB 2015 .....
00007 SSSS W M M M H H Y Y M M OOO 2 0 0 11 5 .....
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00009 StormWater Management Hydrologic Model 222 000 11 555 .....
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00721> R0199:000004-----DRAIN-ID:MSVD-----AREAAb-QPEAKcms-TpeakDate_jh:mm-----RVMm-R.C-----DWFcms
00722> CALIB MSVD 1.0 01:7day 22.86 1.800 No_date 12:23 57.25 487 .000
00723> [CM= 70.0; N= 3.00; Tp= .48]
00724> R0199:000005-----DRAIN-ID:MSVD-----AREAAb-QPEAKcms-TpeakDate_jh:mm-----RVMm-R.C-----DWFcms
00725> ADD STD 1.0 02:6days 22.01 1.932 No_date 12:16 53.14 n/a .000
00726> r 1.0 02:7day 22.86 1.800 No_date 12:23 57.25 n/a .000
00727> SUM= 1.0 01:7total 44.87 3.694 No_date 12:19 55.33 n/a .000
00728> #####
00729> # STOPS
00730> #####
00731> R0199:000002-----
00732> FINISH
00733>
00734>
00735> WARNINGS / ERRORS / NOTES
00736>
00737> Simulation ended on 2023-02-15 at 16:23:19
00738>
00739>
```

```

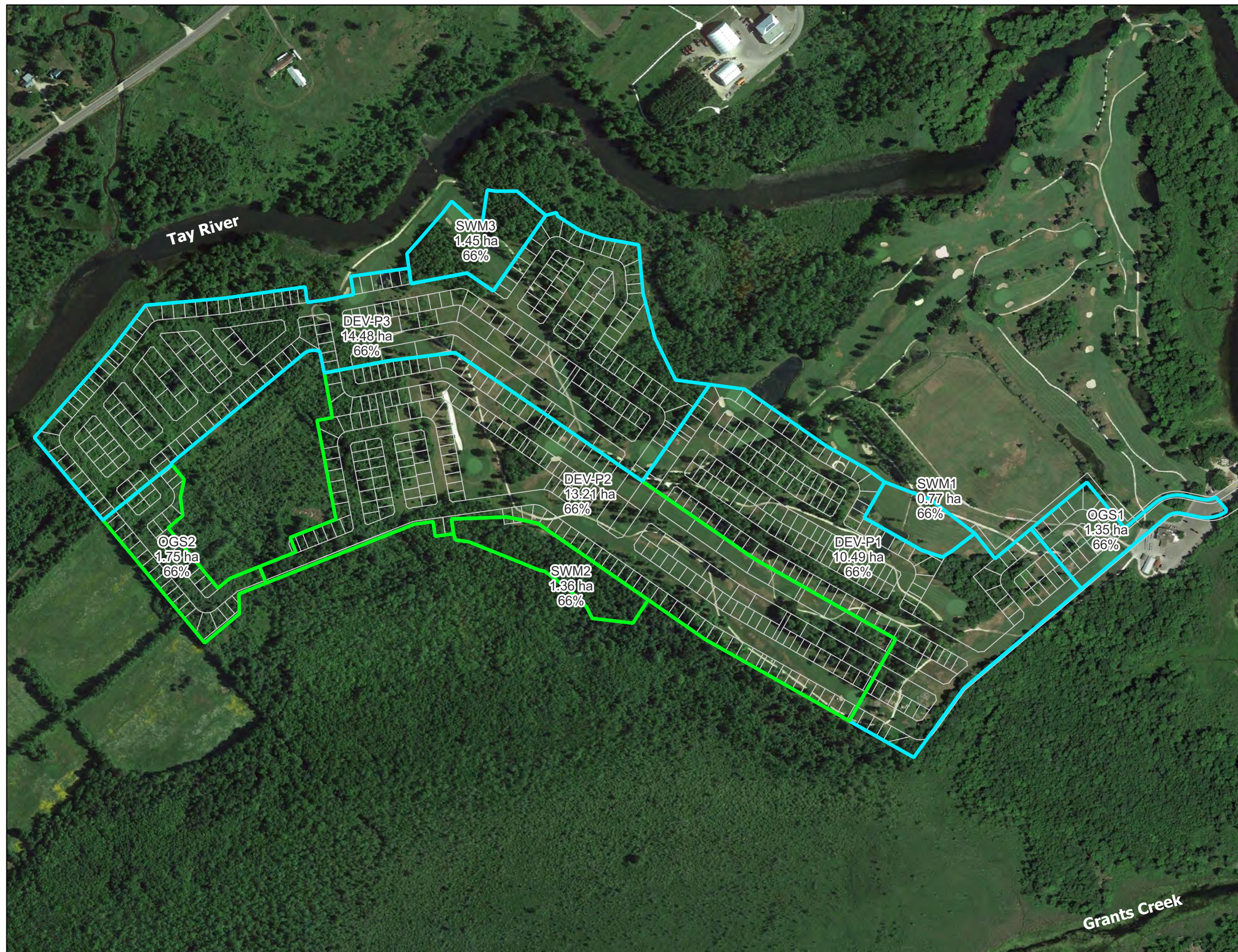
1  20    Metric units / ID numbers OFF
2  *#*****
   *****
3  *#  SWMHYMO  /  INPUT DATA FILE
4  *#*****
   *****
5  *#  Project Name  : [Caivan Perth properties]
6  *#  Project Number: [2118]
7  *#  Date          : [2023 JAN 26]
8  *#  Modeller     : [JB]
9  *#  Company      : J.F. Sabourin and Associates
10 *#  License #    : 2549237
11 *#*****
   *****
12 *# Model developed to simulate runoff from subcatchments under post development
   conditions and to size SWM ponds
13 *#*****
   *****
14 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
15 [ "25MMC3H.stm" ] <--storm filename, one per line for NSTORM time
16 *               [ "100YRSCS24HR.stm" ] <--storm filename, one per line for NSTORM time
17 *               [ "100YRCHI3HR.stm" ] <--storm filename, one per line for NSTORM time
18
19 *%-----|-----|
20 READ STORM      STORM_FILENAME=[ "STORM.001" ]
21 *%-----|-----|
   -----|
22 *#*****
   *****
23 *# TAY RIVER
24 *#*****
   *****
25 CALIB STANDHYD  NHYD=[ "Dev-P1" ], DT=[1](min), AREA=[10.493](ha), XIMP=[0.56],
   TIMP=[0.66], DWF=[0](cms),
26 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
   DCAY=[4.14](/hr), F=[0.00](mm),
27 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
   MNP=[0.25], SCP=[0](min),
28 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[264](m),
   MNI=[0.013], SCI=[0](min),
29 RAINFALL=[ , , -1](mm/hr)
30 *%-----|-----|
   -----|
31 CALIB STANDHYD  NHYD=[ "SWM1" ], DT=[1](min), AREA=[0.767](ha), XIMP=[0.56],
   TIMP=[0.66], DWF=[0](cms),
32 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
   DCAY=[4.14](/hr), F=[0.00](mm),
33 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
   MNP=[0.25], SCP=[0](min),*
34 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[71](m),
   MNI=[0.013], SCI=[0](min),
35 RAINFALL=[ , , -1](mm/hr)
36 *%-----|-----|
   -----|
37 CALIB STANDHYD  NHYD=[ "OGS1" ], DT=[1](min), AREA=[1.353](ha), XIMP=[0.56],
   TIMP=[0.66], DWF=[0](cms),
38 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
   DCAY=[4.14](/hr), F=[0.00](mm),
39 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
   MNP=[0.25], SCP=[0](min),
40 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[95](m),
   MNI=[0.013], SCI=[0](min),
41 RAINFALL=[ , , -1](mm/hr)
42 *%-----|-----|
   -----|
43 ADD HYD        NHYDsum=[ "Pond1-In" ], NHYDs to add=[ "Dev-P1"+"SWM1"+"OGS1" ]
44 *%-----|-----|
   -----|
45 ROUTE RESERVOIR NHYDout=[ "Pond1-Out" ], NHYDin=[ "Pond1-In" ], RDT=[1](min),

```

```

46             TABLE of ( OUTFLOW-STORAGE ) values
47             (cms) - (ha-m)
48             [ 0.0 , 0.0 ]
49             [ 0.001 , 0.0505 ]
50             [ 0.056 , 0.14763 ]
51             [ 0.066 , 0.17252 ]
52             [ 0.126 , 0.23392 ]
53             [ 0.174 , 0.27454 ]
54             [ 0.196 , 0.2805 ]
55             [ 0.241 , 0.3297 ]
56             [ 0.296 , 0.3725 ]
57             [ 0.331 , 0.3745 ]
58             [ 0.355 , 0.41417 ]
59             [ 0.436 , 0.43417 ]
60             [ 0.574 , 0.50267 ]
61             [ 0.683 , 0.55377 ]
62             [ 0.795 , 0.6051 ]
63             [ -1 , -1 ]
64             NHYDovf=[ "Pond1-Over" ],
65 *%-----|-----
66 CALIB STANDHYD NHYD=[ "Dev-P3" ], DT=[1](min), AREA=[14.479](ha), XIMP=[0.56],
TIMP=[0.66], DWF=[0](cms),
67 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
DCAy=[4.14](/hr), F=[0.00](mm),
68 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.25], SCP=[0](min),
69 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[311](m),
MNI=[0.013], SCI=[0](min),
70 RAINFALL=[ , , -1](mm/hr)
71 *%-----|-----
72 CALIB STANDHYD NHYD=[ "SWM3" ], DT=[1](min), AREA=[1.452](ha), XIMP=[0.56],
TIMP=[0.66], DWF=[0](cms),
73 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
DCAy=[4.14](/hr), F=[0.00](mm),
74 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
MNP=[0.25], SCP=[0](min),
75 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[98](m),
MNI=[0.013], SCI=[0](min),
76 RAINFALL=[ , , -1](mm/hr)
77 *%-----|-----
78 ADD HYD NHYDsum=[ "Pond3-In" ], NHYDs to add=[ "Dev-P3"+"SWM3" ]
79 *%-----|-----
80 ROUTE RESERVOIR NHYDout=[ "Pond3-Out" ], NHYDin=[ "Pond3-In" ], RDT=[1](min),
81 TABLE of ( OUTFLOW-STORAGE ) values
82 (cms) - (ha-m)
83 [ 0.0 , 0.0 ]
84 [ 0.002 , 0.0637 ]
85 [ 0.07 , 0.18661 ]
86 [ 0.084 , 0.2178 ]
87 [ 0.159 , 0.2954 ]
88 [ 0.219 , 0.3466 ]
89 [ 0.248 , 0.3528 ]
90 [ 0.305 , 0.4156 ]
91 [ 0.373 , 0.470 ]
92 [ 0.419 , 0.4712 ]
93 [ 0.449 , 0.5221 ]
94 [ 0.55 , 0.54615 ]
95 [ 0.724 , 0.6326 ]
96 [ 0.862 , 0.69617 ]
97 [ 1.005 , 0.761 ]
98 [ -1 , -1 ]
99 NHYDovf=[ "Pond1-Over" ],
100 *%-----|-----
101 *ADD HYD NHYDsum=[ "Tay" ], NHYDs to

```

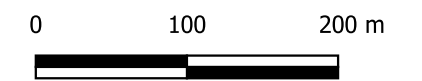



Legend

Drainage Area
[Name]
[Area]
[% Imp]

- Drains to Tay River
- Drains to Grants Creek

SCALE: 1:5000



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 www.jfsa.com

CAIVAN
COMMUNITIES

Perth Golf Course

Figure E5: Post-Development Drainage Area

PROJECT	2118-21
DRAWN	BT
DATE	February 2023


```

102 add=["Pond1-Out"+"Pond1-Over"+"Pond3-Out"+"Pond3-Over"]
103 *%-----|-----
104 *#*****
105 *# GRANTS CREEK
106 *#*****
106 CALIB STANDHYD      NHYD=["DEV-P2"], DT=[1](min), AREA=[13.211](ha), XIMP=[0.56],
107 TIMP=[0.66], DWF=[0](cms),
108 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
109 DCAY=[4.14](/hr), F=[0.00](mm),
110 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
111 MNP=[0.25], SCP=[0](min),
112 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[297](m),
113 MNI=[0.013], SCI=[0](min),
114 RAINFALL=[ , , -1](mm/hr)
115 *%-----|-----
116 CALIB STANDHYD      NHYD=["SWM2"], DT=[1](min), AREA=[1.355](ha), XIMP=[0.56],
117 TIMP=[0.66], DWF=[0](cms),
118 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
119 DCAY=[4.14](/hr), F=[0.00](mm),
120 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
121 MNP=[0.25], SCP=[0](min),
122 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[95](m),
123 MNI=[0.013], SCI=[0](min),
124 RAINFALL=[ , , -1](mm/hr)
125 *%-----|-----
126 CALIB STANDHYD      NHYD=["OGS2"], DT=[1](min), AREA=[1.754](ha), XIMP=[0.56],
127 TIMP=[0.66], DWF=[0](cms),
128 LOSS=[1] Horton Equ: Fo=[76.2](mm/hr), Fc=[13.2](mm/hr),
129 DCAY=[4.14](/hr), F=[0.00](mm),
130 Pervious areas: IAper=[4.67](mm), SLPP=[2.0](%), LGP=[40](m),
131 MNP=[0.25], SCP=[0](min),
132 Impervious areas: IAimp=[1.57](mm), SLPI=[0.5](%), LGI=[108](m),
133 MNI=[0.013], SCI=[0](min),
134 RAINFALL=[ , , -1](mm/hr)
135 *%-----|-----
136 ADD HYD              NHYDsum=["Pond2-In"], NHYDs to add=["DEV-P2"+"SWM2"+"OGS2"]
137 *%-----|-----
138 ROUTE RESERVOIR     NHYDout=["Pond2-Out"], NHYDin=["Pond2-In"], RDT=[1](min),
139 TABLE of ( OUTFLOW-STORAGE ) values
140 (cms) - (ha-m)
141 [ 0 , 0 ]
142 [ 0.002 , 0.0653 ]
143 [ 0.125 , 0.1723 ]
144 [ 0.149 , 0.1974 ]
145 [ 0.288 , 0.26845 ]
146 [ 0.399 , 0.3151 ]
147 [ 0.559 , 0.3773 ]
148 [ 0.688 , 0.4223 ]
149 [ 0.83 , 0.4678 ]
150 [ 1.382 , 0.54086 ]
151 [ 1.653 , 0.5874 ]
152 [ 1.932 , 0.6343 ]
153 [ -1 , -1 ]
154 NHYDovf=["Pond2-Over"],
155 *%-----|-----
156 *SAVE HYD           NHYD=["OGSGrant"], # OF PCYCLES=[-1], ICASEsh=[1]
157 *                   HYD_COMMENT=["Overflows to Grants Creek from uncontrolled"]
158 *%-----|-----
159 *ADD HYD            NHYDsum=["Grant"], NHYDs to add=["Pond2-Out"+"Pond2-Over"]
160 *%-----|-----

```

```

-----|
149 *ADD HYD          NHYDsum=["Total"], NHYDs to add=["Tay"+"Grant"]
150 *%-----|-----
-----|
151 *#####
152 *# STORMS
153 *#####
154 *% 25 mm Storm based on 2-Year, 3-Hour Chicago Storm
155 *%START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[001]
156 *%              ["25MMC3H.stm"] <--storm filename, one per line for NSTORM time
157 *%-----|-----
158 *% 2-Year, 3-Hour Chicago Storm
159 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[002]
160 ["002YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
161 *%-----|-----
162 *% 5-Year, 3-Hour Chicago Storm
163 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[005]
164 ["005YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
165 *%-----|-----
166 *% 10-Year, 3-Hour Chicago Storm
167 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[010]
168 ["010YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
169 *%-----|-----
170 *% 25-Year, 3-Hour Chicago Storm
171 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[025]
172 ["025YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
173 *%-----|-----
174 *% 50-Year, 3-Hour Chicago Storm
175 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[050]
176 ["050YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
177 *%-----|-----
178 *% 100-Year, 3-Hour Chicago Storm
179 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[099]
180 ["100YRCHI3HR.stm"] <--storm filename, one per line for NSTORM time
181 *%-----|-----
182 *% 2-Year, 24-Hour SCS Storm
183 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[102]
184 ["002YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
185 *%-----|-----
186 *% 5-Year, 24-Hour SCS Storm
187 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[105]
188 ["005YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
189 *%-----|-----
190 *% 10-Year, 24-Hour SCS Storm
191 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[110]
192 ["010YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
193 *%-----|-----
194 *% 25-Year, 24-Hour SCS Storm
195 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[125]
196 ["025YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
197 *%-----|-----
198 *% 50-Year, 24-Hour SCS Storm
199 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[150]
200 ["050YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
201 *%-----|-----
202 *% 100-Year, 24-Hour SCS Storm
203 START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[199]
204 ["100YRSCS24HR.stm"] <--storm filename, one per line for NSTORM time
205 *%-----|-----
206 *% 100-Year, 24-Hour SCS Storm + 20%
207 *%START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[998]
208 *%              ["SC24l00x+.stm"] <--storm filename, one per line for NSTORM time
209 *%-----|-----
210 *% 100-Year, 3-Hour Chicago Storm + 20%
211 *%START          TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[999]
212 *%              ["100YRCHI3HR+.stm"] <--storm filename, one per line for NSTORM time
213 *%-----|-----
214 FINISH
215

```

00001 *****
00002 *****
00003 SSSSS W W M M H H Y Y M M O O 222 000 11 5555 *****
00004 S W W M M M M H H Y Y M M O O 2 0 0 11 5 *****
00005 SSSSS W W M M M M M M M M O O 222 000 11 555 *****
00006 S W W M M M H H Y Y M M O O 222 0 0 11 555 FEB 2015 *****
00007 SSSSS W W M M H H Y Y M M O O 2 0 0 11 5 *****
00008 *****
00009 *****
00010 StormWater Management Hydrologic Model 222 000 11 555 *****
00011 *****
00012 *****
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```

00361 [MstOfsed=2954E+00 m3, TotOfVol=0.000E+00 m3, N-Ovf= 0, TotDurOfV= 0 hrs]
00362 *****
00363 # GRANTS CREEK
00364 # *****
00365 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00366 CALIS STANDHYD 1.0 01:DEV-P2 13.21 1.652 No.Date 1:02 24.40 648 .000
00367 [XIMP:56:TIMP:66]
00368 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00369 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00370 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00371 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00372 CALIS STANDHYD 1.0 01:DEV-P2 1.36 .213 No.Date 1:00 24.40 648 .000
00373 [XIMP:56:TIMP:66]
00374 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00375 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00376 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00377 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00378 CALIS STANDHYD 1.0 01:OSG2 1.75 .272 No.Date 1:00 24.40 648 .000
00379 [XIMP:56:TIMP:66]
00380 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00381 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00382 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00383 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00384 ADD HYD + 1.0 02:DEV-P2 13.21 1.652 No.Date 1:02 24.40 n/a .000
00385 + 1.0 02:SMW2 1.36 .267 No.Date 1:00 29.24 n/a .000
00386 + 1.0 02:OSG2 1.75 .272 No.Date 1:00 24.40 n/a .000
00387 SIM + 1.0 01:Pond2-In 16.32 2.094 No.Date 1:02 24.40 n/a .000
00388 R005C00016-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00389 ROUTE RESERVOIR > 1.0 02:Pond2-In 16.32 2.094 No.Date 1:02 24.40 n/a .000
00390 out <= 1.0 01:Pond2-out 16.32 .298 No.Date 1:41 24.40 n/a .000
00391 overlow <= 1.0 03:Pond1-Over .00 .000 No.Date 0:00 .00 n/a .000
00392 [MstOfsed=2948E+00 m3, TotOfVol=0.000E+00 m3, N-Ovf= 0, TotDurOfV= 0 hrs]
00393 *****
00394 # STORMS
00395 # *****
00396 ** END OF RUN : 9
00397 *****
00400 *****
00401 *****
00402 *****
00403 *****
00404 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00405 *****
00406 START [TZED= 0 hrs on 0]
00407 [MTCUT= 2 (1=imperial, 2=metric output)]
00408 [MTCUT= 2]
00409 [MTCUT= 2]
00410 [MTCUT= 2]
00411 # SWHYMO / INPUT DATA FILE
00412 # *****
00413 # Project Name : [Calvan Perth properties]
00414 # Project Number : [2118]
00415 # Date : [2023 JAN 26]
00416 # Modeller : [J]
00417 # License # : [2549237]
00418 # Company : [J.F. Sabourin and Associates]
00419 # Model developed to simulate runoff from subcatchments under post development conditions and to size SWM ponds
00420 *****
00421 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00422 CALIS STANDHYD 1.0 01:DEV-P2 10.49 1.708 No.Date 1:00 29.24 668 .000
00423 [XIMP:56:TIMP:66]
00424 READ STORM
00425 Filename = STORM.001
00426 Comment = 010VCHM-Perth MTD IDP
00427 [STP:1.0:SDR= 3.00:PTOT= 51.36]
00428 # TAY RIVER
00429 # *****
00430 *****
00431 R005C00003-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00432 CALIS STANDHYD 1.0 01:DEV-P2 13.21 1.652 No.Date 1:02 29.24 668 .000
00433 [XIMP:56:TIMP:66]
00434 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00435 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00436 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00437 R005C00004-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00438 CALIS STANDHYD 1.0 01:SMW1 .77 .155 No.Date 1:00 29.24 668 .000
00439 [XIMP:56:TIMP:66]
00440 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00441 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00442 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00443 R005C00005-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00444 CALIS STANDHYD 1.0 01:OSG2 1.35 .266 No.Date 1:00 29.24 668 .000
00445 [XIMP:56:TIMP:66]
00446 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00447 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00448 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00449 R005C00006-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00450 ADD HYD + 1.0 02:SMW2 1.36 .332 No.Date 1:00 29.24 n/a .000
00451 + 1.0 02:SMW2 1.36 .332 No.Date 1:00 29.24 n/a .000
00452 + 1.0 02:OSG1 1.75 .272 No.Date 1:00 29.24 n/a .000
00453 SIM + 1.0 01:Pond1-In 12.61 1.619 No.Date 1:01 29.24 n/a .000
00454 R005C00007-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00455 ROUTE RESERVOIR > 1.0 02:Pond1-In 12.61 1.619 No.Date 1:01 29.24 n/a .000
00456 out <= 1.0 01:Pond1-out 12.61 .174 No.Date 1:53 29.23 n/a .000
00457 overlow <= 1.0 03:Pond1-Over .00 .000 No.Date 0:00 .00 n/a .000
00458 [MstOfsed=2745E+00 m3, TotOfVol=0.000E+00 m3, N-Ovf= 0, TotDurOfV= 0 hrs]
00459 *****
00460 R005C00008-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00461 CALIS STANDHYD 1.0 01:DEV-P3 14.48 2.448 No.Date 1:03 29.24 668 .000
00462 [XIMP:56:TIMP:66]
00463 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00464 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00465 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00466 R005C00009-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00467 CALIS STANDHYD 1.0 01:SMW3 1.45 .355 No.Date 1:00 35.82 695 .000
00468 [XIMP:56:TIMP:66]
00469 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00470 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00471 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00472 R005C00010-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00473 CALIS STANDHYD 1.0 01:SMW3 1.45 .355 No.Date 1:00 35.82 695 .000
00474 [XIMP:56:TIMP:66]
00475 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00476 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00477 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00478 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00479 ADD HYD + 1.0 02:SMW3 1.45 .355 No.Date 1:00 35.82 n/a .000
00480 + 1.0 02:SMW3 1.45 .355 No.Date 1:00 35.82 n/a .000
00481 + 1.0 02:OSG1 1.75 .272 No.Date 1:00 29.24 n/a .000
00482 SIM + 1.0 01:Pond2-In 15.93 3.121 No.Date 1:02 35.82 n/a .000
00483 R005C00012-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00484 ROUTE RESERVOIR > 1.0 02:Pond2-In 15.93 3.121 No.Date 1:02 35.82 n/a .000
00485 out <= 1.0 01:Pond2-out 15.93 .305 No.Date 1:48 35.81 n/a .000
00486 overlow <= 1.0 03:Pond1-Over .00 .000 No.Date 0:00 .00 n/a .000
00487 [MstOfsed=415E+00 m3, TotOfVol=0.000E+00 m3, N-Ovf= 0, TotDurOfV= 0 hrs]
00488 *****
00489 R005C00013-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00490 CALIS STANDHYD 1.0 01:DEV-P3 14.48 2.816 No.Date 1:03 35.82 695 .000
00491 [XIMP:56:TIMP:66]
00492 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00493 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00494 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00495 R005C00014-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00496 CALIS STANDHYD 1.0 01:SMW3 1.45 .355 No.Date 1:00 35.82 695 .000
00497 [XIMP:56:TIMP:66]
00498 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00499 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00500 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00501 R005C00015-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00502 ADD HYD + 1.0 02:SMW3 1.45 .355 No.Date 1:00 35.82 n/a .000
00503 + 1.0 02:SMW3 1.45 .355 No.Date 1:00 35.82 n/a .000
00504 + 1.0 02:OSG1 1.75 .272 No.Date 1:00 29.24 n/a .000
00505 SIM + 1.0 01:Pond2-In 15.93 3.121 No.Date 1:02 35.82 n/a .000
00506 R005C00016-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00507 ROUTE RESERVOIR > 1.0 02:Pond2-In 15.93 3.121 No.Date 1:02 35.82 n/a .000
00508 out <= 1.0 01:Pond2-out 15.93 .305 No.Date 1:48 35.81 n/a .000
00509 overlow <= 1.0 03:Pond1-Over .00 .000 No.Date 0:00 .00 n/a .000
00510 [MstOfsed=3773E+00 m3, TotOfVol=0.000E+00 m3, N-Ovf= 0, TotDurOfV= 0 hrs]
00511 *****
00512 # STORMS
00513 # *****
00514 ** END OF RUN : 49
00515 *****
00516 *****
00517 *****
00518 *****
00519 *****
00520 *****
00521 *****
00522 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00523 *****
00524 START [TZED= 0 hrs on 0]
00525 [MTCUT= 2 (1=imperial, 2=metric output)]
00526 [MTCUT= 2]
00527 [MTCUT= 2]
00528 [MTCUT= 2]
00529 # SWHYMO / INPUT DATA FILE
00530 # *****
00531 # Project Name : [Calvan Perth properties]
00532 # Project Number : [2118]
00533 # Date : [2023 JAN 26]
00534 # Modeller : [J]
00535 # License # : [2549237]
00536 # Company : [J.F. Sabourin and Associates]
00537 # Model developed to simulate runoff from subcatchments under post development conditions and to size SWM ponds
00538 *****
00539 R005C00011-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00540 CALIS STANDHYD 1.0 01:DEV-P2 13.21 2.063 No.Date 1:00 29.24 668 .000
00541 [XIMP:56:TIMP:66]
00542 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00543 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00544 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00545 R005C00012-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00546 CALIS STANDHYD 1.0 01:SMW3 1.45 .285 No.Date 1:00 29.24 668 .000
00547 [XIMP:56:TIMP:66]
00548 [Horton parameters: Fw 76.20:Frc 13.20:DCAY4:14: Fw =.00]
00549 [Previous area: IApex=4.67:SLP2+2.00:LDP= 40.0NDP:250:SCP=.0]
00550 [Impervious area: IAImp=1.57:SLP1+.50:LD1= 98.0MI+.013:SCI=.0]
00551 R005C00013-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00552 ADD HYD + 1.0 02:SMW2 1.36 .267 No.Date 1:00 29.24 n/a .000
00553 + 1.0 02:SMW2 1.36 .267 No.Date 1:00 29.24 n/a .000
00554 + 1.0 02:OSG2 1.75 .274 No.Date 1:00 29.24 n/a .000
00555 SIM + 1.0 01:Pond2-In 16.32 2.614 No.Date 1:02 29.24 n/a .000
00556 R005C00014-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00557 ROUTE RESERVOIR > 1.0 02:Pond2-In 16.32 2.614 No.Date 1:02 29.24 n/a .000
00558 out <= 1.0 01:Pond2-out 16.32 .399 No.Date 1:34 29.24 n/a .000
00559 overlow <= 1.0 03:Pond1-Over .00 .000 No.Date 0:00 .00 n/a .000
00560 [MstOfsed=318E+00 m3, TotOfVol=0.000E+00 m3, N-Ovf= 0, TotDurOfV= 0 hrs]
00561 *****
00562 # STORMS
00563 # *****
00564 ** END OF RUN : 24
00565 *****
00566 *****
00567 *****
00568 *****
00569 *****
00570 *****
00571 *****
00572 *****
00573 *****
00574 *****
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00647 *****
00648 # SWHYMO / INPUT DATA FILE
00649 # *****
00650 # Project Name : [Calvan Perth properties]
00651 # Project Number : [2118]
00652 # Date : [2023 JAN 26]
00653 # Modeller : [J.F. Sabourin and Associates]
00654 # License # : [2549237]
00655 # *****
00656 *****
00657 # Model developed to simulate runoff from subcatchments under post development conditions and to size SWM ponds
00658 *****
00659 R005C0002-----DtnIn-ID-NHYD-----AREAh-QPFArcms-TpeakDate_hh:mm-----Rvwm-R.C-----DWfms
00660 READ STORM
00661 Filename = STORM.001
00662 Comment = 020VCHM-Perth MTD IDP
00663 [STP:1.0:SDR= 3.00:PTOT= 57.24]
00664 # TAY RIVER
00665 # *****
00666 *****
00667 *****
00668 *****
00669 *****
00670 *****
00671 *****
00672 *****
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10000 *****

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00721 [XMPM:56:TIMP=.66]
00722 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00723 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00724 [Impervious area: IArea=1.57;SLPI=.50;LGI= 297.0;MI=.013;SCT=.0]
00725 R050:CO011-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00726 CALIS STANHYD 1.0 01:SWM2 1.36 .377 No.date 1:00 40.85 714 .000
00727 [XMPM:56:TIMP=.66]
00728 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00729 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00730 [Impervious area: IArea=1.57;SLPI=.50;LGI= 95.0;MI=.013;SCT=.0]
00731 R050:CO014-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00732 CALIS STANHYD 1.0 01:SWM2 1.75 .483 No.date 1:00 40.85 714 .000
00733 [XMPM:56:TIMP=.66]
00734 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00735 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00736 [Impervious area: IArea=1.57;SLPI=.50;LGI= 108.0;MI=.013;SCT=.0]
00737 R050:CO015-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00738 ADD HYD 1.0 02:DEV-P2 13.21 3.017 No.date 1:02 40.85 n/a .000
00739 + 1.0 02:SWM2 1.36 .377 No.date 1:00 40.85 n/a .000
00740 * 1.0 02:OS2S 1.75 .483 No.date 1:00 40.85 n/a .000
00741 SUM 1.0 01:POnd3-In 16.32 3.812 No.date 1:01 40.85 n/a .000
00742 R050:CO016-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00743 ROUTE RESERVOIR -> 1.0 02:POnd3-In 16.32 3.812 No.date 1:01 40.85 n/a .000
00744 out <= 1.0 03:POnd3-Out 16.32 .688 No.date 1:29 40.84 n/a .000
00745 overflow <= 1.0 03:POnd3-Over .00 .000 No.date 0:00 .00 n/a .000
00746 [MxdtOfsed=.4223E+00 m3, TotOfVol=.0000E+00 m3, N-Ofv= 0, TotDurOfv= 0 hrs]
00747 *****
00748 # STORMS
00749 *****
00750 ** END OF RUN # 98
00751 *****
00752 *****
00753 *****
00754 *****
00755 *****
00756 *****
00757 *****
00758 RUN#COMMANDS
00759 R059:CO001-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00760 START
00761 [TZERO = .00 hrs on 0]
00762 [NETOUT = 2 (Imperial, 2-metric output)]
00763 [NETFORM = 1]
00764 [NSUN = 009]
00765 *****
00766 # SWMHYD / INPUT DATA FILE
00767 *****
00768 # Project Name : [Caivan Perth properties]
00769 # Project Number : [218]
00770 # Date : [2023 JAN 26]
00771 # Modeller : [J]
00772 # Company : J.F. Sabourin and Associates
00773 # License # : 2549237
00774 *****
00775 # Model developed to simulate runoff from subcatchments under post development conditions and to size SWM ponds
00776 *****
00777 R059:CO002-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00778 READ STORM
00779 File name = STORM_001
00780 Comment = 100YRSCH24HR-Perth MTD IDF
00781 [SPT=10.0;SDUR= 3.00;PROT= 63.12]
00782 *****
00783 # TAY RIVER
00784 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00785 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00786 [Impervious area: IArea=1.57;SLPI=.50;LGI= 95.0;MI=.013;SCT=.0]
00787 R059:CO003-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00788 CALIS STANHYD 1.0 01:DEV-P1 10.49 2.815 No.date 1:02 46.06 731 .000
00789 [XMPM:56:TIMP=.66]
00790 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00791 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00792 [Impervious area: IArea=1.57;SLPI=.50;LGI= 264.0;MI=.013;SCT=.0]
00793 R059:CO004-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00794 CALIS STANHYD 1.0 01:SWM2 1.77 .250 No.date 1:00 46.06 731 .000
00795 [XMPM:56:TIMP=.66]
00796 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00797 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00798 [Impervious area: IArea=1.57;SLPI=.50;LGI= 71.0;MI=.013;SCT=.0]
00799 R059:CO005-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00800 CALIS STANHYD 1.0 01:SWM3 1.35 .432 No.date 1:00 46.06 731 .000
00801 [XMPM:56:TIMP=.66]
00802 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00803 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00804 [Impervious area: IArea=1.57;SLPI=.50;LGI= 311.0;MI=.013;SCT=.0]
00805 R059:CO006-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00806 ADD HYD 1.0 02:DEV-P1 10.49 2.815 No.date 1:02 46.06 n/a .000
00807 + 1.0 02:OS2S 1.75 .483 No.date 1:00 46.06 n/a .000
00808 * 1.0 02:OS2S 1.36 .377 No.date 1:00 46.06 n/a .000
00809 SIM 1.0 01:POnd3-In 12.61 3.439 No.date 1:01 46.06 n/a .000
00810 R059:CO007-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00811 ROUTE RESERVOIR -> 1.0 02:POnd3-In 12.61 3.439 No.date 1:01 46.06 n/a .000
00812 out <= 1.0 03:POnd3-Out 12.61 .415 No.date 1:43 46.03 n/a .000
00813 overflow <= 1.0 03:POnd3-Over .00 .000 No.date 0:00 .00 n/a .000
00814 [MxdtOfsed=.1411E+00 m3, TotOfVol=.0000E+00 m3, N-Ofv= 0, TotDurOfv= 0 hrs]
00815 R059:CO008-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00816 CALIS STANHYD 1.0 01:DEV-P3 14.48 3.792 No.date 1:02 46.06 731 .000
00817 [XMPM:56:TIMP=.66]
00818 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00819 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00820 [Impervious area: IArea=1.57;SLPI=.50;LGI= 411.0;MI=.013;SCT=.0]
00821 R059:CO009-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00822 CALIS STANHYD 1.0 01:SWM2 1.45 .462 No.date 1:00 46.06 731 .000
00823 [XMPM:56:TIMP=.66]
00824 [Horton parameters: Fw=76.20;Fp=13.20;DCAY=4.14; F= .00]
00825 [Previous area: IArea=4.67;SLPP=2.00;LDP= 40.0;MNP=.250;SCP=.0]
00826 [Impervious area: IArea=1.57;SLPI=.50;LGI= 99.0;MI=.013;SCT=.0]
00827 R059:CO010-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00828 ADD HYD 1.0 02:DEV-P2 14.48 3.792 No.date 1:02 46.06 n/a .000
00829 + 1.0 02:OS2S 1.75 .483 No.date 1:00 46.06 n/a .000
00830 * 1.0 02:OS2S 1.36 .377 No.date 1:00 46.06 n/a .000
00831 SIM 1.0 01:POnd3-In 15.93 4.211 No.date 1:02 46.06 n/a .000
00832 R059:CO011-----DtmIn-ID:HYND-----AREAh-QPEARcMs-TpaeDate_hh:mm-----RvM-R.C-----DWfMcs
00833 ROUTE RESERVOIR -> 1.0 02:POnd3-In 15.93 4.211 No.date 1:02 46.06 n/a .000
00834 out <= 1.0 03:POnd3-Out 15.93 .449 No.date 1:44 46.05 n/a .000
00835 overflow <= 1.0 03:POnd3-Over .00 .000 No.date 0:00 .00 n/a .000
00836 [MxdtOfsed=.4678E+00 m3, TotOfVol=.0000E+00 m3, N-Ofv= 0, TotDurOfv= 0 hrs]
00837 *****
00838 # STORMS
00839 *****
00840 # GRANTS CREEK
00841 *****
00842 *****
00843 *****
00844 *****
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01100 *****

01081 [XIMP:56:TIMP:66]
01082 [Horton parameters: Fw= 76.20:Fc= 13.20:DCAV4:14: F= .00]
01083 [Previous area: IApex= 4.67:SLPP2:2.00:LDW= 40.0:MPD:250:SCP= .0]
01084 [Impervious area: IApex= 1.57:SLP1: .50:LDI= 98.0:MMI: .013:ICI= .0]
01085 R0105-C0001-----DRAIN-ID:INHYD-----AREAA-GPEARcm-TpaeDate_hh:mm-----RvM-R.C-----DWFMcm
01086 CALIB STANBYHD 1.0 01:00S2 1.75 .317 No.Date 12:00 47.59 677 .000
01087 [XIMP:56:TIMP:66]
01088 [Horton parameters: Fw= 76.20:Fc= 13.20:DCAV4:14: F= .00]
01089 [Previous area: IApex= 4.67:SLPP2:2.00:LDW= 40.0:MPD:250:SCP= .0]
01090 [Impervious area: IApex= 1.57:SLP1: .50:LDI= 98.0:MMI: .013:ICI= .0]
01091 R0105-C0001-----DRAIN-ID:INHYD-----AREAA-GPEARcm-TpaeDate_hh:mm-----RvM-R.C-----DWFMcm
01092 ADD HYD + 1.0 02:DEV2 1.36 .250 No.Date 12:00 47.59 n/a .000
01093 SIMM + 1.0 01:POSD2-in 16.32 2.411 No.Date 12:01 47.59 n/a .000
01094 R0105-C0001-----DRAIN-ID:INHYD-----AREAA-GPEARcm-TpaeDate_hh:mm-----RvM-R.C-----DWFMcm
01095 ROUTE RESERVOIR -> 1.0 02:POSD-in 16.32 2.411 No.Date 12:01 47.59 n/a .000
01096 out <= 1.0 03:POSD-out 16.32 .672 No.Date 12:22 47.58 n/a .000
01097 overlow <= 1.0 03:POSD-Over 0.00 .000 No.Date 0:00 .00 n/a .000
01098 [MstOfUsed:4182=00 m3, TotOfVol:0.0000=00 m3, N-OfV= 0, TotDurOfV= 0 hrs]
01099 *****
01100 ** END OF RUN : 109
01101 *****
01102 # STORMS
01103 *****
01104 ** END OF RUN : 109
01105 *****
01106 *****
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01441 [XIMP=.56;TIMP=.66]
01442 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01443 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01444 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 108.;NMI=.013;SCI=.0]
01445 R0150:CO001-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01446 ADD HYD + 1.0 02:DEV-P2 13.21 3.554 Mo_date 12:01 75.82 n/a .000
01447 + 1.0 02:RMSL 1.36 .460 Mo_date 12:00 84.07 n/a .000
01448 + 1.0 02:OOS2 1.75 .527 Mo_date 12:00 75.81 n/a .000
01449 SIM+ 1.0 01:Pond2-In 16.32 4.464 Mo_date 12:01 75.82 n/a .000
01450 R0150:CO001-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01451 ROUTE RESERVOIR -> 1.0 02:Pond2-In 16.32 4.464 Mo_date 12:01 75.82 n/a .000
01452 out <= 1.0 01:Pond2-Out 16.32 1.652 Mo_date 12:13 75.80 n/a .000
01453 overflow <= 1.0 03:Pond2-Over .00 .000 Mo_date 0:00 .00 n/a .000
01454 [MsdToSeed=.5874E+00 n3, TotDvVol=.0000E+00 n3, N-Ovfl= 0, TotDurDvfl= 0.hrs]
01455 #####
01456 # STORMS
01457 #####
01458 ** END OF RUN : 198
01459
01460 -----
01461
01462
01463
01464
01465
01466 RUN:COMMANDS
01467 R0159:CO002-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01468 START
01469 [TZERO=.00 hrs on 0]
01470 [METOPT= 2 (1=imperial, 2=metric output)]
01471 [NETFORM= 1]
01472 [NMIN= 0.19]
01473 #####
01474 # SWMHYMO // INPUT DATA FILE
01475 #####
01476 # Project Name : [Calvin Perth properties]
01477 # Project Number : [2118]
01478 # Date : [2023 JAN 26]
01479 # Modeler : [JFS]
01480 # Company : [J.F. Sabourin and Associates]
01481 # License # : [2549237]
01482 #####
01483 # Model developed to simulate runoff from subcatchments under post development conditions and to size SWM ponds
01484 #####
01485 R0159:CO002-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01486 READ STORM
01487 File name = STORM.001
01488 Comment = 100YRSC24HR-Perth MTD IDF
01489 [IDF=10.00;SDRM= 24.00;PTOT= 117.60]
01490 #####
01491 # TAY RIVER
01492 #####
01493 R0159:CO003-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01494 CALIB STANDHYD 1.0 01:DEV-P1 10.49 3.184 Mo_date 12:01 84.07 .715 .000
01495 [XIMP=.56;TIMP=.66]
01496 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01497 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01498 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 264.;NMI=.013;SCI=.0]
01499 R0159:CO004-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01500 CALIB STANDHYD 1.0 01:RMSL 10.49 3.184 Mo_date 12:01 84.07 .715 .000
01501 [XIMP=.56;TIMP=.66]
01502 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01503 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01504 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 71.;NMI=.013;SCI=.0]
01505 R0159:CO005-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01506 CALIB STANDHYD 1.0 01:OOS1 10.49 3.184 Mo_date 12:01 84.07 .715 .000
01507 [XIMP=.56;TIMP=.66]
01508 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01509 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01510 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 95.;NMI=.013;SCI=.0]
01511 R0159:CO006-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01512 ADD HYD + 1.0 02:DEV-P1 10.49 3.184 Mo_date 12:01 84.06 n/a .000
01513 + 1.0 02:RMSL 1.36 .460 Mo_date 12:00 84.07 n/a .000
01514 + 1.0 02:OOS1 1.75 .527 Mo_date 12:00 75.81 n/a .000
01515 SIM+ 1.0 01:Pond1-In 12.61 3.672 Mo_date 12:01 84.07 n/a .000
01516 R0159:CO007-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01517 ROUTE RESERVOIR -> 1.0 01:Pond1-In 12.61 3.672 Mo_date 12:01 84.07 n/a .000
01518 out <= 1.0 01:Pond1-Out 12.61 .795 Mo_date 12:22 84.05 n/a .000
01519 overflow <= 1.0 03:Pond1-Over .00 .000 Mo_date 0:00 .00 n/a .000
01520 [MsdToSeed=.601E+00 n3, TotDvVol=.0000E+00 n3, N-Ovfl= 0, TotDurDvfl= 0.hrs]
01521 R0159:CO008-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01522 CALIB STANDHYD 1.0 01:DEV-P3 14.48 4.320 Mo_date 12:01 84.07 .715 .000
01523 [XIMP=.56;TIMP=.66]
01524 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01525 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01526 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 311.;NMI=.013;SCI=.0]
01527 R0159:CO009-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01528 CALIB STANDHYD 1.0 01:RMSL 14.48 4.320 Mo_date 12:00 84.07 .715 .000
01529 [XIMP=.56;TIMP=.66]
01530 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01531 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01532 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 95.;NMI=.013;SCI=.0]
01533 R0159:CO010-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01534 ADD HYD + 1.0 02:DEV-P3 14.48 4.320 Mo_date 12:01 84.07 n/a .000
01535 + 1.0 02:RMSL 1.36 .460 Mo_date 12:00 84.06 n/a .000
01536 + 1.0 02:OOS2 1.75 .586 Mo_date 12:00 84.07 n/a .000
01537 SIM+ 1.0 01:Pond3-In 15.93 4.797 Mo_date 12:01 84.07 n/a .000
01538 R0159:CO011-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01539 ROUTE RESERVOIR -> 1.0 02:Pond3-In 15.93 4.797 Mo_date 12:01 84.07 n/a .000
01540 out <= 1.0 01:Pond3-Out 15.93 1.005 Mo_date 12:23 84.06 n/a .000
01541 overflow <= 1.0 03:Pond3-Over .00 .000 Mo_date 0:00 .00 n/a .000
01542 [MsdToSeed=.7610E+00 n3, TotDvVol=.0000E+00 n3, N-Ovfl= 0, TotDurDvfl= 0.hrs]
01543 #####
01544 # GRANTS CREEK
01545 #####
01546 R0159:CO012-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01547 CALIB STANDHYD 1.0 01:DEV-P2 13.21 3.562 Mo_date 12:01 84.07 .715 .000
01548 [XIMP=.56;TIMP=.66]
01549 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01550 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01551 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 297.;NMI=.013;SCI=.0]
01552 R0159:CO013-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01553 CALIB STANDHYD 1.0 01:RMSL 13.21 3.562 Mo_date 12:00 84.07 .715 .000
01554 [XIMP=.56;TIMP=.66]
01555 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01556 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01557 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 95.;NMI=.013;SCI=.0]
01558 R0159:CO014-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01559 CALIB STANDHYD 1.0 01:OOS2 1.75 .586 Mo_date 12:00 84.07 .715 .000
01560 [XIMP=.56;TIMP=.66]
01561 [Horton parameters: Fw= 76.20;Pc= 13.20;DCAY=4.14; Fw=.00]
01562 [Previous area: IArea= 4.67;SLPP=2.00;LDP= 40.;NMP=.250;SCP=.0]
01563 [Impervious area: IAlmp= 1.57;SLPI=.50;LGI= 108.;NMI=.013;SCI=.0]
01564 R0159:CO015-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01565 ADD HYD + 1.0 02:DEV-P2 13.21 3.562 Mo_date 12:01 84.07 n/a .000
01566 + 1.0 02:RMSL 1.36 .460 Mo_date 12:00 84.07 n/a .000
01567 + 1.0 02:OOS2 1.75 .586 Mo_date 12:00 84.07 n/a .000
01568 SIM+ 1.0 01:Pond2-In 16.32 4.975 Mo_date 12:01 84.07 n/a .000
01569 R0159:CO016-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01570 ROUTE RESERVOIR -> 1.0 02:Pond2-In 16.32 4.975 Mo_date 12:01 84.07 n/a .000
01571 out <= 1.0 01:Pond2-Out 16.32 1.930 Mo_date 12:13 84.06 n/a .000
01572 overflow <= 1.0 03:Pond2-Over .00 .000 Mo_date 0:00 .00 n/a .000
01573 [MsdToSeed=.643E+00 n3, TotDvVol=.0000E+00 n3, N-Ovfl= 0, TotDurDvfl= 0.hrs]
01574 #####
01575 # STORMS
01576 #####
01577 R0159:CO002-----Dhain-ID:IRVYD-----AREAb-QPEARms-TpeakDate,hh:mm-----RvM-R-C-----DWPFcms
01578 FINISH
01579 #####
01580 WARNINGS / ERRORS / NOTES
01581 #####
01582 Simulation ended on 2023-02-16 at 15:31:37
01583 #####
01584

```

APPENDIX D/



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May 31, 2024

Project Number: 2118

Rideau Valley Conservation Authority
3889 Rideau Valley Drive
Manotick, ON
K4M 1A5

Attention: Eric Lalande, MCIP, RPP

Subject: Perth Golf Course – Proposed Floodplain Mapping Amendment

Introduction

As a part of the proposed Perth Golf Course development, it is important that accurate floodplain mapping is established to ensure that a reliable cut-fill analysis can be prepared for the site. The current official floodplain mapping for this area is based on the 2013 model of the Tay River produced by the Rideau Valley Conservation Authority (RVCA). According to the RVCA 2013 Tay River Floodplain Mapping report, the mapping was completed using data acquired as a part of Land Information Ontario's (LIO) Digital Raster Acquisition Project Eastern Ontario (DRAPE) conducted in the spring of 2008 and 2009 based on RGB orthophotography. There is now new topographic LiDAR data of a higher resolution and quality available that was obtained in 2022. In the interest of using the best available information, JFSA reviewed and amended the floodplain mapping extents for this site using the latest available topographic data, before any proposed cut-fill analysis is completed.

The following memo outlines the procedure for completing this mapping amendment. Note that no modifications have been made to the existing hydraulic model produced by RVCA in 2013, simply the flood elevations have been remapped using the latest available LiDAR. Additionally, the mapping updates outlined in this report are confined to the Perth Golf Course property, all mapping outside of this area has remained unaltered from the 2013 RVCA study.

2022 LiDAR

Airborne Imaging Inc. on behalf of the RVCA, conducted LiDAR acquisition flights over the subject lands between April 18 - 30th, 2022 as part of a broader data acquisition project coordinated between the various Eastern Ontario Conservation Authorities. The LiDAR acquired as a part of this project is publicly available to download from the Land Information Ontario's (LIO) Digital Terrain Model GIS interface¹ and has been provided at a 0.5 m cell resolution. An overview of the LiDAR for the subject area has been provided in **Figure A1 in Attachment A**

Ground Truthing

To ensure that this LiDAR is sufficiently accurate at this location for floodplain mapping purposes, JFSA staff were on site on May 9th, 2024, with an RTK GPS unit to ground truth the LiDAR. The intention was to collect survey points to assess both the LiDAR's Non-Vegetated Vertical Accuracy (NVA) and Vegetated Vertical Accuracy (VVA). While on site JFSA staff recorded a total of **152** topographic points throughout the site, **59** points on hard surfaces and **93** points on vegetated surfaces. **Figure A2 in Attachment A** provides a visual overview of the points collected.

¹ <https://geohub.lio.gov.on.ca/maps/mnrf::ontario-digital-terrain-model-lidar-derived/>

The elevations recorded for each topographic survey point were then compared with the corresponding LiDAR elevation at each specific location. A full summary of this comparison has been provided in Attachment A. A statistical analysis was then completed to assess the LiDAR's Non-Vegetated Vertical Accuracy (NVA) and Vegetated Vertical Accuracy (VVA). Based on this analysis, the NVA had a 95% confidence interval of **4.2 cm** and the VVA had a 95th percentile value of **11.5 cm**, both of which are within the acceptable range for Level 1 (highest) Flood Mapping Risk Level as specified in Table 3-1 of the MNRF Flooding Hazards: Data Survey and Mapping Specifications bulletin. Therefore, it is concluded that the 2022 LiDAR is appropriate for use for floodplain mapping in this area.

Vertical Datum Conversion

The LiDAR, acquired in 2022, uses the latest Canadian Geodetic Vertical Datum of 2013 (CGVD2013), which is now the new standard for elevations across Canada. The floodplain mapping completed by RVCA in 2013 used the older CGVD28:78 datum. Before completing the mapping update the LiDAR had to be converted to the older CGVD28:78 datum to match the elevations specified in the floodplain mapping model.

Natural Resources Canada (NRC) released a guide to the *Heights Reference System Modernization (2020)*² which provides guidance on how to approach systematic differences of the new vertical datum. Page 6 of this document states: “*Lastly, the third category represents those who transfer heights with precision of less than 2 cm over small regions (e.g. municipal infrastructure). For these users the difference between CGVD28 and CGVD2013 should be considered, but generally applying a constant offset will suffice.*”

To establish an appropriate offset value to apply to the LiDAR data, existing survey benchmark data that is publicly available through the Ministry of Natural Resources and Forestry (MNRF) Control Survey Information Exchange (COSINE) GIS portal³ was acquired. There was a total of 10 existing survey benchmarks surrounding the subject site that have established vertical datums using both CGVD28:78 and CGVD2013. From this analysis, it was found that the average offset between the two data was **0.324 m**, with a standard deviation of **1mm**. The small standard deviation indicates that applying a single vertical offset to this area is appropriate. **Figure A3** outlines the location of the benchmarks used in this analysis. Attachment A also includes the full COSINE benchmark reports and a summary of the elevation differences between the two data for the selected benchmarks.

Floodplain Mapping Amendment Process

The floodplain mapping amendment has been completed using GIS software, based on the regulatory elevations at each cross section from the RVCA model of record, underlaid with the 2022 LiDAR of the subject area. Note that only the floodplain within the Perth Golf Course property boundary has been updated, all other lands outside of this area remain unaltered from RVCA's original 2013 regulatory boundary.

The following describes the approach adopted for this floodplain mapping amendment, which was developed in part with consultation with RVCA:

1. The cross-sections have been extracted from the HEC-RAS model and the associated Energy Gradeline Elevations (EGL) - as presented in the RVCA 2013 Tay River Floodplain mapping Report – have been cross-referenced at each location.
2. A 100-year EGL surface was then created which linearly interpolated the EGL from each of the model cross-sections.

² [https://natural-resources.canada.ca/sites/nrcan/files/files/pdf/Height_reference_system_modernization_\(EN\).pdf](https://natural-resources.canada.ca/sites/nrcan/files/files/pdf/Height_reference_system_modernization_(EN).pdf)

³ <https://www.lioapplications.lrc.gov.on.ca/COSINE/index.html?viewer=COSINE.OntarioViewer&locale=en-CA>

3. Using GIS processes the EGL surface was then compared with the latest topography, and only locations where the EGL was greater than the terrain returned.
4. The remaining areas of the surface were then converted to a polygon vector.
5. This polygon vector was visually inspected to remove low-lying areas identified as flooded that were not hydraulically connected to the dominant floodplain.
6. This layer was then refined through JFSA's internal GIS Floodplain Cleaning Tool which was developed in consultation with RVCA's GIS technical staff. The cleaning tools included: the removal of any holes with areas less than 100 m², the removal of isolated island areas with an area less than 100 m², and the simplification of line work to remove any noise in the floodplain extents due to converting from a raster to a polygon.

The amended floodplain extents within the Perth Golf Course property have been provided in **Figure 1**. Digital copies of this floodplain extent have also been provided. To provide some context to the changes as a part of the addendum, the official RVCA floodplain extents for this area have also been provided in **Figure 2**.

Conclusion

In conclusion, this update to the floodplain mapping for the Perth Golf Course area using the latest available topographic data is a necessary and standard best practice to ensure a reliable cut-fill analysis for the proposed development site. By utilizing the latest available LiDAR data and ground truthing this data, the accuracy of the floodplain mapping within the subject site has been refined, providing a solid foundation for future development decisions.

Yours truly,
JFSA Canada Inc.



Jonathon Burnet, B.Eng, P.Eng
Senior Water Resources Engineer

cc: J.F Sabourin, M.Eng, P.Eng
Director of Water Resources Projects

Figures

Figure 1: JFSA 2024 Amended Floodplain Mapping

Figure 2: RVCA Official Floodplain Mapping

Attachments

Attachment A: LiDAR Ground Truthing and Datum Conversion

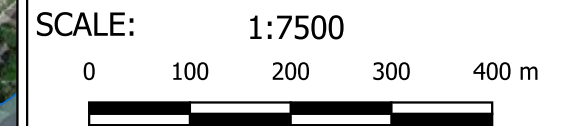




Legend

- Proposed Floodplain Extents
- Property Boundary

NOTE: This mapping is not to be used as official floodplain mapping without RVCA written consent.



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CAIVAN
 COMMUNITIES

Perth Golf Course

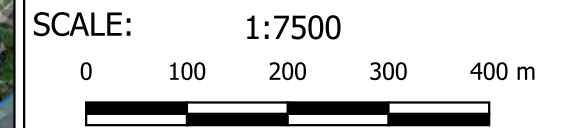
Figure 1: Perth Golf Course
 JFSA 2024 Floodplain Amendment

PROJECT	2118-21
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DATE	May 2024



Legend

- RVCA Official Floodplain
- Property Boundary



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Figure 2: Perth Golf Course
 RVCA Official Floodplain

PROJECT	2118-21
DRAWN	JB
DATE	May 2024

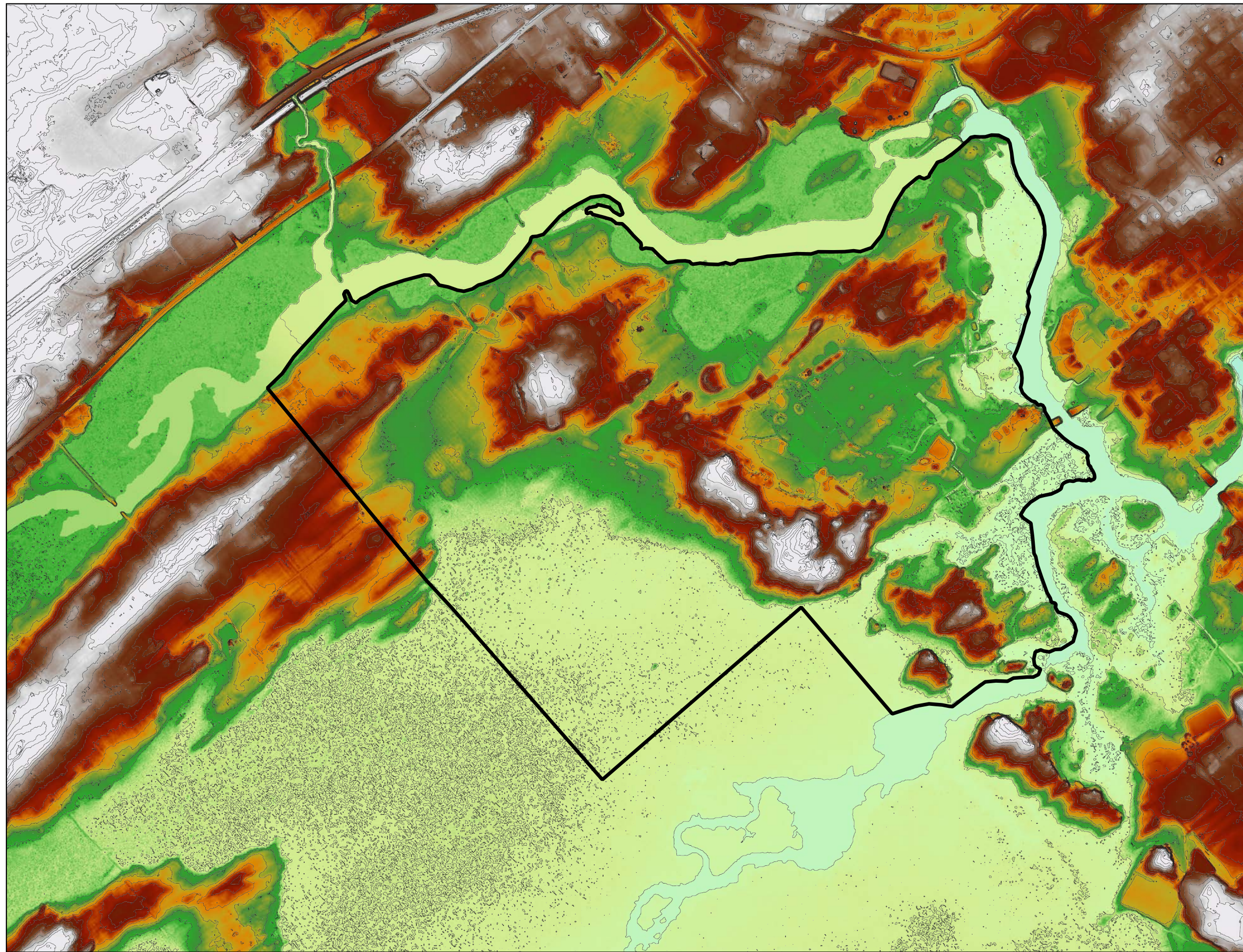


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Attachment A

LiDAR Ground Truthing and Datum Conversion



Legend

Property Boundary

Contours

— 0.5 m

LiDAR Elevations

(m)

133.00

134.00

135.00

136.00

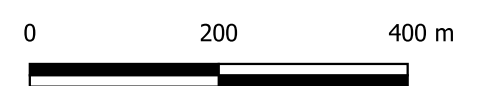
137.00

138.00

139.00



SCALE: 1:8000



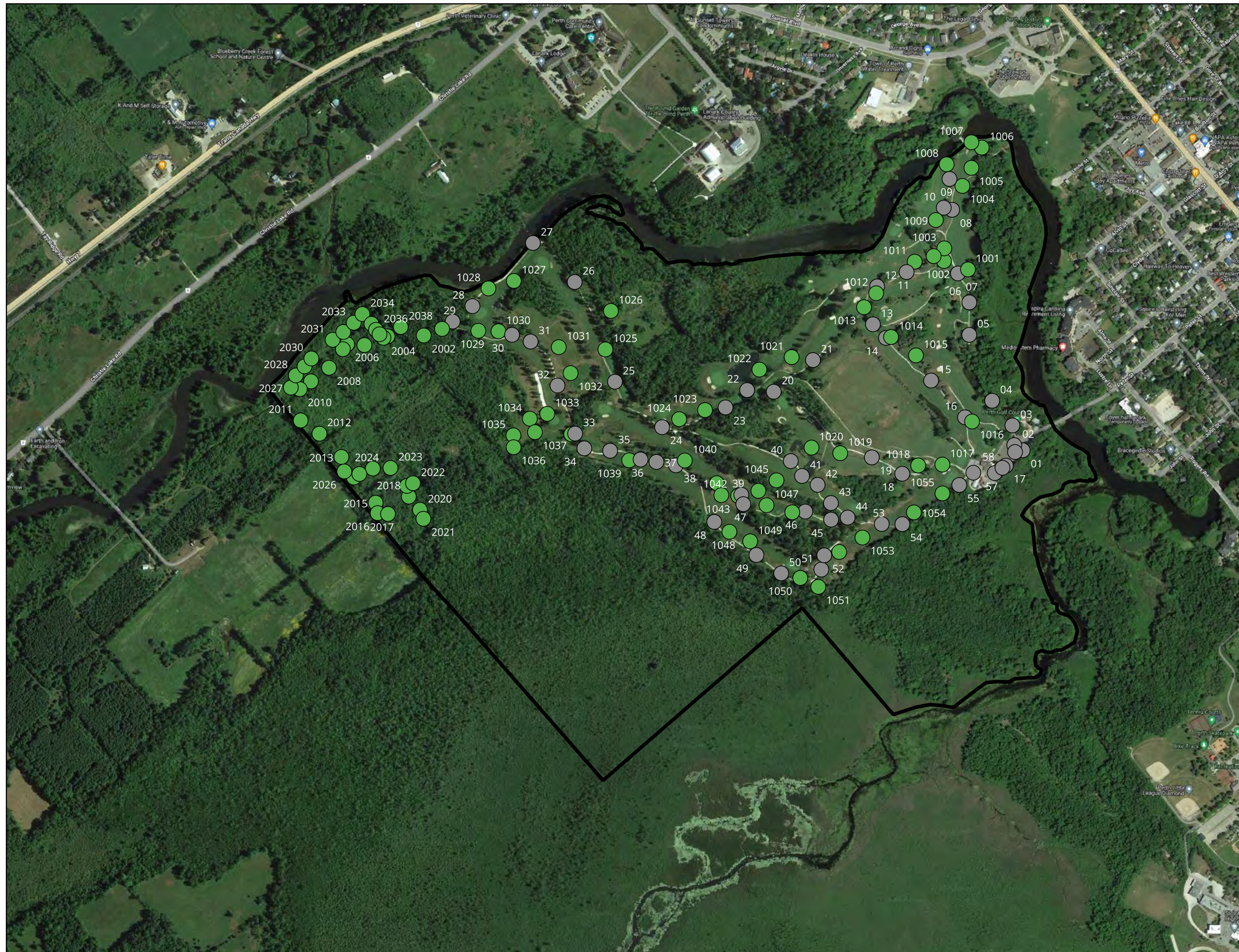
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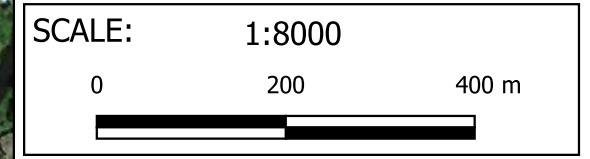
Figure A1: 2022 LiDAR

PROJECT	2118-21
DRAWN	JB
DATE	May 2024



Legend

- Topographic Survey Points
- Hard Surface
- Vegetated Surface
- ▭ Property Boundary



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Perth Golf Course

Figure E2: LiDAR Ground Truthing

PROJECT	2118-21
DRAWN	JB
DATE	May 2024

Table A1 - Hard Surface Ground Truthing

ID	X	Y	Z	CODE	DTM	Difference
1	400575.1	4972226.0	134.794	Hard	134.790	-0.004
2	400557.0	4972237.1	135.110	Hard	135.120	0.010
3	400553.1	4972279.3	135.508	Hard	135.480	-0.028
4	400509.5	4972332.4	133.819	Hard	133.840	0.021
5	400460.4	4972473.9	133.878	Hard	133.860	-0.018
6	400460.5	4972543.7	135.017	Hard	135.020	0.003
7	400436.0	4972605.9	134.069	Hard	134.090	0.021
8	400423.6	4972742.6	134.083	Hard	134.060	-0.023
9	400418.0	4972808.9	134.464	Hard	134.470	0.006
10	400405.6	4972747.1	134.252	Hard	134.250	-0.002
11	400325.8	4972608.9	136.185	Hard	136.200	0.015
12	400262.6	4972578.0	136.496	Hard	136.460	-0.036
13	400253.5	4972496.6	136.581	Hard	136.600	0.019
14	400281.5	4972467.5	135.949	Hard	135.980	0.031
15	400378.5	4972375.8	134.397	Hard	134.400	0.003
16	400451.3	4972296.9	134.041	Hard	134.040	-0.001
17	400538.9	4972193.1	134.573	Hard	134.590	0.017
18	400316.5	4972175.6	134.549	Hard	134.530	-0.019
19	400251.1	4972210.7	134.297	Hard	134.280	-0.017
20	400040.5	4972351.8	134.955	Hard	134.960	0.005
21	400124.7	4972419.8	134.730	Hard	134.770	0.040
22	399983.3	4972355.3	135.386	Hard	135.420	0.034
23	399937.0	4972318.1	135.578	Hard	135.590	0.012
24	399800.7	4972275.6	136.537	Hard	136.540	0.003
25	399699.5	4972373.3	136.111	Hard	136.130	0.019
26	399613.0	4972587.4	134.874	Hard	134.850	-0.024
27	399523.3	4972671.3	134.461	Hard	134.450	-0.011
28	399393.4	4972535.6	134.797	Hard	134.770	-0.027
29	399350.9	4972502.6	135.742	Hard	135.720	-0.022
30	399478.4	4972474.0	135.282	Hard	135.300	0.018
31	399518.4	4972459.5	136.165	Hard	136.210	0.045
32	399576.0	4972365.1	139.340	Hard	139.370	0.030
33	399613.9	4972261.9	134.760	Hard	134.750	-0.010
34	399633.6	4972230.1	134.335	Hard	134.350	0.015
35	399689.1	4972225.0	134.350	Hard	134.370	0.020
36	399754.5	4972206.2	134.836	Hard	134.860	0.024
37	399788.6	4972201.0	135.057	Hard	135.050	-0.007
38	399830.4	4972193.8	135.588	Hard	135.610	0.022
39	399970.9	4972130.5	138.806	Hard	138.800	-0.006
40	400077.1	4972202.7	134.541	Hard	134.530	-0.011
41	400101.3	4972170.8	134.496	Hard	134.440	-0.056
42	400134.6	4972151.8	134.841	Hard	134.850	0.009

Table A1 - Hard Surface Ground Truthing

ID	X	Y	Z	CODE	DTM	Difference
43	400163.3	4972112.6	135.303	Hard	135.310	0.007
44	400199.2	4972081.4	136.396	Hard	136.400	0.004
45	400163.7	4972077.0	136.181	Hard	136.200	0.019
46	400108.3	4972094.4	137.147	Hard	137.150	0.003
47	399975.1	4972110.3	138.523	Hard	138.510	-0.013
48	399912.8	4972072.2	135.713	Hard	135.690	-0.023
49	400003.3	4972001.0	136.950	Hard	136.950	0.000
50	400056.6	4971961.7	138.353	Hard	138.360	0.007
51	400142.7	4971969.8	140.771	Hard	140.740	-0.031
52	400149.0	4972000.4	137.131	Hard	137.160	0.029
53	400272.4	4972067.7	135.136	Hard	135.190	0.054
54	400316.7	4972067.3	134.357	Hard	134.380	0.023
55	400439.0	4972151.5	134.003	Hard	134.040	0.037
56	400469.1	4972177.8	134.390	Hard	134.390	0.000
57	400515.1	4972177.0	134.668	Hard	134.680	0.012
58	400531.7	4972188.4	134.662	Hard	134.670	0.008
59	400558.7	4972222.3	134.948	Hard	134.950	0.002

Statistical Summary - Hard Surfaces - Ground Truthing

Table A2: Elevation Difference Statistical Analysis

Statistical Analysis	Unit	Value
Mean	(m)	0.004
Standard Error	(m)	0.003
Median	(m)	0.005
Mode	(m)	0.021
Standard Deviation	(m)	0.022
Sample Variance	(m)	0.000
Kurtosis	-	0.123
Skewness	-	-0.238
Range	(m)	0.110
Minimum	(m)	-0.056
Maximum	(m)	0.054
Sum	(m)	0.258
Count	-	59
RMSE	(m)	0.022
95th Confidence interval (NVA)	(m)	0.042
95th Percentile (VVA)	(m)	0.037

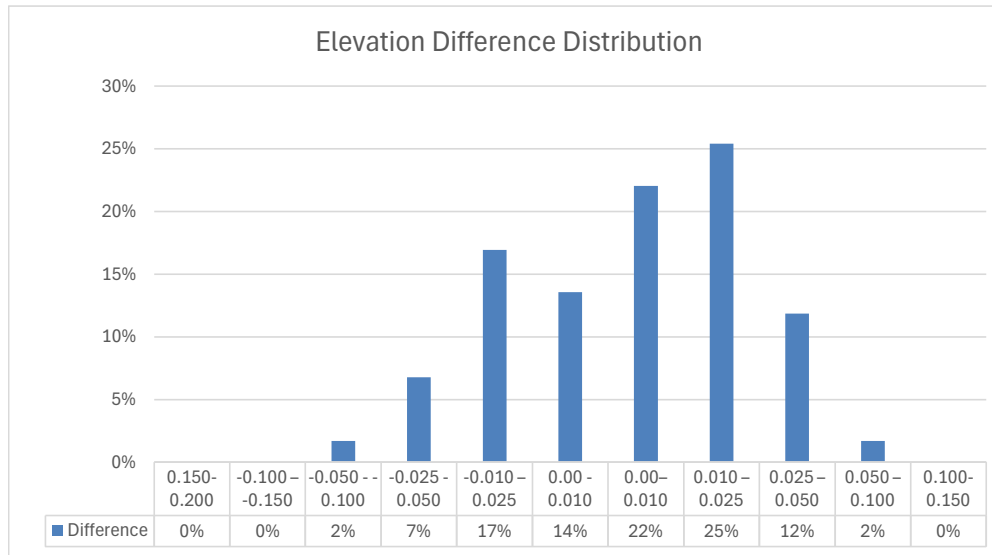
Table A3: Elevation Difference Bins

Bins	Range	Count	Percent
-0.300	<-0.200	0	0%
-0.150	0.150-0.200	0	0%
-0.100	-0.100 - -0.150	0	0%
-0.050	-0.050 - -0.100	1	2%
-0.025	-0.025 -0.050	4	7%
-0.010	-0.010 - 0.025	10	17%
0.000	0.00 - 0.010	8	14%
0.010	0.00-0.010	13	22%
0.025	0.010 - 0.025	15	25%
0.050	0.025 - 0.050	7	12%
0.100	0.050 - 0.100	1	2%
0.150	0.100-0.150	0	0%
0.300	0.150-0.200	0	0%
Sum		59	100.0%

Table A4: Elevation

Distribution Summary	
± 1 cm	36%
± 2.5 cm	78%
± 5 cm	97%
± 10 cm	98%
± 15 cm	100%
± 30 cm	100%

Elevation Difference Distribution



Elevation Difference Distribution Summary

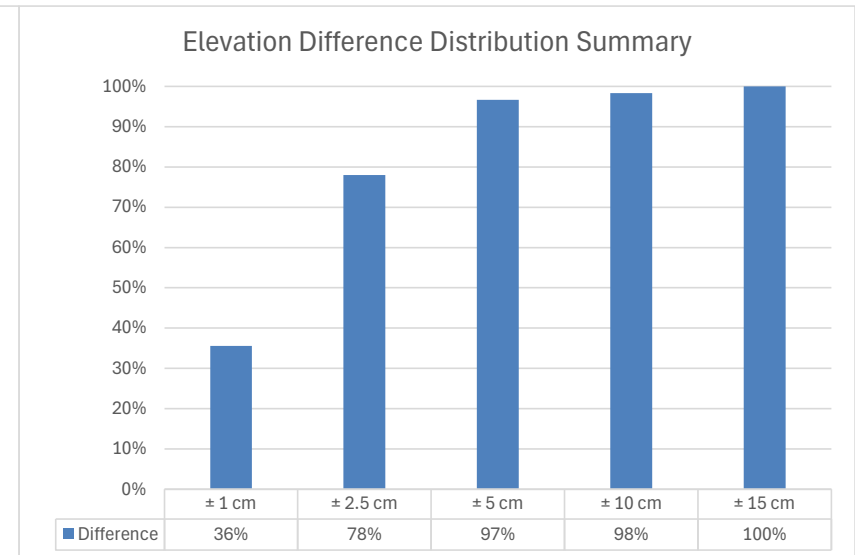


Table A5 -Veg Surface Ground Truthing

ID	X	Y	Z	CODE	DTM	Difference
1001	400457.7	4972614.4	133.835	Veg	133.840	0.005
1002	400406.1	4972632.5	136.794	Veg	136.810	0.016
1003	400406.7	4972659.8	134.654	Veg	134.660	0.006
1004	400445.6	4972793.9	134.542	Veg	134.580	0.038
1005	400465.0	4972832.7	134.328	Veg	134.350	0.022
1006	400487.1	4972876.2	134.691	Veg	134.710	0.019
1007	400465.0	4972887.6	134.489	Veg	134.480	-0.009
1008	400411.8	4972840.2	134.198	Veg	134.220	0.022
1009	400388.6	4972720.6	135.478	Veg	135.500	0.022
1010	400384.0	4972643.1	135.193	Veg	135.210	0.017
1011	400343.2	4972631.1	135.541	Veg	135.540	-0.001
1012	400260.4	4972563.3	136.453	Veg	136.480	0.027
1013	400234.4	4972533.3	136.575	Veg	136.600	0.025
1014	400292.4	4972469.3	136.047	Veg	136.050	0.003
1015	400345.9	4972429.8	134.472	Veg	134.490	0.018
1016	400466.4	4972287.8	134.252	Veg	134.250	-0.002
1017	400402.7	4972195.3	135.328	Veg	135.340	0.012
1018	400350.7	4972193.4	134.971	Veg	134.990	0.019
1019	400183.3	4972219.7	134.811	Veg	134.830	0.019
1020	400121.6	4972231.7	134.252	Veg	134.260	0.008
1021	400078.9	4972426.1	135.977	Veg	136.000	0.023
1022	400009.6	4972399.3	134.969	Veg	134.990	0.021
1023	399892.9	4972312.4	135.532	Veg	135.550	0.018
1024	399837.4	4972292.8	136.436	Veg	136.430	-0.006
1025	399678.9	4972442.6	136.364	Veg	136.420	0.056
1026	399690.8	4972525.3	137.460	Veg	137.470	0.010
1027	399482.3	4972589.8	134.542	Veg	134.560	0.018
1028	399427.8	4972573.7	134.560	Veg	134.600	0.040
2001	399328.1	4972486.8	136.229	Veg	136.300	0.071
2002	399288.8	4972472.7	136.315	Veg	136.400	0.085
2003	399211.0	4972470.7	135.451	Veg	135.490	0.039
2004	399202.2	4972467.6	135.472	Veg	135.560	0.088
2005	399161.4	4972451.9	135.263	Veg	135.330	0.067
2006	399124.3	4972453.8	135.117	Veg	135.200	0.083
2007	399115.1	4972443.2	135.151	Veg	135.220	0.069
2008	399085.4	4972403.3	135.162	Veg	135.230	0.068
2009	399046.6	4972374.0	135.178	Veg	135.210	0.032
2010	399022.6	4972357.4	135.518	Veg	135.540	0.022
2011	399024.5	4972290.2	135.716	Veg	135.770	0.054
2012	399064.8	4972261.7	136.665	Veg	136.730	0.065
2013	399112.0	4972211.6	135.814	Veg	135.870	0.056
2014	399138.6	4972167.5	135.378	Veg	135.400	0.022

Table A5 -Veg Surface Ground Truthing

ID	X	Y	Z	CODE	DTM	Difference
2015	399185.5	4972113.3	135.050	Veg	135.190	0.140
2016	399189.7	4972091.0	135.232	Veg	135.340	0.108
2017	399212.0	4972089.3	135.180	Veg	135.250	0.070
2018	399256.7	4972126.6	135.163	Veg	135.230	0.067
2019	399252.7	4972149.4	134.833	Veg	134.870	0.037
2020	399280.7	4972098.2	134.569	Veg	134.610	0.041
2021	399288.1	4972078.9	135.740	Veg	135.790	0.050
2022	399265.7	4972155.2	134.517	Veg	134.540	0.023
2023	399217.4	4972188.1	134.425	Veg	134.500	0.075
2024	399179.5	4972186.8	134.637	Veg	134.760	0.123
2025	399150.5	4972175.1	135.132	Veg	135.170	0.038
2026	399118.2	4972180.6	135.551	Veg	135.610	0.059
2027	399003.3	4972360.7	135.538	Veg	135.630	0.092
2028	399014.2	4972386.8	135.419	Veg	135.500	0.081
2029	399032.9	4972406.2	135.255	Veg	135.320	0.065
2030	399047.5	4972423.1	135.160	Veg	135.220	0.060
2031	399093.1	4972463.3	135.004	Veg	135.070	0.066
2032	399116.0	4972480.1	135.244	Veg	135.270	0.026
2033	399138.5	4972500.0	135.309	Veg	135.410	0.101
2034	399157.1	4972518.6	134.823	Veg	134.860	0.037
2035	399176.5	4972497.5	134.921	Veg	135.100	0.179
2036	399186.2	4972484.8	135.535	Veg	135.720	0.185
2037	399193.9	4972475.2	135.284	Veg	135.430	0.146
2038	399239.4	4972490.9	135.313	Veg	135.400	0.087
1029	399406.0	4972482.7	134.894	Veg	134.920	0.026
1030	399449.2	4972481.6	135.021	Veg	135.040	0.019
1031	399578.8	4972447.5	137.509	Veg	137.550	0.041
1032	399604.2	4972391.7	138.409	Veg	138.430	0.021
1033	399554.0	4972304.3	138.019	Veg	138.020	0.001
1034	399516.7	4972293.8	137.250	Veg	137.310	0.060
1035	399481.4	4972258.1	135.925	Veg	136.010	0.085
1036	399481.8	4972232.6	135.361	Veg	135.440	0.079
1037	399527.6	4972265.0	135.791	Veg	135.900	0.109
1038	399605.5	4972260.6	134.731	Veg	134.770	0.039
1039	399730.0	4972204.5	134.465	Veg	134.520	0.055
1040	399849.4	4972204.1	136.020	Veg	136.060	0.040
1041	399918.0	4972155.3	138.976	Veg	139.000	0.024
1042	399927.8	4972129.7	139.167	Veg	139.200	0.033
1043	399964.1	4972128.3	138.733	Veg	138.750	0.017
1044	400007.5	4972138.9	136.609	Veg	136.690	0.081
1045	400046.4	4972161.4	135.193	Veg	135.220	0.027
1046	400080.2	4972092.8	135.704	Veg	135.730	0.026

Table A5 -Veg Surface Ground Truthing

ID	X	Y	Z	CODE	DTM	Difference
1047	400025.1	4972107.9	136.263	Veg	136.260	-0.003
1048	399945.2	4972051.0	136.028	Veg	136.020	-0.008
1049	399989.9	4972030.4	136.039	Veg	136.060	0.021
1050	400097.5	4971951.8	138.318	Veg	138.370	0.052
1051	400136.1	4971932.6	137.873	Veg	137.910	0.037
1052	400181.0	4972007.5	137.307	Veg	137.370	0.063
1053	400231.1	4972038.6	138.117	Veg	138.140	0.023
1054	400341.4	4972092.6	134.822	Veg	134.860	0.038
1055	400402.8	4972133.4	134.059	Veg	134.110	0.051

Statistical Summary - Veg Surfaces - Ground Truthing

Table A6: Elevation Difference Statistical Analysis

Statistical Analysis	Unit	Value
Mean	(m)	0.046
Standard Error	(m)	0.004
Median	(m)	0.038
Mode	(m)	0.037
Standard Deviation	(m)	0.039
Sample Variance	(m)	0.001
Kurtosis	-	2.263
Skewness	-	1.325
Range	(m)	0.194
Minimum	(m)	-0.009
Maximum	(m)	0.185
Sum	(m)	4.300
Count	-	93
RMSE	-	0.060
95th Confidence interval (NVA)	-	0.119
95th Percentile (VVA)	-	0.115

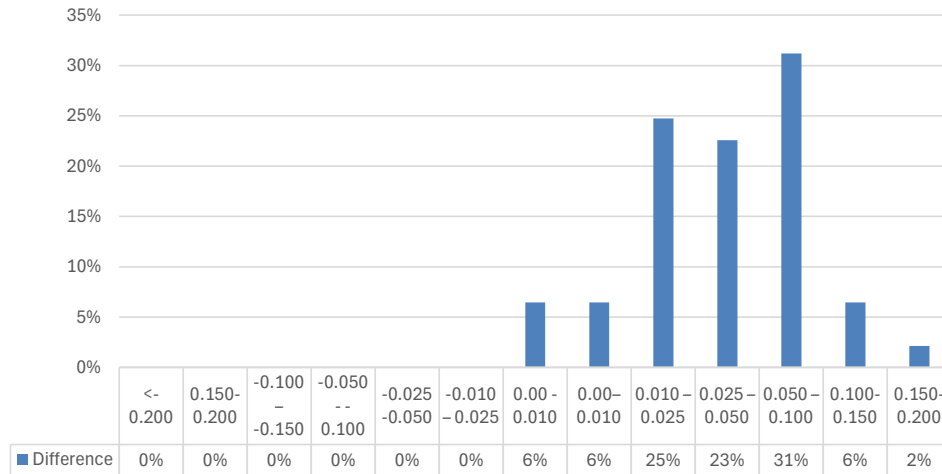
Table A7: Elevation Difference Bins

Bins	Range	Count	Percent
-0.300	<-0.200	0	0%
-0.150	0.150-0.200	0	0%
-0.100	-0.100 - -0.150	0	0%
-0.050	-0.050 - -0.100	0	0%
-0.025	-0.025 -0.050	0	0%
-0.010	-0.010 - 0.025	0	0%
0.000	0.00 - 0.010	6	6%
0.010	0.00-0.010	6	6%
0.025	0.010 - 0.025	23	25%
0.050	0.025 - 0.050	21	23%
0.100	0.050 - 0.100	29	31%
0.150	0.100-0.150	6	6%
0.300	0.150-0.200	2	2%
Sum		93	100.0%

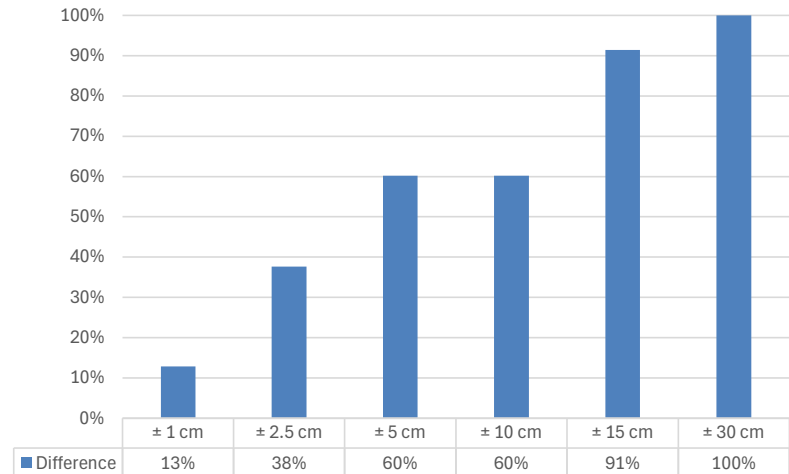
Table A8: Elevation

Distribution Summary	
± 1 cm	13%
± 2.5 cm	38%
± 5 cm	60%
± 10 cm	60%
± 15 cm	91%
± 30 cm	100%

Elevation Difference Distribution



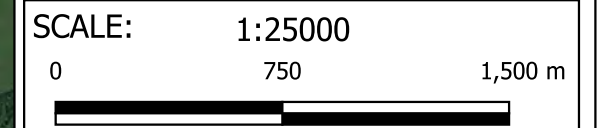
Elevation Difference Distribution Summary





Legend

- Benchmark Points
- Property Boundary



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Perth Golf Course

Figure A3: Benchmarks

PROJECT	2118(01)-21
DRAWN	JZ
DATE	May 2024

Table A9 - Vertical Translation of CGVD 2013 to CGVD 28:78

Benchmark ID	CGVD 2013	CGVD 28:78	Difference
0011915U114G	133.958	134.282	-0.324
0011915U115G	134.118	134.443	-0.325
0011990U001	140.057	140.379	-0.322
0011990U002	136.304	136.627	-0.323
0011990U003	133.535	133.86	-0.325
0011990U004	139.672	139.997	-0.325
00819738524	144.008	144.333	-0.325
00819738525	141.133	141.457	-0.324
00819738526	140.827	141.149	-0.322
00819848111	138.63	138.951	-0.321
		Average	-0.324
		Std Dev	0.001



Ontario Ministry of Natural Resources
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**Control Survey Information
Exchange**

COSINE Station Report

Retrieval Date: 2024-May-28

Station: **0011915U114G**

AKA Names: 00115U114G, 114G, 15U114G, CP90208, V00115U114G

Number of Ref Sketches: 0

Networks [usage]: 0044V [FIX]

Known Status: Existing

Last Reported Visit:

Monument Type: CAP

Station Type: SPIR

Location Description:

Township: PERTH MCMILLAN BUILDING (FORMERLY PUBLIC LIBRARY), ALONG HIGHWAY NO. 43 (GORE STREET), 0.5 KM NORTHWEST OF JUNCTION WITH HIGHWAY NO. 43 HEADING EAST, 0.2 KM SOUTHEAST OF FOSTER STREET, BOLT IN SOUTHWEST STONE WALL, 10.4 M NORTHEAST OF CENTRE LINE OF HIGHWAY NO. 43 (GORE STREET), 13.0 M NORTHWEST OF CENTRE LINE OF BASIN STREET, 4.2 M NORTHWEST OF SOUTH CORNER OF BUILDING, 1.22 M SOUTHEAST OF CENTRE OF MAIN ENTRANCE, 35 CM ABOVE SIDEWALK, SLIGHTLY ABOVE HIGHWAY LEVEL.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified		Ellipsoidal Order: Unclassified	
Latitude: N44° 54' 07.0xxxxx"		Longitude: W76° 15' 21.0xxxxx"		Ellipsoidal elev: 135.xxx	
*UTM Zone: 18	E: E400849.xxx	N: N4972607.xxx	c. s. f.: 0.99969972	Mrdl Conv: -0° 53' 11.6"	
*MTM Zone: 9	E: E324083.xxx	N: N4973361.xxx	c. s. f.: 0.99988340	Mrdl Conv: 0° 10' 20.5"	

Datum: NAD-1983:ORIG		Horiz Order: Unclassified		Ellipsoidal Order:	
Latitude: N44° 54' 10.0xxxxx"		Longitude: W76° 15' 20.0xxxxx"		Ellipsoidal elev:	
*UTM Zone: 18	E: E400875.xxx	N: N4972917.xxx	c. s. f.: 0.99972081	Mrdl Conv: -0° 53' 11.0"	
*MTM Zone: 9	E: E324104.xxx	N: N4973671.xxx	c. s. f.: 0.99990458	Mrdl Conv: 0° 10' 21.2"	

Vertical (Geoidal) Control Data


Datum: CGVD2013		Vert Order: First Order		Elevation: 133.958	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		
Datum: CGVD28:78		Vert Order: First Order		Elevation: 134.282	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		

Maintenance / History

Date	Description
2019-Mar-20	GSC; last inspected: 1990

Reference Sketches

Reference sketch for 0011915U114G is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

COSINE Station Report

Retrieval Date: 2024-May-28

Station: **0011915U115G**

AKA Names: 00115U115G, 115G, 15U115G, CP90208, V00115U115G

Number of Ref Sketches: 0

Networks [usage]: 0044V [FIX]

Known Status: Existing

Last Reported Visit:

Monument Type: CAP

Station Type: SPIR

Location Description:

Township: PERTH TOWN HALL (1863) ALONG HIGHWAY NO. 43 (GORE STREET), 0.4 KM NORTHWEST OF JUNCTION WITH HIGHWAY NO. 43 HEADING EAST, 0.3 KM SOUTHEAST OF FOSTER STREET, 0.2 KM NORTHWEST OF HARVEY STREET, BOLT IN NORTHEAST STONE WALL, 13.0 M NORTHWEST OF CENTRE LINE OF MARKET STREET, 13.8 M SOUTHWEST OF CENTRE LINE OF HIGHWAY NO. 43 (GORE STREET), 3.4 M NORTHWEST OF EAST CORNER OF BUILDING, 9.1 M SOUTHEAST OF CENTRE OF MAIN ENTRANCE, 36 CM ABOVE SIDEWALK, SLIGHTLY ABOVE HIGHWAY LEVEL.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified		Ellipsoidal Order: Unclassified	
Latitude: N44° 54' 00.0xxxxx"		Longitude: W76° 15' 03.0xxxxx"		Ellipsoidal elev: 135.XXX	
*UTM Zone: 18	E: E401240.XXX	N: N4972385.XXX	c. s. F.: 0.99969876	Mrd1 Conv: -0° 52' 58.8"	
*MTM Zone: 9	E: E324478.XXX	N: N4973146.XXX	c. s. F.: 0.99988359	Mrd1 Conv: 0° 10' 33.2"	

Datum: NAD-1983:ORIG		Horiz Order: Unclassified		Ellipsoidal Order:	
Latitude: N44° 54' 10.0xxxxx"		Longitude: W76° 15' 10.0xxxxx"		Ellipsoidal elev:	
*UTM Zone: 18	E: E401095.XXX	N: N4972913.XXX	c. s. F.: 0.99972028	Mrd1 Conv: -0° 53' 03.9"	
*MTM Zone: 9	E: E324323.XXX	N: N4973672.XXX	c. s. F.: 0.99990468	Mrd1 Conv: 0° 10' 28.3"	

Vertical (Geoidal) Control Data

Datum: CGVD2013		Vert Order: First Order		Elevation: 134.118	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		


Datum: CGVD28:78		Vert Order: First Order		Elevation: 134.443	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		

Maintenance / History

Date	Description
2019-Mar-20	GSC; last inspected: 1990

Reference Sketches

Reference sketch for 0011915U115G is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

AKA Names: 00190U001, 90U001, CP90208
Number of Ref Sketches: 0
Networks [usage]:

COSINE Station Report
Retrieval Date: 2024-May-28

Station: **0011990U001**

Known Status: Existing
Last Reported Visit:
Monument Type: -
Station Type: SPIR

Location Description:
Township: PERTH LARGE GREEN SHINGLED DOME STORAGE BUILDING IN GROUNDS OF COUNTY OF LANARK MAINTANANCE GARAGE NO. 1, ALONG HIGHWAY NO. 43 (WILSON STREET), 0.3 KM SOUTHEAST OF JUNCTION WITH HIGHWAY NO 7, 170 M NORTHWEST OF C.P.R. CROSSING, TABLET IN TOP OF CONCRETE SUPPORT PAD ON EASTERLY SIDE OF DOME, 112 M SOUTHWEST OF CENTRE LINE OF HIGHWAY NO. 43 (WILSON STREET), 8.6 M NORTHWEST OF CENTRE OF PAVED ACCESS ROAD NEAR BUILDING, 4.2 M SOUTHWEST OF PROJECTED SOUTHWEST FACE AND 21.0 M NORTHWEST OF PROJECTED NORTHWEST WALL OF LARGE GARAGE, 40 CM ABOVE PAVEMENT, SLIGHTLY ABOVE HIGHWAY LEVEL.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified	Ellipsoidal Order: Unclassified	
Latitude: N44° 54' 25.0xxxxx"		Longitude: W76° 15' 43.0xxxxx"	Ellipsoidal elev: 141.XXX	
*UTM Zone: 18	E: E400375.xxx	N: N4973170.xxx	c. s. f.: 0.99969993	Mrd1 Convg: -0° 53' 27.4"
*MTM Zone: 9	E: E323598.xxx	N: N4973915.xxx	c. s. f.: 0.99988223	Mrd1 Convg: 0° 10' 05.0"

Vertical (Geoidal) Control Data


Datum: CGVD2013		Vert Order: First Order	Elevation: 140.057	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:	
Datum: CGVD28:78		Vert Order: First Order	Elevation: 140.379	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:	

Maintenance / History

Date	Description
2019-Mar-20	GSC; last inspected: 1990

Reference Sketches

Reference sketch for 0011990U001 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

AKA Names: 00190U002, 90U002, CP90208
Number of Ref Sketches: 0
Networks [usage]:

COSINE Station Report
Retrieval Date: 2024-May-28

Station: **0011990U002**

Known Status: Existing
Last Reported Visit:
Monument Type: -
Station Type: SPIR

Location Description:

Township: PERTH GROUND ROD UNDER ACCESS COVER IN GRASSED AREA IN FRONT OF STEWART SCHOOL ON SOUTHWEST SIDE OF HIGHWAY NO. 43 (WILSON STREET), 0.8 KM SOUTHEAST OF JUNCTION WITH HIGHWAY NO. 7, 350 M SOUTHEAST OF CANADIAN PACIFIC RAILWAY CROSSING, 0.6 KM NORTHWEST OF JUNCTION WITH FOSTER STREET, 39.5 M SOUTHWEST OF CENTRE LINE OF HIGHWAY NO 43 (WILSON STREET), 10.2 M NORTHWEST OF CENTRE OF MOST NORTHERLY PORTION OF "U" SHAPED BUS DRIVEWAY IN FRONT OF SCHOOL, 30.2 M SOUTH OF MOST NORTHEASTERLY CORNER OF SCHOOL, 4.0 M SOUTHEAST OF PROJECTION OF SOUTHEAST WALL OF TWO STORY (CIVIC NO. 77) ON OPPOSITE SIDE OF HIGHWAY, SLIGHTLY ABOVE HIGHWAY LEVEL.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified	Ellipsoidal Order: Unclassified
Latitude: N44° 54' 14.0xxxxx"		Longitude: W76° 15' 28.0xxxxx"	Ellipsoidal elev: 137.xxx
*UTM Zone: 18	E: E400699.xxx	N: N4972826.xxx	c. s. f.: 0.99969977
			Mrd1 Convg: -0° 53' 16.7"
*MTM Zone: 9	E: E323929.xxx	N: N4973577.xxx	c. s. f.: 0.99988302
			Mrd1 Convg: 0° 10' 15.6"

Vertical (Geoidal) Control Data

Datum: CGVD2013		Vert Order: First Order	Elevation: 136.304
Geoid:	Meridional defl:	Prime vert defl:	Undulation:


Datum: CGVD28:78		Vert Order: First Order	Elevation: 136.627
Geoid:	Meridional defl:	Prime vert defl:	Undulation:

Maintenance / History

Date	Description
2019-Mar-20	GSC; last inspected: 1990

Reference Sketches

Reference sketch for 0011990U002 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

AKA Names: 00190U003, 90U003, CP90208
Number of Ref Sketches: 0
Networks [usage]:

COSINE Station Report
Retrieval Date: 2024-May-28

Station: **0011990U003**

Known Status: Existing
Last Reported Visit:
Monument Type: -
Station Type: SPIR

Location Description:

Township: PERTH WATER TOWER ALONG HARVEY STREET, 150 M NORTHWEST ALONG GORE STREET FROM INTERSECTION WITH HIGHWAY NO. 43 HEADING EAST, 110 M SOUTHWEST OF JUNCTION WITH GORE STREET, 29.5 M NORTHEAST OF CENTRE LINE OF WILSON STREET EAST, 18.5 M NORTHWEST OF CENTRE LINE OF HARVEY STREET, TABLET IN TOP AT SOUTHEAST CORNER OF CONCRETE BASE, 2.0 M NORTHWEST OF PROJECTION OF SOUTHEAST WALL AND 40 CM SOUTHWEST OF PROJECTION OF NORTHEAST WALL OF SMALL BRICK SHED AT BASE OF TOWER, 5 CM ABOVE GROUND LEVEL, SLIGHTLY BELOW ROAD LEVEL.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified	Ellipsoidal Order: Unclassified	
Latitude: N44° 53' 49.0xxxxx"		Longitude: W76° 14' 52.0xxxxx"	Ellipsoidal elev: 134.xxx	
*UTM Zone: 18	E: E401476.xxx	N: N4972042.xxx	c. s. f.: 0.99969835	Mrd1 Conv: -0° 52' 50.9"
*MTM Zone: 9	E: E324721.xxx	N: N4972808.xxx	c. s. f.: 0.99988387	Mrd1 Conv: 0° 10' 40.9"

Vertical (Geoidal) Control Data

Datum: CGVD2013	Vert Order: First Order	Elevation: 133.535	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:


Datum: CGVD28:78	Vert Order: First Order	Elevation: 133.860	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:

Maintenance / History

Date	Description
2019-Mar-20	GSC; last inspected: 1990

Reference Sketches

Reference sketch for 0011990U003 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

AKA Names: 00190U004, 90U004, CP90208
Number of Ref Sketches: 0
Networks [usage]:

COSINE Station Report
Retrieval Date: 2024-May-28

Station: **0011990U004**

Known Status: Existing
Last Reported Visit:
Monument Type: -
Station Type: SPIR

Location Description:
Township: PERTH QUEEN ELIZABETH SCHOOL ALONG WILSON STREET EAST, 0.4 KM SOUTHEAST ALONG GORE STREET FROM INTERSECTION WITH HIGHWAY NO. 43 HEADING EAST, 150 M SOUTHWEST ALONG HALTON STREET, 90 M NORTHWEST OF CENTRE LINE OF HALTON STREET, 50 M SOUTHWEST OF CENTRE LINE OF WILSON STREET EAST, TABLET IN NORTHEASTERLY FACE OF CONCRETE FOUNDATION WALL, 30 CM NORTHWEST OF JOG IN NORTHEAST FACE OF SCHOOL, 11.3 M SOUTH AND 3.3 M EAST OF FLAG POLE, 15 CM ABOVE PAVEMENT, SLIGHTLY ABOVE ROAD LEVEL.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified	Ellipsoidal Order: Unclassified	
Latitude: N44° 53' 31.0xxxxx"		Longitude: W76° 14' 34.0xxxxx"	Ellipsoidal elev: 140.xxx	
*UTM Zone: 18	E: E401863.xxx	N: N4971481.xxx	c. s. f.: 0.99969647	Mrd1 Conv: -0° 52' 37.9"
*MTM Zone: 9	E: E325117.xxx	N: N4972253.xxx	c. s. f.: 0.99988312	Mrd1 Conv: 0° 10' 53.5"

Vertical (Geoidal) Control Data


Datum: CGVD2013		Vert Order: First Order	Elevation: 139.672	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:	
Datum: CGVD28:78		Vert Order: First Order	Elevation: 139.997	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:	

Maintenance / History

Date	Description
2019-Mar-20	GSC; last inspected: 1990

Reference Sketches

Reference sketch for 0011990U004 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

AKA Names: 738524, V008738524
Number of Ref Sketches: 0
Networks [usage]: 0044V [FIX]

COSINE Station Report
Retrieval Date: 2024-May-28

Station: **00819738524**

Known Status: Existing
Last Reported Visit:
Monument Type: CAP
Station Type: SPIR

Location Description:
738524: SMALL FLAT ROCK OUTCROP ON SOUTH SIDE OF HWY 7, 3.5 KM WEST OF JCT OF HWYS 7 AND 43 IN THE TOWN OF PERTH, 1.2 KM EAST OF GLEN TAY RD, 49.1 M EAST OF ENTRANCE TO MOTOR VEHICLE REPAIR SHOP AND 50.6 M SOUTH OF CENTRELINE OF HWY 7. TABLET IS SET VERTICALLY IN TOP OF ROCK, 22.2 M SOUTH OF SOUTH RAIL OF CPR, 17.5 M SOUTH OF SOUTH RAILWAY RIGHT-OF-WAY FENCE, 17.1 M EAST OF PROPERTY LINE FENCE AND IS MARKED BY A MARKER PICKET SET 24 CM SOUTH OF BENCH MARK.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified		Ellipsoidal Order: Unclassified	
Latitude: N44° 53' 30.0xxxxx"		Longitude: W76° 18' 00.0xxxxx"		Ellipsoidal elev: 144.xxx	
*UTM Zone: 18	E: E397344.xxx	N: N4971520.xxx	c. s. f.: 0.99970700	Mrdl Conv: -0° 55' 03.3"	
*MTM Zone: 9	E: E320597.xxx	N: N4972210.xxx	c. s. f.: 0.99988049	Mrdl Conv: 0° 08' 28.2"	

Datum: NAD-1983:ORIG		Horiz Order: Unclassified		Ellipsoidal Order:	
Latitude: N44° 53' 30.0xxxxx"		Longitude: W76° 18' 00.0xxxxx"		Ellipsoidal elev:	
*UTM Zone: 18	E: E397347.xxx	N: N4971738.xxx	c. s. f.: 0.99972957	Mrdl Conv: -0° 55' 03.3"	
*MTM Zone: 9	E: E320597.xxx	N: N4972427.xxx	c. s. f.: 0.99990306	Mrdl Conv: 0° 08' 28.2"	

Vertical (Geoidal) Control Data

Datum: CGVD2013	Vert Order: First Order	Elevation: 144.008	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:


Datum: CGVD28:78	Vert Order: First Order	Elevation: 144.333	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:

Maintenance / History

Date	Description
2019-Mar-20	MTO

Reference Sketches

Reference sketch for 00819738524 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

COSINE Station Report

Retrieval Date: 2024-May-28

Station: **00819738525**

AKA Names: 0011973D8525, 00173D8525, 6009738525, 738525, 73D8525, CP90208, V008738525

Number of Ref Sketches: 0

Networks [usage]: 0033V [FIX], 0044V [FIX]

Known Status: Existing

Last Reported Visit:

Monument Type: CAP

Station Type: SPIR

Location Description:

Township: PERTH SMALL FLAT ROCK OUTCROP ON NORTH SIDE OF HWY 7, 1.9 KM WEST OF JCT OF HWYS 7 AND 43 IN THE TOWN OF PERTH, 1.2 KM WEST OF LANARK CTY RD 1, 30 M EAST OF ENTRANCE TO MRS. NORA ROSE RESIDENCE AND 36.0 M NORTH OF CENTRELINE OF HWY 7. TABLET IS SET VERTICALLY IN TOP OF ROCK, 17.7 M NORTH OF NORTH RIGHT-OF-WAY FENCE, 19.0 M EAST OF PROPERTY LINE FENCE AND IS MARKED BY A MARKER PICKET SET 21 CM NORTH OF BENCH MARK.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified		Ellipsoidal Order: Unclassified	
Latitude: N44° 54' 00.0xxxxx"		Longitude: W76° 17' 00.0xxxxx"		Ellipsoidal elev: 141.xxx	
*UTM Zone: 18	E: E398674.xxx	N: N4972425.xxx	c. s. F.: 0.99970414	Mrdl Conv: -0° 54' 21.4"	
*MTM Zone: 9	E: E321911.xxx	N: N4973139.xxx	c. s. F.: 0.99988149	Mrdl Conv: 0° 09' 10.6"	

Datum: NAD-1983:ORIG		Horiz Order: Unclassified		Ellipsoidal Order:	
Latitude: N44° 54' 10.0xxxxx"		Longitude: W76° 17' 10.0xxxxx"		Ellipsoidal elev:	
*UTM Zone: 18	E: E398463.xxx	N: N4972954.xxx	c. s. F.: 0.99972677	Mrdl Conv: -0° 54' 28.6"	
*MTM Zone: 9	E: E321691.xxx	N: N4973664.xxx	c. s. F.: 0.99990350	Mrdl Conv: 0° 09' 03.5"	

Vertical (Geoidal) Control Data

Datum: CGVD2013		Vert Order: First Order		Elevation: 141.133	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		


Datum: CGVD28:78		Vert Order: First Order		Elevation: 141.457	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		

Maintenance / History

Date	Description
2019-Mar-20	MT0; GSC: last inspected: 1990

Reference Sketches

Reference sketch for 00819738525 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

COSINE Station Report

Retrieval Date: 2024-May-28

Station: **00819738526**

AKA Names: 0011973D8526, 00173D8526, 6009738526, 738526, 73D8526, V008738526, VA215

Number of Ref Sketches: 0

Networks [usage]: 0033V [FIX], 0044V [FIX]

Known Status: Existing

Last Reported Visit:

Monument Type: CAP

Station Type: SPIR

Location Description:

Township: PERTH LARGE CONCRETE BLOCK WORKSHOP ON SOUTH SIDE OF HWY 7, 31 M EAST OF JCT OF HWYS 7 AND 43 IN THE TOWN OF PERTH, 0.7 KM EAST OF LANARK CTY RD 1 AND 29.0 M SOUTH OF CENTRELINE OF HWY 7. TABLET IS SET HORIZONTALLY IN WEST FACE OF CONCRETE FOUNDATION ,4.6 M SOUTH OF N.W. CORNER, 15 CM ABOVE GROUND AND 15 CM BELOW BLOCKWORK.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified		Ellipsoidal Order: Unclassified	
Latitude: N44° 54' 30.0xxxxx"		Longitude: W76° 15' 48.0xxxxx"		Ellipsoidal elev: 141.xxx	
*UTM Zone: 18	E: E400268.xxx	N: N4973326.xxx	c. s. F.: 0.99970020	Mrdl Conv: -0° 53' 31.0"	
*MTM Zone: 9	E: E323488.xxx	N: N4974069.xxx	c. s. F.: 0.99988218	Mrdl Conv: 0° 10' 01.5"	

Datum: NAD-1983:ORIG		Horiz Order: Unclassified		Ellipsoidal Order:	
Latitude: N44° 54' 40.0xxxxx"		Longitude: W76° 15' 50.0xxxxx"		Ellipsoidal elev:	
*UTM Zone: 18	E: E400232.xxx	N: N4973852.xxx	c. s. F.: 0.99972239	Mrdl Conv: -0° 53' 32.6"	
*MTM Zone: 9	E: E323443.xxx	N: N4974595.xxx	c. s. F.: 0.99990427	Mrdl Conv: 0° 10' 00.1"	

Vertical (Geoidal) Control Data

Datum: CGVD2013		Vert Order: First Order		Elevation: 140.827	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		


Datum: CGVD28:78		Vert Order: First Order		Elevation: 141.149	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:		

Maintenance / History

Date	Description
2019-Mar-20	MTO; GSC: last inspected: --

Reference Sketches

Reference sketch for 00819738526 is not available.



Ontario Ministry of Natural Resources
and Forestry

**Control Survey Information
Exchange**

COSINE Station Report

Retrieval Date: 2024-May-28

Station: **00819848111**

Known Status: Existing

Last Reported Visit:

Monument Type: BM

Station Type: SPIR

AKA Names: 848111

Number of Ref Sketches: 0

Networks [usage]:

Location Description:

848111: CONCRETE BRIDGE CARRYING HWY 511 OVER BLUEBERRY MARSH, 1.00 KM NORTH OF JCT OF HWYS 7 AND 511 IN PERTH, 0.40 KM SOUTH OF BATHURST-DRUMMOND CON 4 RD. TABLET IS SET HORIZONTALLY IN THE EAST FACE OF COPING AT S.E. CORNER OF BRIDGE, 1.90 M NORTH OF THE S.E. CORNER, 10 CM BELOW TOP OF COPING, 16 CM ABOVE BOTTOM OF COPING AND 5.80 M EAST OF CENTRELINE OF HWY 511.

No Photo

Horizontal (Ellipsoidal) Control Data

Datum: NAD-1927:SCAL		Horiz Order: Unclassified	Ellipsoidal Order: Unclassified	
Latitude: N44° 54' 48.0xxxxx"		Longitude: W76° 16' 42.0xxxxx"	Ellipsoidal elev: 139.xxx	
*UTM Zone: 18	E: E399092.xxx	N: N4973900.xxx	c. s. f.: 0.99970341	Mrd1 Conv: -0° 54' 09.5"
*MTM Zone: 9	E: E322302.xxx	N: N4974622.xxx	c. s. f.: 0.99988197	Mrd1 Conv: 0° 09' 23.4"

Vertical (Geoidal) Control Data

Datum: CGVD2013		Vert Order: First Order	Elevation: 138.630	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:	
Datum: CGVD28:78		Vert Order: First Order	Elevation: 138.951	
Geoid:	Meridional defl:	Prime vert defl:	Undulation:	

Maintenance / History

Date	Description
2019-Mar-20	MTO

Reference Sketches

Reference sketch for 00819848111 is not available.